

		REINFORCING STEEL		<del></del> _	T			-				
NO. SIZE LENGTH MARK LOCATION	BENDING SKETCHES	& CUTTING DIAGRAMS	NO.	_	· .		LOCATION	NO,	SIZE	LENGTH	MARK	LOCATIO
Substructure Int. Bts. 2.3,48.5		191 3:84	Supe		ucture		Bent "G			tructui		Corit)
2 11 24'-3" F21 Facting	14.75 14.75 14.75	19\$" 3' 12' 19\$" 3'8\$" 11' 9\$1	1 2	3	12:01	611	Wing	964	7 5	5: "	R7	POTOF
G 11 15'G" F22 " 2 145 15'-G" F23 "	14-75	<u> </u>	27	5	5:0"	C/3		<del> </del>			100	
* G 7:0" F24 "			2	9	15:9"	C14 C61	<u>"</u>		12	206	R9	
13 9 219" F25 "		Control of	3	8	18:5"	C62	//	1	1	23.9	211	
4 // 17-3" F3/ "	10:72 8:72"	8:64 6:118	<u> </u>	-	1-10-3	1002		1-:-	15	2/:3"	RIZ	
8 6 6-9 152	/9!9" /9:3"	16-0" 15-6"	8	9	37:5"	HII	Beam	8	+=	26.9		//
3 // 20:3" F5/ "	3-H15 Cut 3 3-H16 Cut 3	3 417 cut 3 3 418 cut3	8	6	34'-6"	H12	.,	1	<del>                                     </del>	1200	1,7,5	
7 G 8:9" F52 "		-1" -1"	/	6	12:6"	1119	Wing	1		<del>†-</del>	1	
4 11 5'9 F26 "	20 5-Ca" 2 1 1-93" 16-715 16-81	5" 222" 62" 235"	3	7	16:0"	H17		3	5	29'3"	R16	,
Substructure Enc Bent 6	16-15-4		3	6	15:6"	H18	"	24	5	25'.3"	R17	./
8 9 4-9" DG/ Footing	Commence of the commence of th		3	7	22'-0"	.461	- 77	12	15	22:6'	R18	//
0 5 4:3" D62 "	100 012 01 03 01		3	6	21:6"	H62	<del></del>	8	5	28:3"	R19	
0 0/ 0// 1/5/ 3-1/	12.24 9:94 11:114 9:64	222" 6:102" 234" 6:98"	1/2	<u>(4</u>	17-6"	1463	· · ·	ļ., <u></u>	<del> </del>		-	
3   9   4'-9"   VG1   Column   3   9   12'-3"   VG2   "	22:0" 21:6	8:9" 8:9"	12	Ĝ	8:3"	1419		22	1//	19:6"	13/	Top Sk
9 11:0" V63 "	3 HG/ Cut 3 3-HG2 Cut3	13-VII cut 13 10-VIZ CUT 20	2	<i>5</i>	4:9"	RI	End Post	22	10	32:6"	52	,, ,
3 9 9 9 164	42" 21" 203"				5'6"	R2		25	10	51-9"	53	" "
7 3 8:0 V66 "		_20\$"	2	<b>5</b>	613"	R3 R4		22	11	19:9" 34:0"	34	" "
	1 3 R	3"R	2	5	6'-9" 7:0"	RG RS	n 11	25	1//	55:3"	S5	,, ,,
IDERSTRUCTURE End Bent #1	1/25	1/25	4	5	7:0"	RG RG	, , ,	22	1/0	19:6"	56 57	" "
8   16-0"   C10   Ning	2/* 7:0"		18	9	5:3"	R7	Parapet	22	11	35'3	38	
E 13:0" (11 "	8'9" 303	· 605 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	4	5	10:3"	R15	"	25	1//	58:3"	59	11
6 13:9" C12 "	15-VG5 CU1 15 3'R	1 1	4	5	15:3"	RI4	.,	22	10	18:9"	3/0	"
6 10:9" C13 "	3'R	3'R						22	10	32:6"	511	11
5 5:0" C/4 "	12.5	12.0	2	6	16:0	712	Wing	25	10	52:9"	5/2	,, ,
	*G3	*62	2	6	20'-6"	761	//	70	4	21:0"	5/3	
9 37:0" HII Beam	_	1 - 3"R						70	4	21:9"	5/4	" '
6 34-6" H12 "	15" 34" 6" 15" HII	id /25	70	6	11:-G"	UII	<u> Seam</u>	70	4	20°C"	5/5	
6 15.6" HI3 Wing	15" 34-6" 15" 111	115-001						35	14	28:3"	316	,,
6 12-6" H14 "	212 13:8" 212 F31	100 VE 1 - 204"	10	4	8:9"	V12	Wing	70	4	19:9"	5/7	,, ,
7 19 <sup>1</sup> 9" H15 "	2/2 16:8" 2/2" F5/	12 1/34	/5	4	8:9"	V65	2 11					
	H//-F3/-F5/	137R		اا				13.14	6	34:6"	37/	TOP 31
7 16'-0" H17 " 6 15'-6" H18 "	HII-FUI-FUI	12.5			erstru			160	7	34:6"	S72	
6 15:6" H18 " 6 8:3" H19 "		* GI-G4	4	5	34:3"	<i>C</i> /	Curb	192	5	33! 9"	573	
5 4.9" RI End Post	B 22 m	1/2 i W/	12	5	32:0" 29:3"	C2 C3	.,	192		35:6"	574	"
5 5 6" R2 " "	333	13 W2 3 9 P2/	9	5	33:0"			192	5	35!O"	ST5	
5 63" 83 " "	12 100	34 W3 P21-V6-6	1	5	32:9"	C4	. "	21	8	54.0"	SB/	Bott 3
5 6'9" R4 " "		parameter in the contract of t	12	5	30°G"	<u>C</u> G		20	9	50:0"	302	<u> Bott. 3</u> "
5 7'0" R5 . " "			6	6	28:0"	20		18	9	35!9"	383	
5 7:0" RG "	W1-W2 W	'3 <del> <sup>''</sup>  </del>	1	6		23	;	21	9	65' 6"	3B4	
5 5:3" R7 Parapet	+ $+$ $+$ $+$ $+$ $+$ $+$ $+$ $+$ $+$		78:-	5		29	//	20	10	59:3"	385	"
5 13:3" P8 "			387	6		<u>67</u>	Girder	18	10	44'9"	586	
5 10:3" R15 "	MARK A B C INTT		1181	6		<u>02</u>	<i>"</i>	21	73	68:0	387	"
	R2 2-1" 72" 82"	9	//97	6	8:6"	G3	"	20	10	60:0"	588	,,
6 18:6" TII Wing	R3 2:53 73" 83"	`LJ	403	6	8:6"	GA	,,	18	10	44:3"	559	"
1772	K4 1 1 /2   W 2	C.	12	Ö,	and the second s	re-re-re-re-re-re-re-re-re-re-re-re-re-r		- Visitantijanaja		3/15	28/0	,,
	R5 2:10" 72" 82"	4:5" C9	6	8	42:3"	G12	7,	20	8	5/:0"	SBII	17
6 11-6" UII Beam	RG 2:102 72" 72"					7		18	10	43:3"	38/2	,, ,,
+ + + + + + + + + + + + + + + + + + + +	R7 2:0" 7½" 7½"		40	4	27:6"	G14	17	21	8		<i>58/3</i>	,, ,
4 8'9" VII Wing	R2-R3-R4-R5-R6-R7	U21	16	4	31:6"	G15	"	20	9		SB14	" ,
4 8:9" VIZ "		13-6" 2.5" 711						18	9		55/5	" "
7-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1		10:7" 2:1" 112		8		C21	"	21	8		SBIG	" '
erstructure Inl. Bis 2, 3,46 5 1 145 346 5	2º 9" UII 6:2½" 6:2½"	5:5 2:2 76		8	42' 9"			21	2	30'.3"		· · ·
3 145 34'-G" H21 Beom 145 20'-0" H22 "	14:10" F21	139 ; FET - A	40	4	27:9"			21	8		SB18	
6 29.6" H23		- 12 9 0 0 1 1 1 M	16	4	3/-9"			21:	8		SB/9	"
3 7 29-6" [424 "	Sign / 1/3	0/0/0/0	12	8	47:3"			<i>3</i> 63		<u>-29:3"</u>	SB20	
+ + + + + + + + + + + + + + + + + +	W5	たたけ大火火ン		3		G33	,,	40	6	5 <sup>:</sup> 9"	3821	11 /1
8 4 12'9" P21 Column	W5	222		4	28:3" ( 32:3" (	G34			<del> </del>		<del> </del>	
2 290011	7747	WATER TO THE PARTY OF THE PARTY	16	8	47:9"			121	1	5'C"	W/	Dinah
2 6 18 9" UZI Beam	UII-F21	T11-T12-TG1	<del></del>	8	43:9"		· · ·	126				Diogh "
			6	<u>ر</u>	42.8	542		12G 12G		4.6"	W2 W3	
11 35'6" Y21 Column	$\frac{132''}{82''}$ $82''$ $7-3'$	. MA	40	4	28:6"	.44		126	6		WA	
o 9 28'C' V31		The state of the s		4	32:9",		<del></del>	84	6		W5	"
a 9 27:0" V41 "			1.50	<del>-</del>	Ja- J . K	272		42		8:3"	NG	" "
? 9 35'3" 1/5/ "	W O KW WIG	1 6 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	<del>                                     </del>					46	-	<u>ر- ر</u>	110	
	17/9	W46:48" NO	<b></b>			<b></b>			<del> </del>		<del>                                     </del>	
<u> </u>	C14	WG. 6'6"	$\vdash$	+					+			

5151: 110:	MO.	1 403. 70.	10	NO.	SHEETS
FED. ROAD DIST. NO.		FED AID PROJ. NO.		SHEEY NO.	TOTAL SHEETS

ESTIMATED QUANTITIES								
ITEM		SUBSTR.	SUPERSTR.	TOTAL				
Closs ! Excavation for Structur	es Cu. Yds.	-7.7.0		430				
Steel Piles in Place (10")	1.7. F.		ļ	.3.2				
Steel Pile in Place(12")	LIO Ft.	123		198				
Class B Concrete	Cu. Yds.	3.3	<del> </del>	9:3				
Class Bl Concrete	Cu. Vds		1083.9	1083.9				
Reinforcing Steel	16s	94.90	369,510	374,300				
Fobricated Structural Carbon	Steel Lbs.		900	300				
Bridge Rail (Single Tube Ty	(pc) Lin Ft.		617	E17				

Note: Concrete in end posts, parapets and curtis is included with superstructure concrete. All concrete and reinforcement acove fastings in Intermediate bents is included in superstructure quantities. No payment for excovotion will be allowed of End Bent No. 1 .

#### GENERAL NOTES

Design Specifications : A. A.S.H.O. - 1961 Design Loading: 4320-44 15#15q.ft. Future Wearing Eurface
Modified 24,000 Tondem Fixle
Earth 120 Equivalent Fluid Pressure 30# Design Unit Stresses: Class B (oncret (substructure) fo: 1,200ps).

Class Bi Concrete (superstructure) fo: 1,600,051.

Reinforcing Steel fs: 20,000 psi.

Steel Pile (A.S.T.M. A3G-G2T) fb: 9,000 psi. Superstructure deck to be surface sealed. Fainting: Structural Steel acress store shall be cleaned and pointed in the field or may be cleaned and pointed one cout of red ead in the shop with the wo remaining costs applied in the field, except that final roat on access doors about be gruy. In lieu of pointing, the contractor may, if he prefers, galvanize this material. Oll galvanizing shall be done offer fabrication. Cost of printing or galvanizing to be included in price bid for other tems.

> BRIDGE OVER RAMP | & RIGHT LANE I-29 STATE ROAD - INTERSTATE ROUTE 29 IN KANSAS CITY PROJECT NO. [-29-1(36) (RTE. I-29) STA. 43+89.45 RAMP 4 PLATTE COUNTY

> > A-1688

BURGWIN & MARTIN CONSULTING ENGINEERS NEDF D. Foland DETAILED J. Carter

ON CKAG. LOTHOM DETAIL CK. A.G. LOTHO

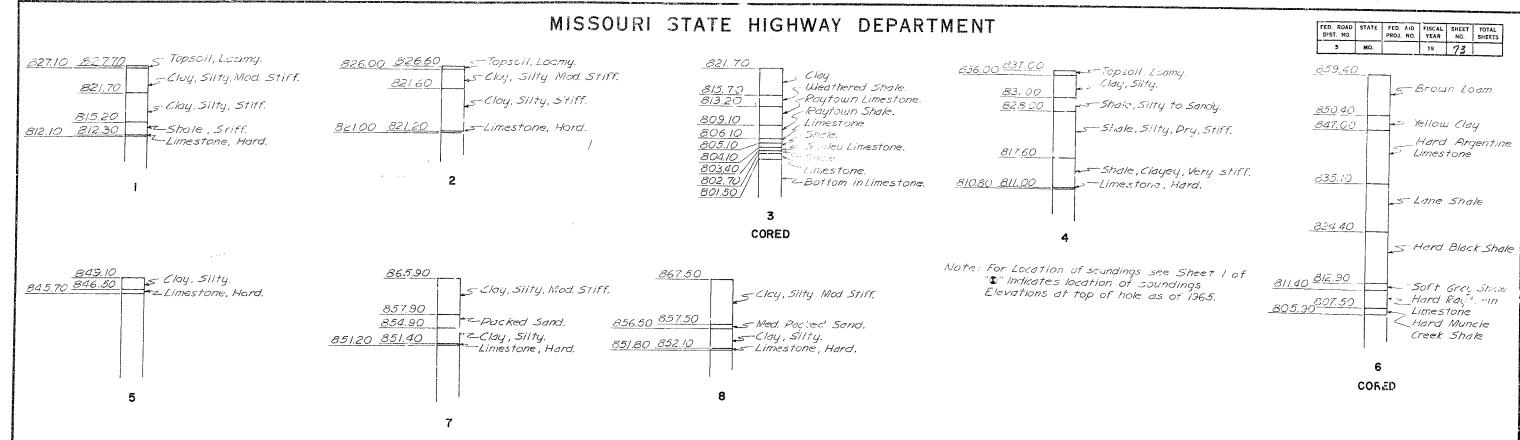
\* Top bend may be made in field or shop. X May ben I in field or shop.

Note: This drawing is not to scale. Follow dimensions.

Sheet No. 2 of 22.

## FOR INFORMATION ONLY

# A16884, Sht. 5



BORING DATA

Note: For location of borings see Sheet No. 1 of 21.

	F	OOTING	AND	PIL	E.	DAT	Α		
	BENT	NO.		. 1	2	3	4	5	6
Spend	Founda	tion Mat	eriol.			Rock	Pock	Shole	Rock
Footions	1.0000	according ,	101010111	1	8 .	9.5	9.0	56	73
	See St	ondara	'Spec	2/fic	2011	0175	50	2.4.2	?
		e and	Size	108902	2825				
Bearing	Number	_		5	111				
-		nate Lei							
Piles .	Design E	Bearing Va	TUE TOTA	5/	70				
	Hoi. imer	Erxergy Re	ort* Ft In	11.500	15700				

Note: Footings shall be carried G" into hord, solid, undisturbed rock or 18" into soil rock or shale and cost against vertical faces of same.

In no case shall footings of all Interimediale Bents be placed higher than elevations snown.

\* Minimum Energy Requirement of hammer based on plan length and design bearing value at piles. Increase by the factor (W+W) 2W when the weight of the rom (W) 15 less than the weight be driven to practical refusal.

BRIDGE OVER RAMP I & RIGHT LANE I-29

STATE ROAD - INTERSTATE ROUTE 29
IN KANSAS CITY

FROJECT NO. I-29-I(36)/(RTE. I-29) STA. 43+89.45 RAMP 4

PLATTE

COUNTY

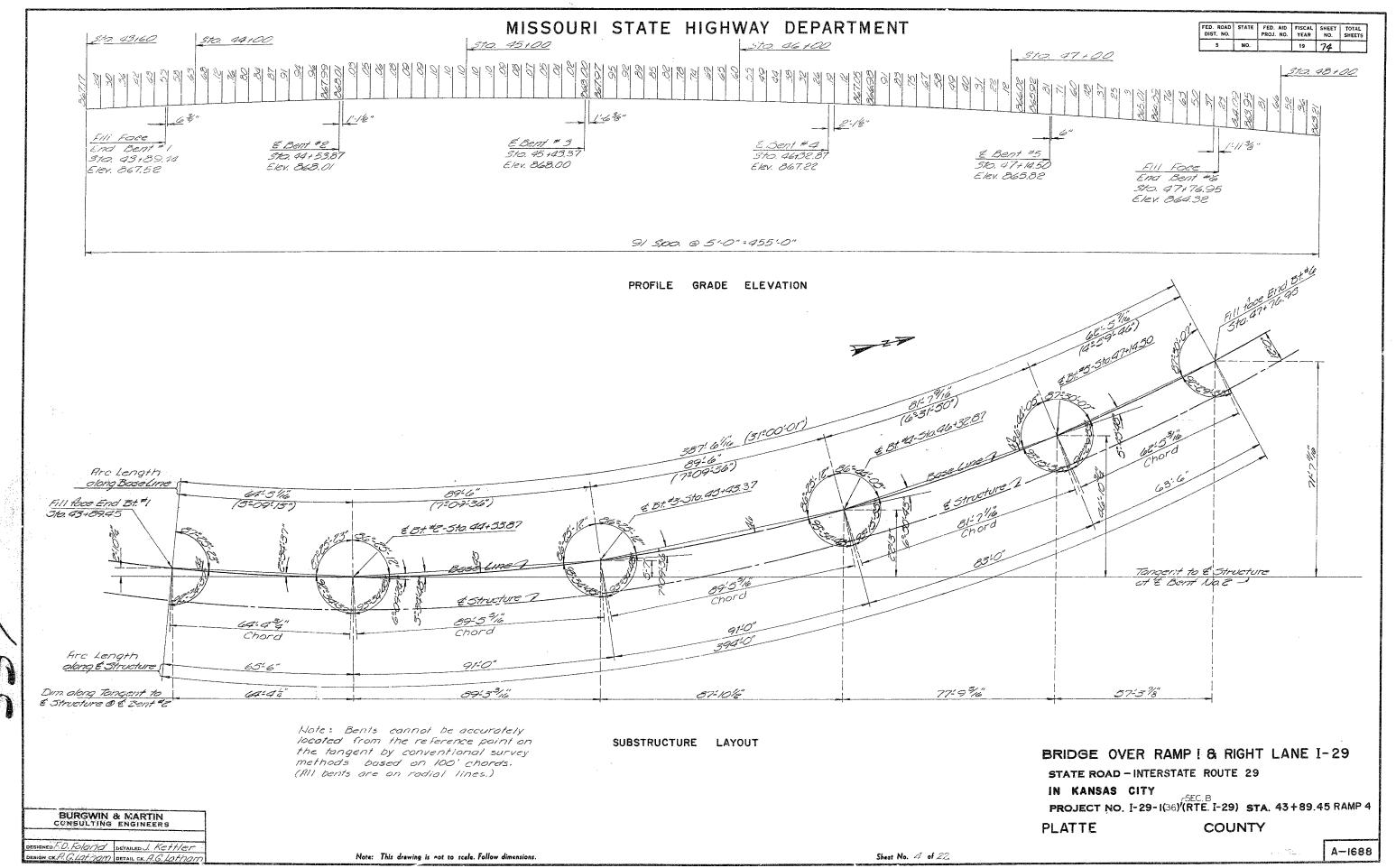
A-1688

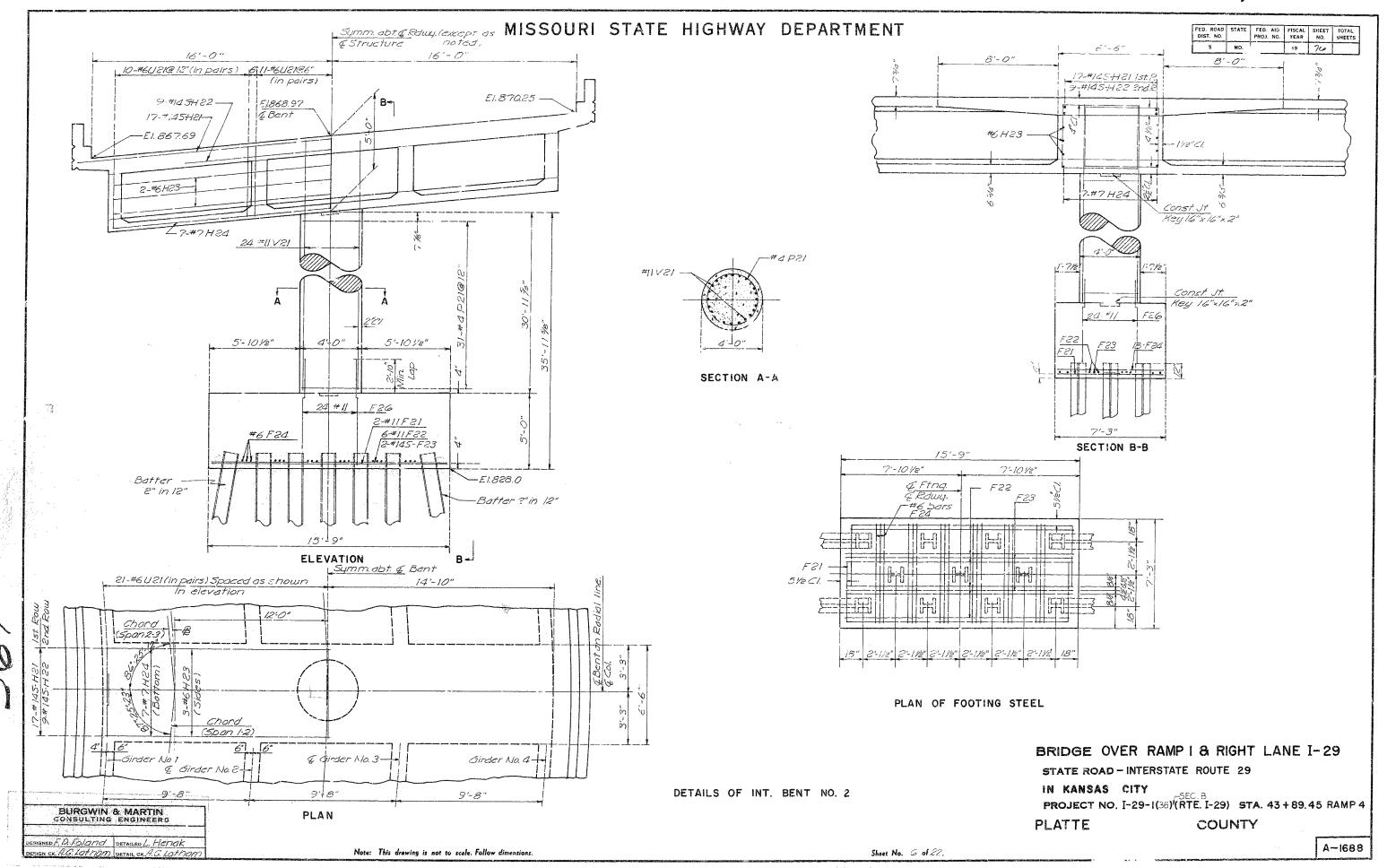
BURGWIN & MARTIN
CONSULTING ENGINEERS

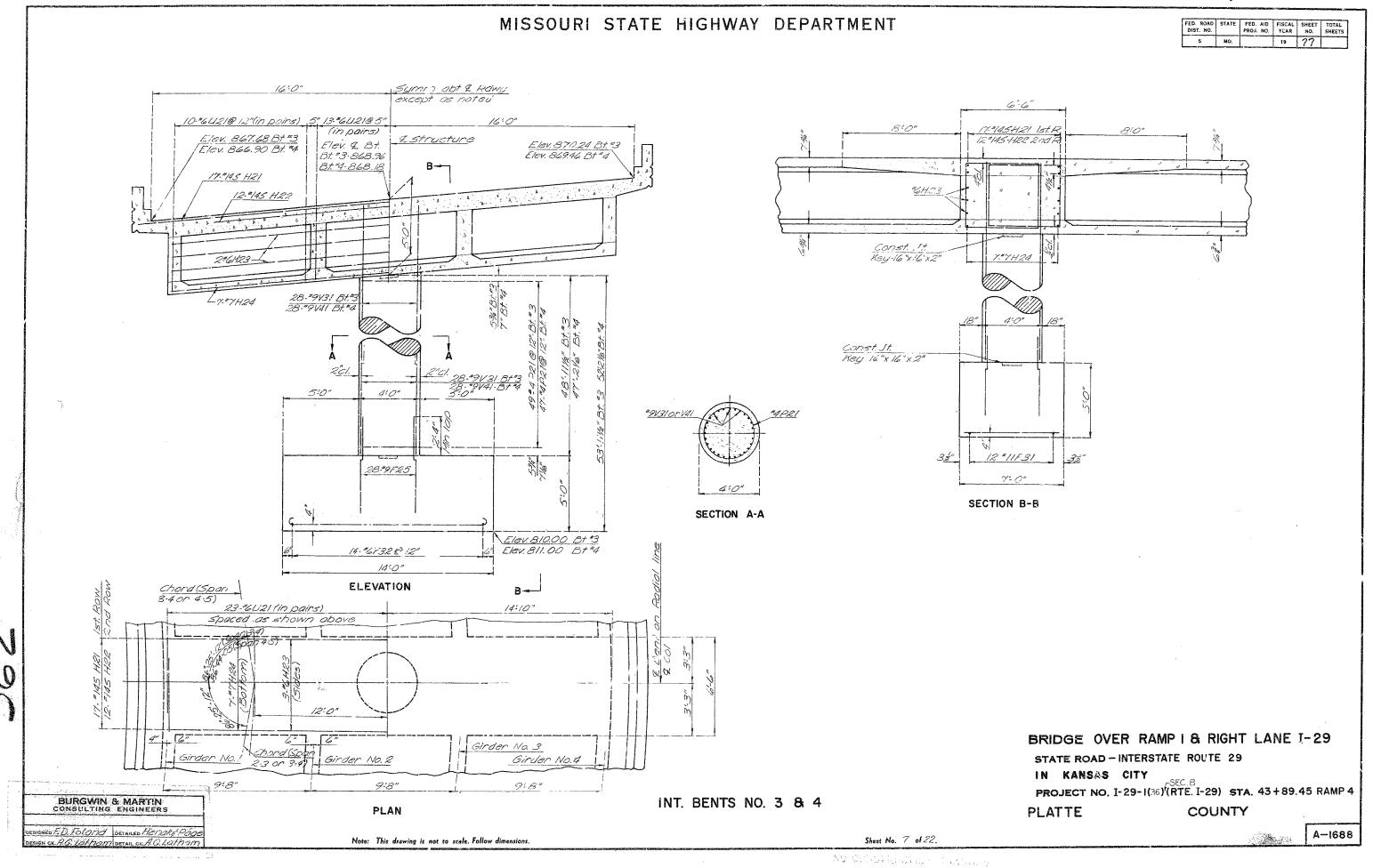
DISIGNED TO FOLOTIO DETAILED L. HENOK
SESSON CK. A.G. LOSTOM
DETAIL CK. A.G. LOSTOM

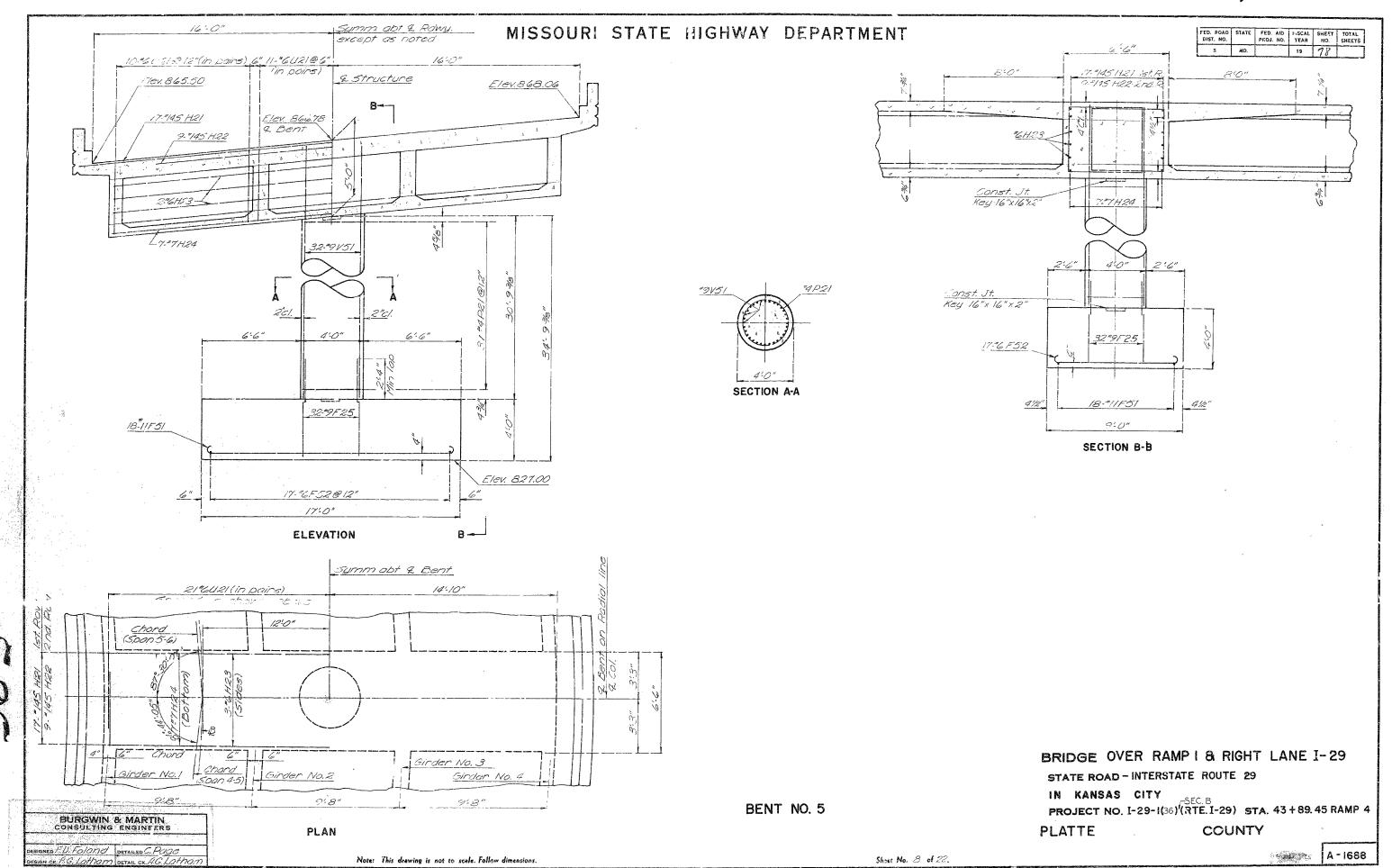
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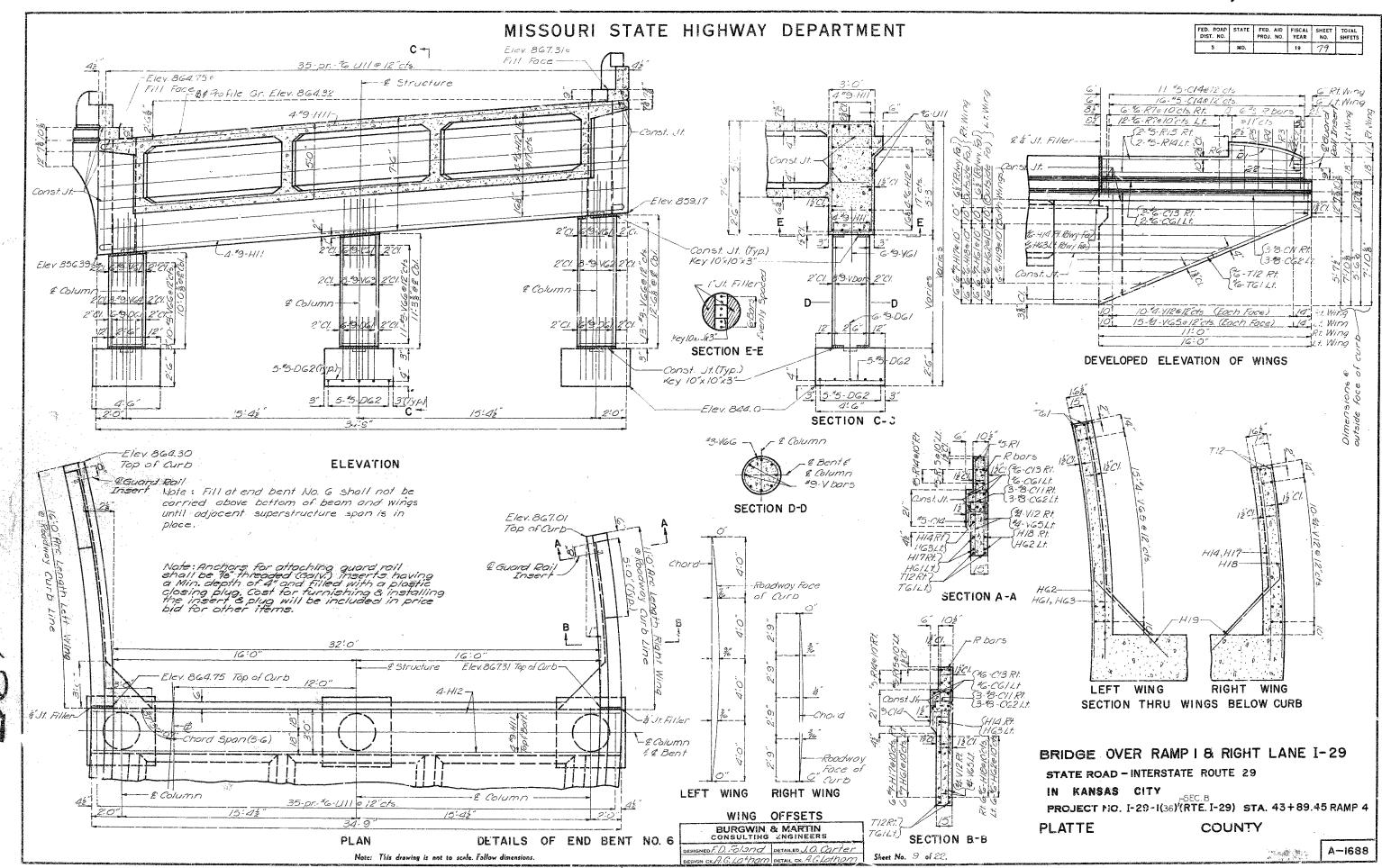
Sheet No. 3 of 22.











BURGWIN & MARTIN CONSULTING ENGINEERS

DESIGNED F.D. FOLOND DETAILED HEMOKENOTTIS

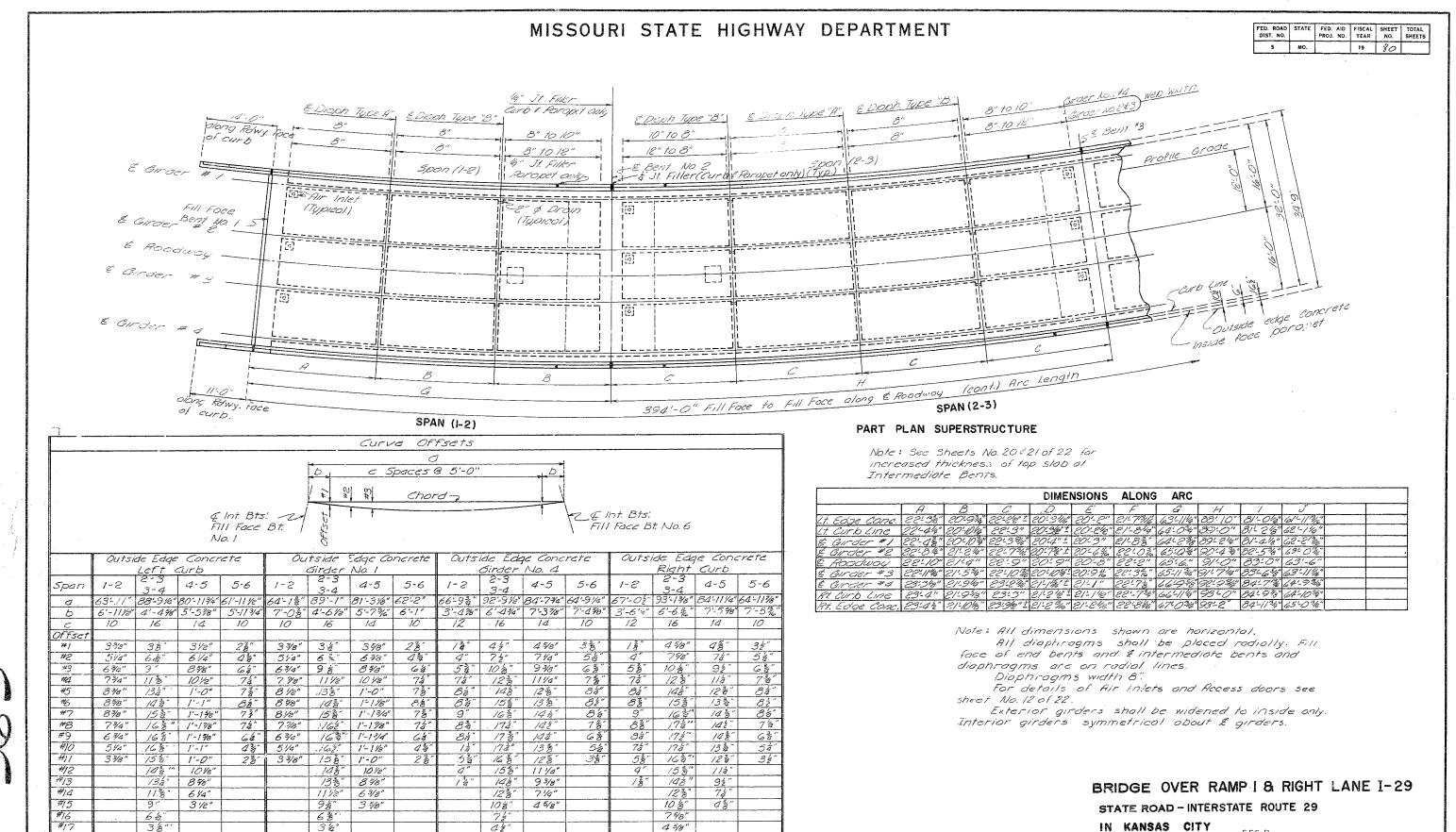
Note: This drawing is not to scale. Follow dimensions

PROJECT NO. I-29-1(36) (RTE. I-29) STA. 43+89.45 RAMP 4

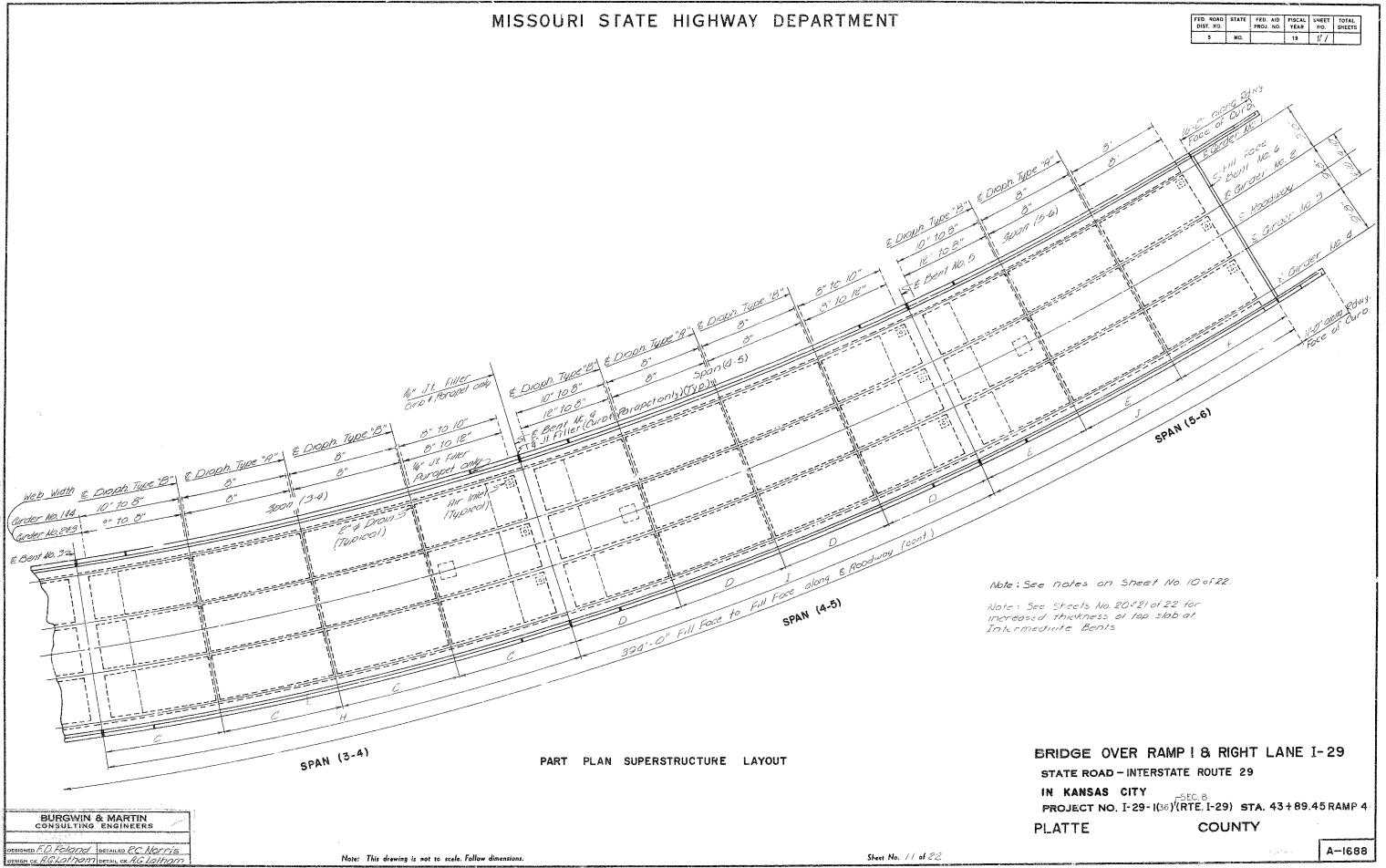
PLATTE

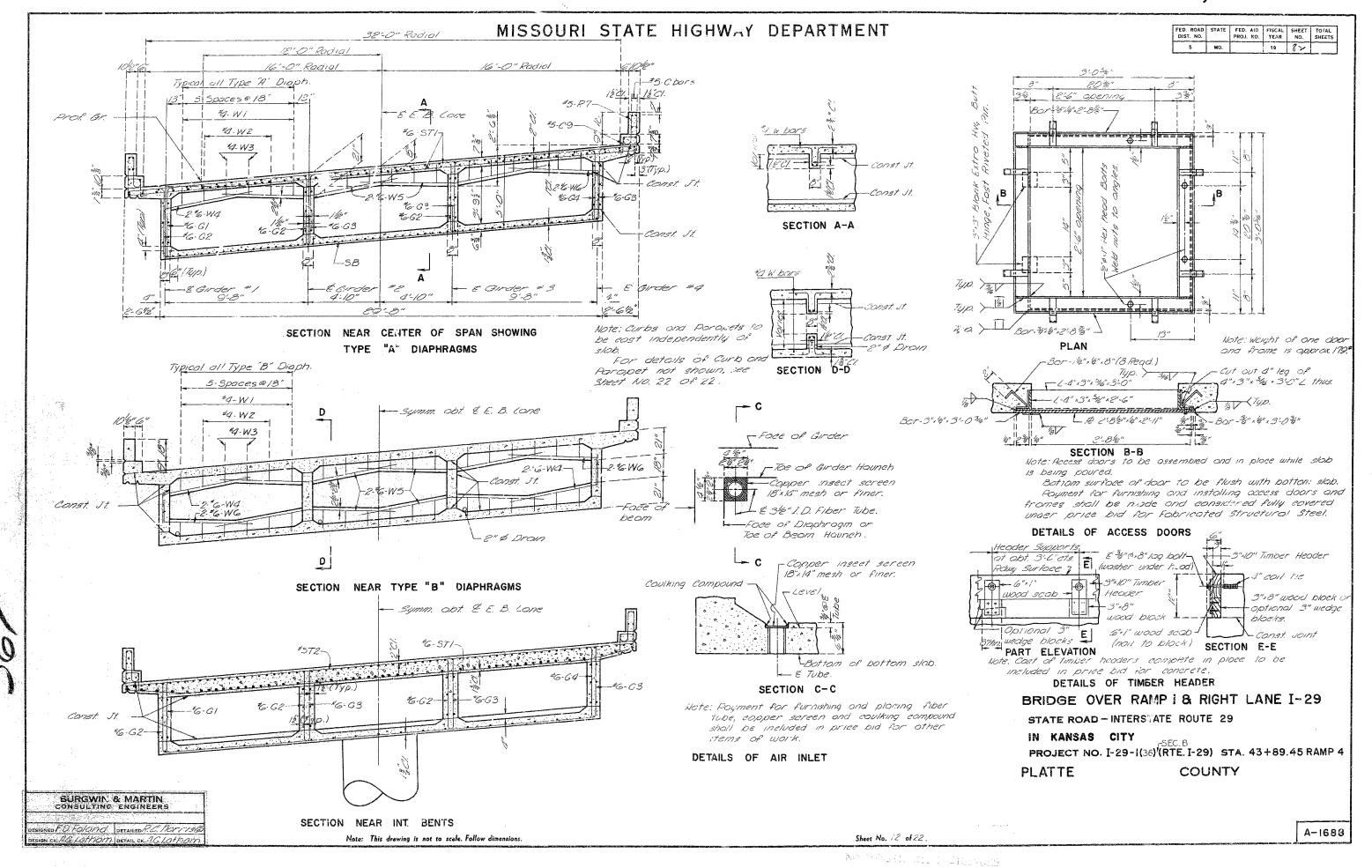
COUNTY

A-1788

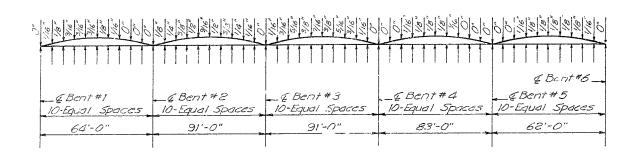


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FED. ROAD DIST. NO.		FED. AID PROJ. NO.		SHEZT NO.	TOTAL SHEETS
5	MO.		19	83	



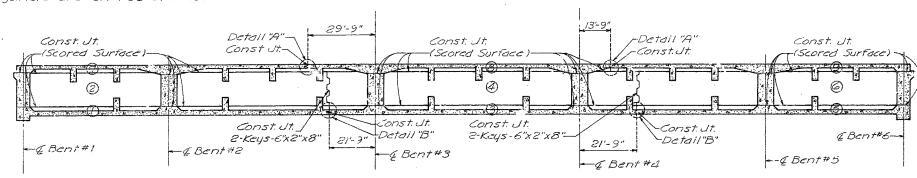
DETAIL "A"

Bottom of Key to extend full Bottom slab.— width of slab.

CAMBER DIAGRAM

DETAIL "B"

Note: Longitudinal dimensions shown are are lengths at 4 structure. All transverse construction joinsts are on rodial lines.



# LONGITUDINAL SECTION SHOWING POURING SEQUENCE

Note: "Numbers in circles indicate the basic pouring sequence. Langitudinal joints in roadway slabs, unless specifically on plans, will not be permitted.

The contractor all use an approved oscillating screed type, self-propelled mechanical finishing machine and shall pour roadway slabs at a rate of not less than 25 cubic yards per hour. He shall observe the basic pouring sequence unless he can demonstrate to the engineer that he can pour and satisfactorily

finish the superstructure concrete at a rate which will permit the combining of such of the basic pours as may be specifically designated by the engineer as being compatible with design. Finish machine loads will not be permitted on concrete less than 48 hours old.

With use of forms and basic falsework meeting the approval of the engineer, the girder webs and diaphragms may be poured with the bottom slab sections on which they

All forms shall be removed from the interior of box girders except top slab forms which may be left in place.

200

BURGWIN & MARTIN CONSULTING ENGINEERS

DESIGNED F.D. FOLAND DETAILED L. HENAK DESIGN CK. P.G. LOT NAM DETAIL CK. R.G. LOT NON BRIDGE OVER RAMP I & RIGHT LANE I-29
STATE ROAD-INTERSTATE ROUTE 29

IN KANSAS CITY

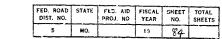
PROJECT NO. I-29-I(36) (RTE. I-29) STA. 43+89.45 RAMP 4

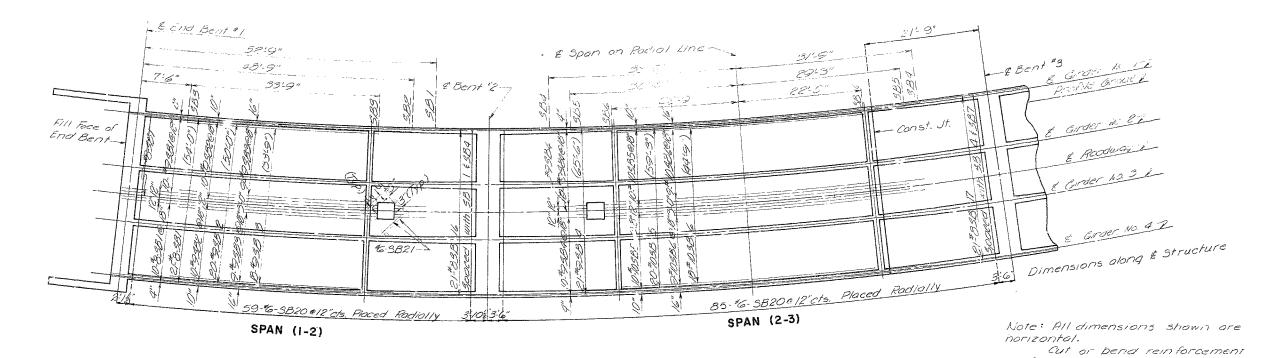
PLATTE

COUNTY



A-1688





REINFORCING IN BOTTOM SLAB

BURGWIN & MARTIN CONSULTING ENGINEERS ESIGNED F. D. FOLOTIO DETAILED Z.C. NOTTIS

Note: This drawing is not to scale, Follow dimensions.

BRIDGE OVER RAMP I & RIGHT LANE I-29

STATE ROAD - INTERSTATE ROUTE 29

in field to clear access doors.

Longitudine! reinforcement to be placed on are parallel to

center line.

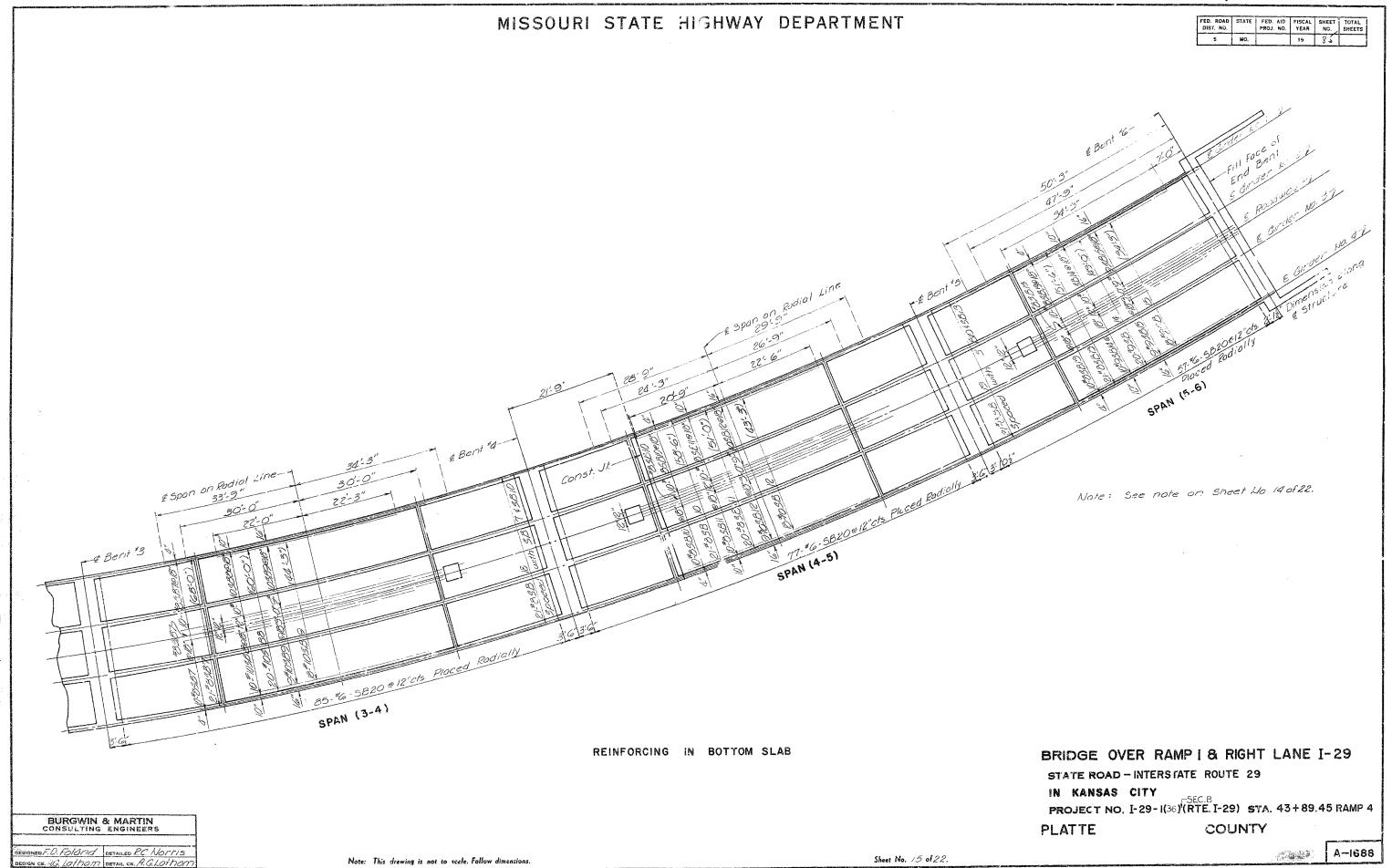
IN KANSAS CITY

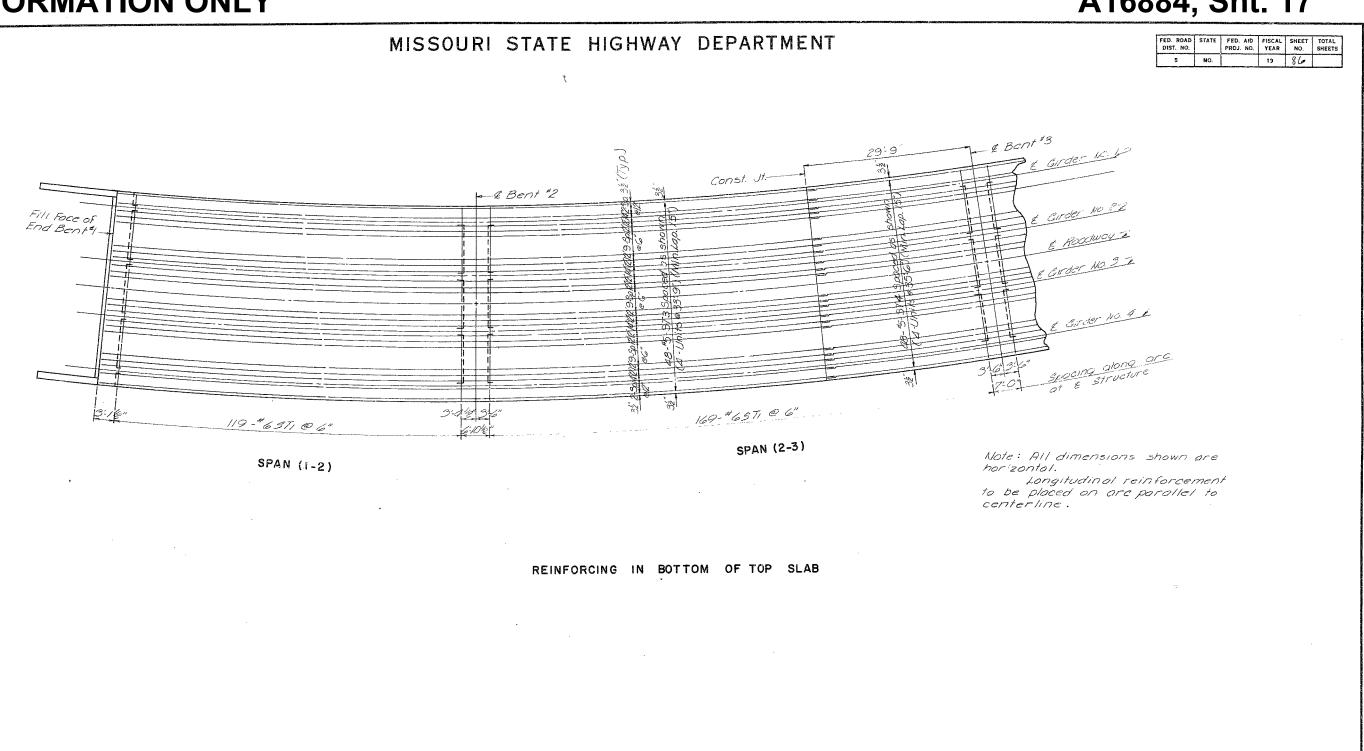
PROJECT NO. I-29-I(36) (RTE. I-29) STA. 43+89.45 RAMP 4

PLATTE

COUNTY

A-1688





BURGWIN & MARTIN CONSULTING ENGINEERS

esign cx: #G. Lo Floam detail cx. #G. Latham

BRIDGE OVER RAMP | & RIGHT LANE I-29 STATE ROAD - INTERSTATE ROUTE 29

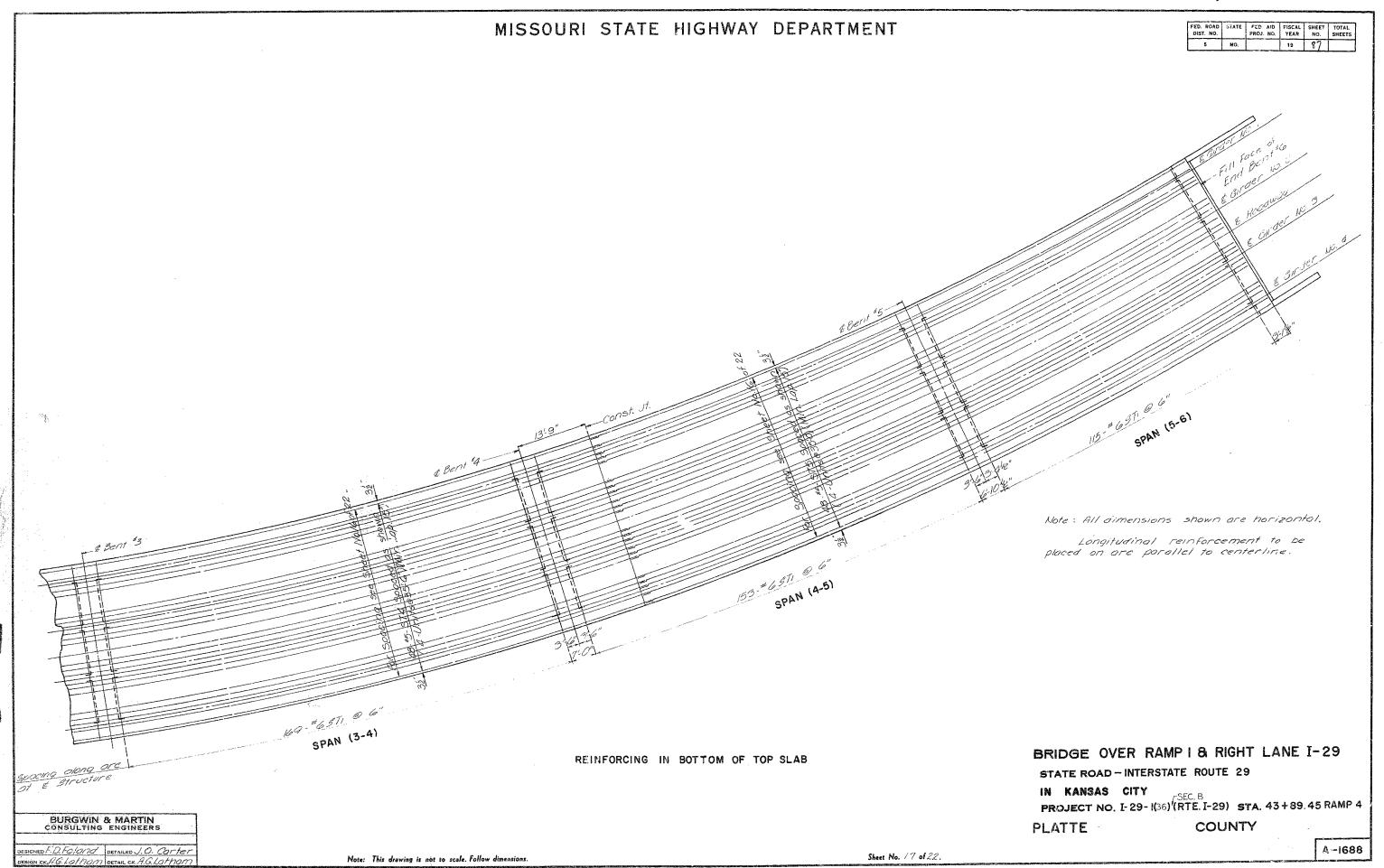
IN KANSAS CITY

PROJECT NO. I-29-1(36) (RTE. I-29) STA. 43+89.45 RAMP 4

PLATTE

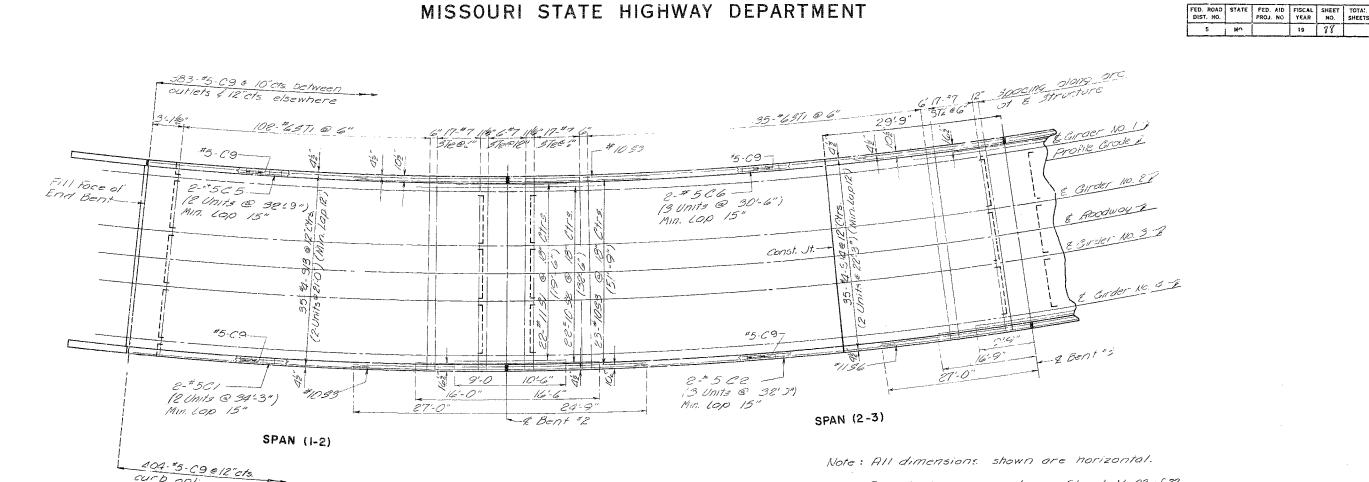
COUNTY

2.13/146/0 A-1688



curb only

19 88



REINFORCING IN TOP OF TOP SLAB

BRIDGE OVER RAMP I & RIGHT LANE I-29

STATE ROAD - INTERSTATE ROUTE 29

IN KANSAS CITY

PROJECT NO. I-29-1(36)/(RTE.I-29) STA. 43+89.45 RAMP 4

PLATTE

For steel in parapet see Sheet No. 22 of 22. Longitudinal reinforcement to be piaced on

are parallel to centerline.

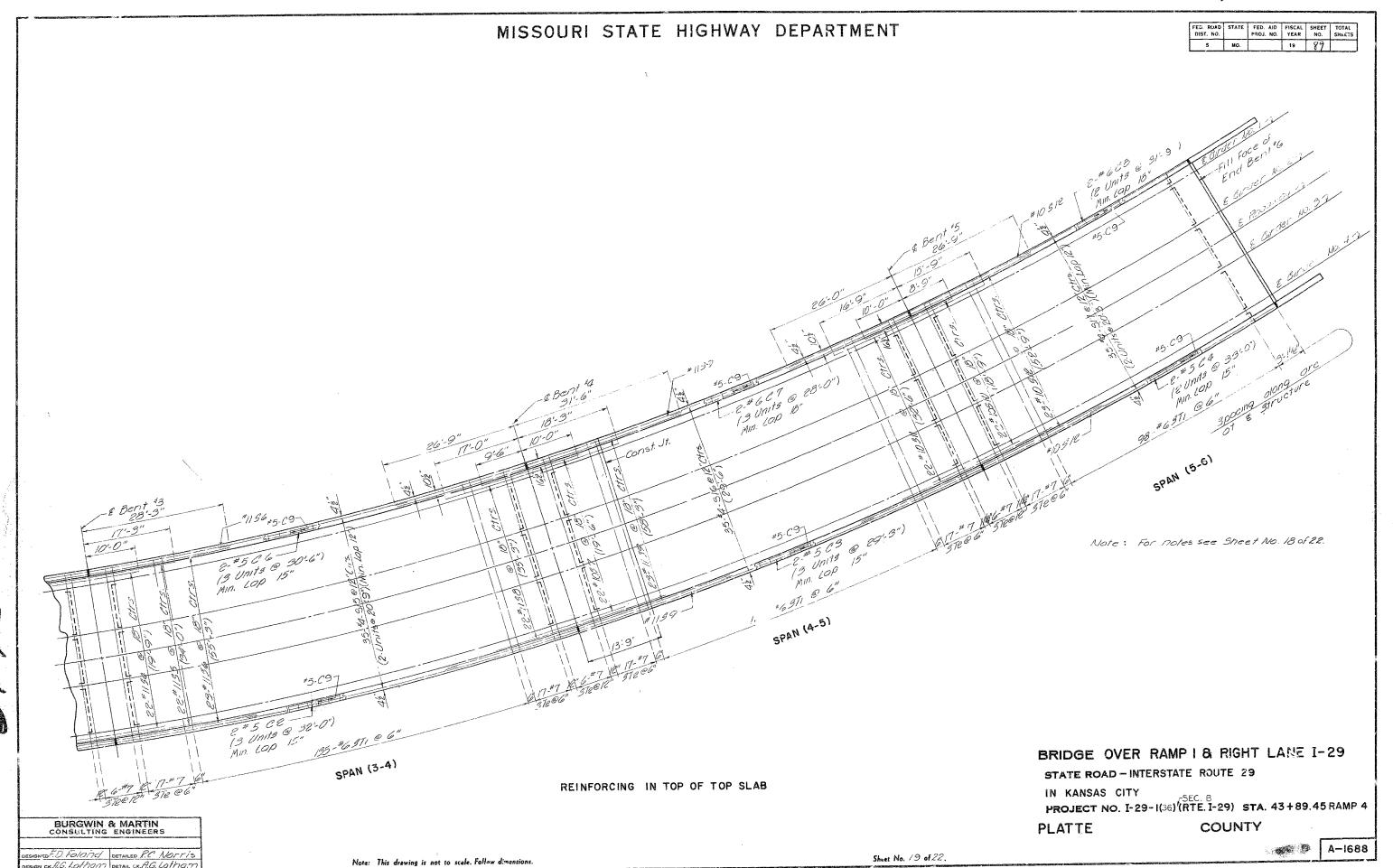
COUNTY

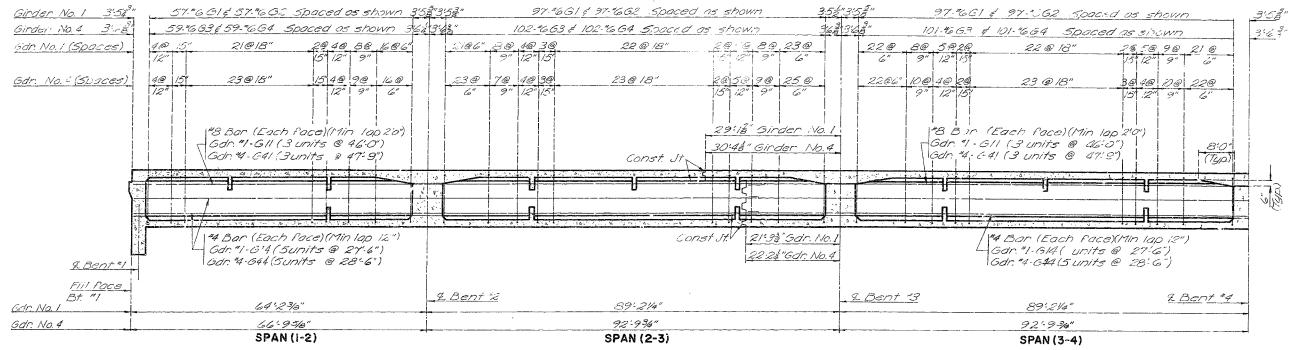
A-1688

BURGWIN & MARTIN CONSULTING ENGINEERS DESIGNED D. FOLONO DETAILED R.C. NORTIS
DESIGN CK. H. LOTHOM DETAIL CK. A.G. LOTHOM

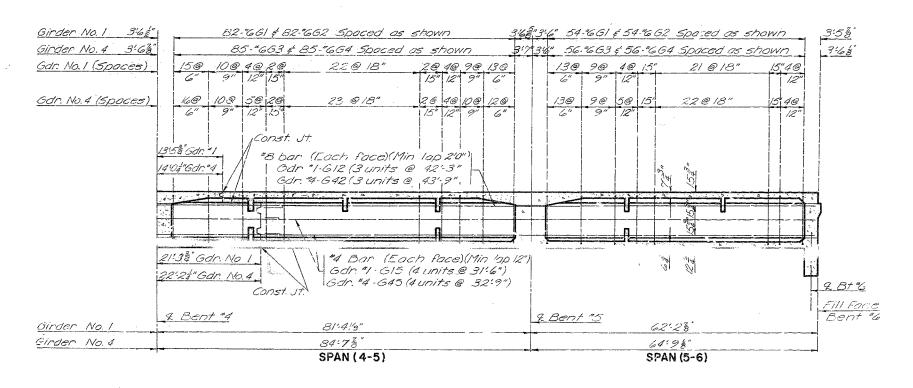
Note: This drawing is not to scale. Follow dimensions.

Sheet No. 18 of 22.





Note: Longitudinal dimensions shown are Horizontal Arc Lengths.



EXTERIOR GIRDER ELEVATIONS

BRIDGE OVER RAMP I & RIGHT LANE I-29

STATE ROAD - INTERSTATE ROUTE 29

IN KANSAS CITY

PROJECT NO. I-29-I(36) (RTE. I-29) STA. 43+89.45 RAMP 4

PLATTE COUNTY

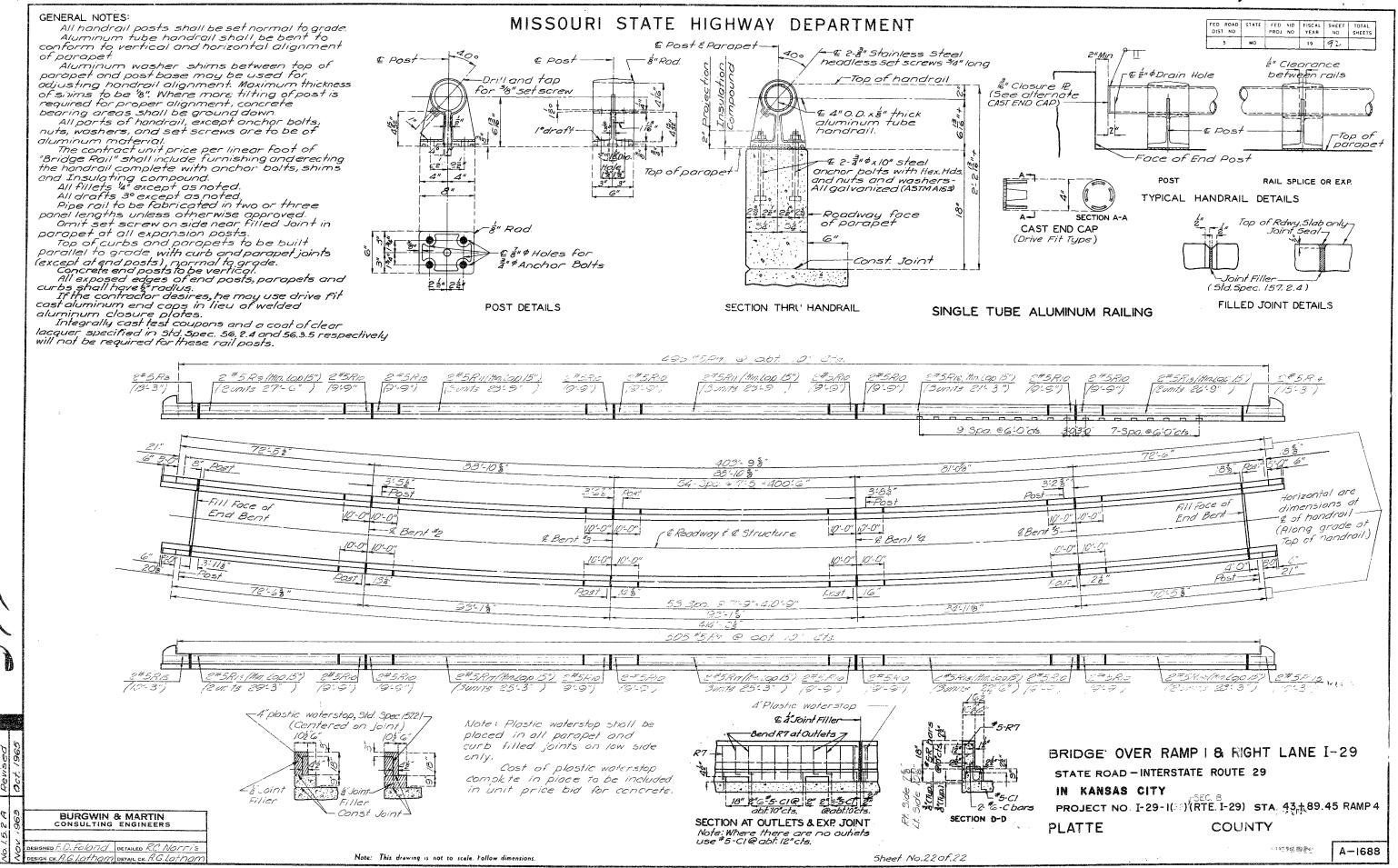
DESIGNAD FOLDING DETAILED C. POGS

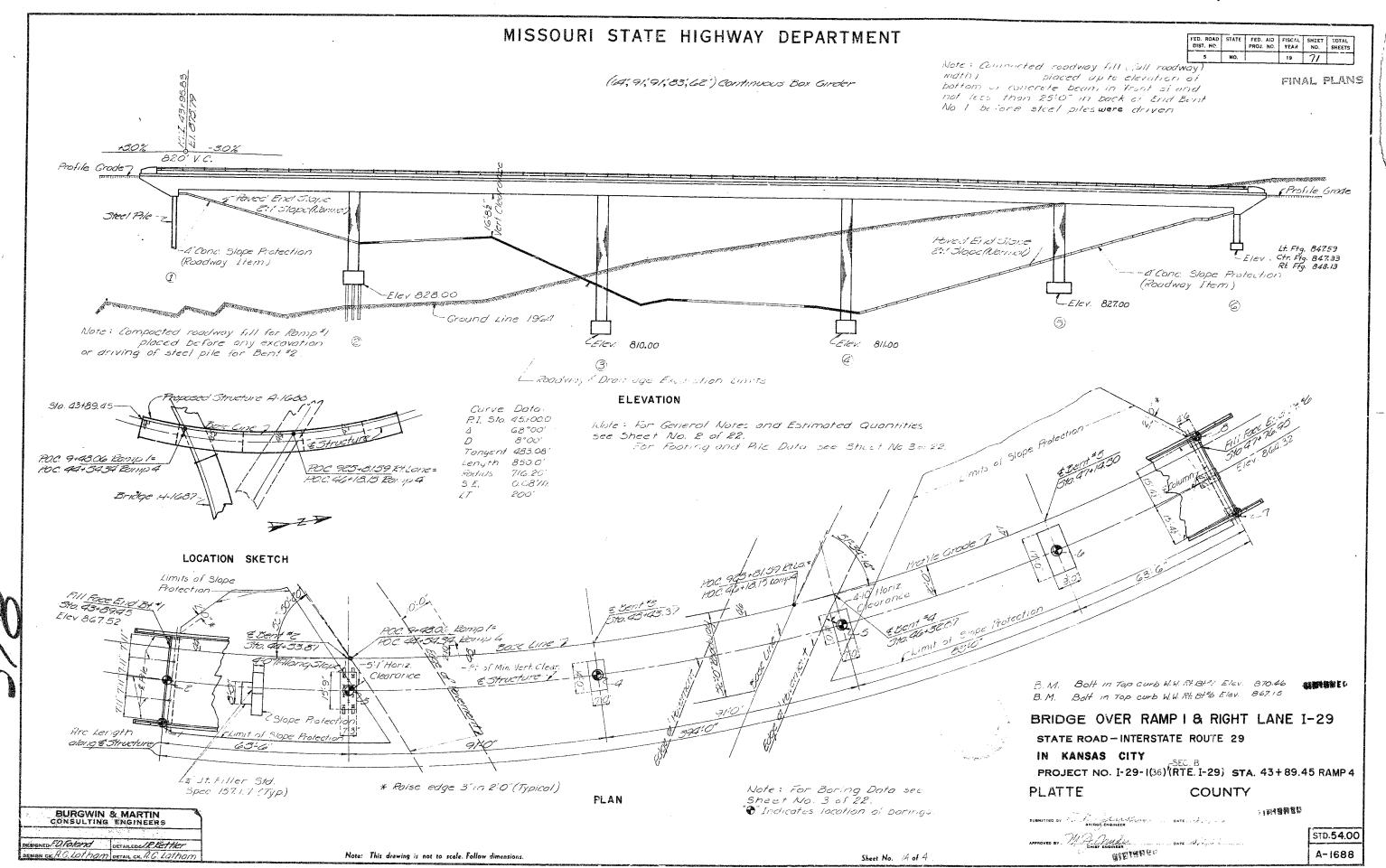
BURGWIN & MARTIN CONSULTING ENGINEERS

Note: This drawing is not to scale. Follow dimensions

Sheet No. 20 of 22.

A-1688





BURGWIN & MARTIN CONSULTING ENGINEERS

IGNED FUTURIS DEVAILED I CUTTET

## MISSOURI STATE HIGHWAY DEPARTMENT

	COMPLETE BILL OF REINFORCING STEEL	4.		J							
NO. SIZE LENGTH MARK LOCATION	BENDING SKETCHES & CUTTING DIAGRAMS	NO.	SIZI	E LENGT	TH MAI	K LOCATION	NO.	SIZE	LENGTH	MARK	LOCATION
Lostructure Int. Bts. 2.3.4455		Sup	ersti	ructur	e En	d Bent &G.			structu		Conit.)
2 1/ 3" 121 Footing	2:0" 4'-10:1" 2:0" 4" 75" [95" 3' 112" 195" 3:82"	13	8	13:			960		5:31	R7	Porop
G // /5'.6" F22 " 2 /45 /5'.6" F23	11:93	1-2	6	10:5			1				
12 6 7:0: 524 "		27	5	15: 3			69	15	27.6"	R9	
9 2'9" F25 "		3	8		5" CG		29	5	23.9	RIO	
24	1012 8 10 1 10 75 8:15 5:98 7:25 8:65 6:116			3.55		~	1/2	10	2/:3"	P/2	.,
28 6 6 9 532	19:9" 19:3" 16:0" 15:6"	8	9		_		18	5	26.9		
/8	3-H15 Cut 3 3-H16 Cut 3 3 H17 Cut 3 3 H18 Cut 3	8	6						- <del>+-</del>	I	
32 1/1 5 9 526	24 500 242 49 5 225 62 234	3	5	12:0			+	+	-		<del> </del>
Subst. ucture fine Bent 6		3	6	15:0			24	15	25:3"	RIG RII	.9
18 9 4:9" DGI Footing		3	7	225			12	+=	22:5	1215	<u> </u>
$50$ 5 4.3 DG2 " $\sim$		3	6	21-6	5" HG		8	5	28:3"	R19	"
18 9 4:9" VGI Column	VC 22 1 9 9% VI 1/4 9 6% 222 6 102" 134" 6 9%		0	17:6	HG						
18 9 4.9" VG1 Column 8 9 8-2" VG2 82"	2200 216 39	12	5	8:			22	144	19:6"	15/	Top Slut
3 9 7'-8" VG3 78"	3 HG1 Cut 3 3-14G2 Cut 3 13-VII Cut 13 10-VI2 Cc 20	2	5	5.0			22	10	132.6	<i>S2</i>	, ,
8 9 6'2" V64	42 21	2	5	6'3		<del></del>	22	177	19' 9"	54	, ,
24 5 8:0 VGG	3'R 3'R	2	5	6'			22	111	34'0"	S5	// "
	125	2	5	750	)" R5		25	11	55'3"	36	" "
Superstructure End Bent 1	125		5	7.0			22		19'6"	57	74 P
3 3 50 011	8 9	18	5	10:3			22	11/	13.2"	38	
2 4 /3'9" C/2	15-V65 CH 15	3	5	15			25 22	10	<u>58</u> :3"   18:9"	59	
2 6 10:9" (13 "	3R 1 3'R		1	T	1/2	4	22	10	32:6	5//	· · · · · · · · · · · · · · · · · · ·
25 5 5 C C/4 1 4	725	2	6	160	7/2	Wing	25	10	52:9	5/2	
8 9 37'0" HII Beam	*63	2	6	200-6	6 761		70	4	21:0	5/3	
8 9 37'0" HII Beam 3	C D F F F F F F F F F F F F F F F F F F	<b>-</b>		1			70	4	2/19	3/4	
1 6 56" HI3 Wing	75 34 6 36 HI	-, 70	6	11'-6	0///	Bearn	70		120°C	5/5	
1 6 12-6" 144 18	1/2 P21 V 221	10	17	8'9	1 1/2	Wine	35 70	4	128131 19191	5/6	" " "
3 7 15:5" H15 "	2/2 13 2" 2/8 F3/	75	4				10	+	13.5	3//	<del></del>
3 6 19'3" HIG "	212 16 8 1212 F51						1314	6	34'6"	<i>S77</i>	TOD 3/0
3 7. 16'0" H17 """"""""""""""""""""""""""""""""""	HI1-F31-F51			versti		<i>'-e</i>	160		34:6.	ST2	., .,
3 6 15'6" H18 " 18 6 8'3" H19 "	3 × GI-94	4	<u>5</u>	34'3			192	5	33'9"	573	
4 5 4'9" RI End Post	\$ \\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	12	5	32 C			192 192	5	35'0"	S75	,, ,,
2 5 5'6" 22 " "	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	2	5	33.0			192	5	35 0	3/5	<del></del>
2 5 G:3" R3 " "	00 B P21-V66	d	-5	3215		17 17 14 10 10 10	21	8	54:0"	SB/	Bott Sla
2 5 G 9" R4 " " " " 2 2 5 7"0" R5 " " " " " " " " " " " " " " " " " "		12	5	30-6	166	<b>.</b>	20	9	50'0"	582	,, ,,
2 5 7:0" R5	W/-W2-W3 /2-	6	6	28:0		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	18	9	33:9"	383	,, ,,
16 5 5.3" R7 Parapet		187	6	3/9		-	21	9	65°C''	584	
4 5 13'3" R8 "		387	5) 6	86		Girder 0	20. 18	10	59'3" 44'9"	SB3	n n
4 5 10:3 R15 "	MARK A B C MITTI	1181	6	8:6		" "	21	18	68:0"	58G 887	- " · · · · · · · · · · · · · · · · · ·
	R2 2'4" 75 85" V)	1197	خ	8:6	″ <i>G</i> 3	4	20	10	60°. Ci"	388	, e
2 6 5 711 Ning	R3 2:51 72" 82"	403	Œ	86	" G4	2.6	18	10	44'3"	589	
3 6 60 712	R4 2'82 72 82 82 82 82 82 82 82 82 82 82 82 82 82	12	ප	46'0			21	8	53.6	5BIO	,, ,,
72 6 11-5" UII Beam	R5 2' 10" 7½" 8½" 4'5" 69	6	8	42:3	" G12	- "	20	8	5/:0"	SBII	
	R7 2:0 72 72 8	45	4	271	" G/4	- W	13	10 8	43'5 51'6'	SB12 3813	z
(3 4 8'9" VII Wing	P2-P3-P4 P5-P7 P2 1101	16	4	316	19/9	1000	20	9		5514	· · · · · ·
0 4 89" 112 "	13.6° 23° 77/						18	9	34'3"	58/5	. a
per structure Int. Bit. 2.3.485	10' 2'1" 78	12	8	46.9			21	8	57:9"	SBIG	
68 145 34'6" H21 Bearn	2:9" UII 6:25 6:25 [4:10" F2	<u>6</u>	8	42' 9	" G23	. 9	21	8	30:3"	SB/7	" "
12 145 11 112	14:10° F201	10 16	4	27:3 3/-9	" 324	# 1) ji	21	8		SB:8	
4 6 23 6' H23 "	To the second of	12			" G51		21 363	8	2813" 2913"	5 <i>819</i> 3820	
28 7 29-5 H24 "	0000	6	8	43:3	333		40	6	5'9"	30201 3021	
58 0 10:54 00:	W5 000 KIXA	40	4	28:3	" G34	"					
58 4 12:3" P21 Column		16	0		036	" .					
1 6 13-3 USI Beam	UII- F21 TII-TIZ-T61	12	8	47:9			126			WI	Diogri
- Jan 3. Jan Je J	· · · · · · · · · · · · · · · · · · ·	6	8	43: 3	' G42		126	4		Will	
. 1, 36'5 Y21 Column	132 7-3	40	9	28:6	" G44		126	4		W5	<del></del>
4 3 35 V3/			1	32'9			84		12'6	W5	,
9 277 1/2 "			1				42		a:3"	NG	
32 9 35:5" 75/	HI9 WIND AND THE MINE THE PARTY OF THE PARTY	$ \Box$									
	14248	1	1								
	CIA WG GG"										

FED ROAD STATE FED AID FISCAL SHEET TOTAL DIST. MO. FRUJ NO. YEAR NO. SIEETS

MO. 19 72

FINAL PLANS

ESTIMATED QUANTITIES								
ITEM	SUBSTR.	SUPERSTR.	TOTAL					
Closs ! Excavation for Structures Culds		I	403.5					
teel Pries in Place (10", Lin Ft.	2.56		2.56					
Steel Pile in Piuce (12) Lin Ft.	177		177 /					
Class B Concrete Cu. Yus.	89.8	'	89.8					
Class Bl Concrete Cu. Vds		1083.9	1,083.9					
Reinforcing Precl 255.	9.5.0	364,610	373,970					
Fobricated Structural Corbon Steel Los		900	900					
Bridge Rail (Single Tube Type) Lin. Ft.		817	817					
Drilled Test Hoizs	24		24					
Ein Steel Pile @ 50% of Bid Price			70					

Mote: Concrete in end posts, parapets and curbs is included with superstructure concrete:

All concrete and reinforcement above isotrops in Entermediate beings is included in superstructure quantities.

No payment for exercition will be allowed at End Berit No. 1.

#### GENERAL NOTES

Design Specifications: A. A. S. A.O. - 1961

Design Loading:

H320-94

15 sq.ft. Future Visaring Surface

Modified 24,000 Tandem fixle

Forth 120: Equivalent Fluid Pressure 30 to Design Unit Stresses:

Class B Concrete (substructure) for 1,200 psi.

Class Bi Concrete (superstructure) for 1,600 psi.

Reinforcing Steel for 20,000 psi.

Sizel File (A.S.T.M. A36-G2T) + b. 9,000 psi.

Superstructure deak to be surface seaked.

Painting: I will be surface seaked.

Painting: I will be done of a point of the sleared and painted one cost of red read in the sleared and painted one cost of red read in the sleared and painted one cost of red read in the slear, except mother of an access dears of side of the surface of the surfac

MALABORE

BRIDGE OVER RAMP! & RIGHT LANE I-29

STATE ROAD -INTERSTATE ROUTE 29

IN KANSAS CITY

PROJECT NO. I-29-16: (RTE. I-29) STA. 43+89.45 RAMP 4

PLATTE COUNTY

A-1688

Note: This drawing is not to scale. Follow dimensions

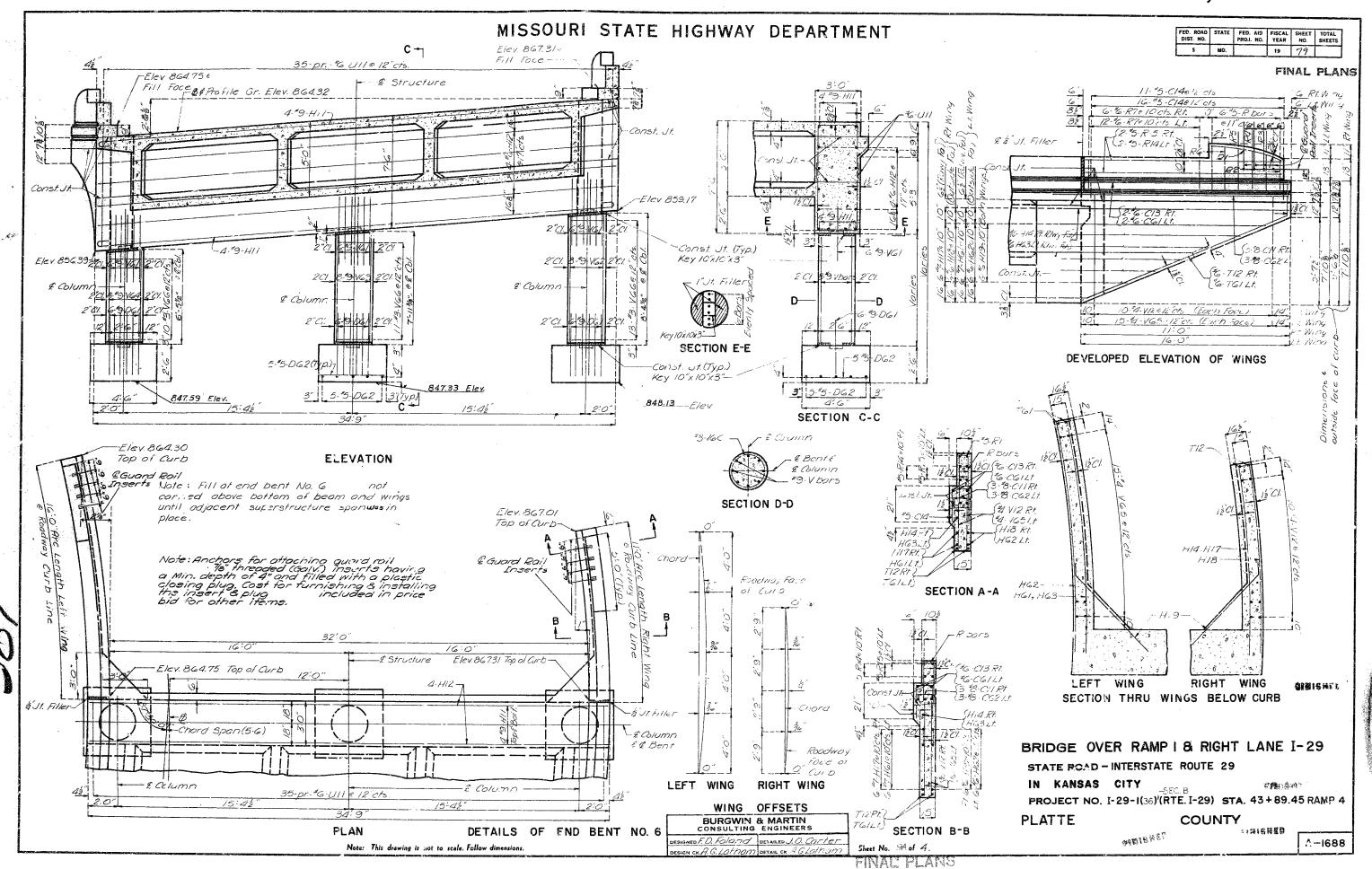
x May bend in field or shop.

\* Top bend may be made in field or shop.

Sheet A

Note: This drawing is not to scale. Follow dimensions

Sheet No. 5A of 🕿. A



# TITLE SHEET SEE

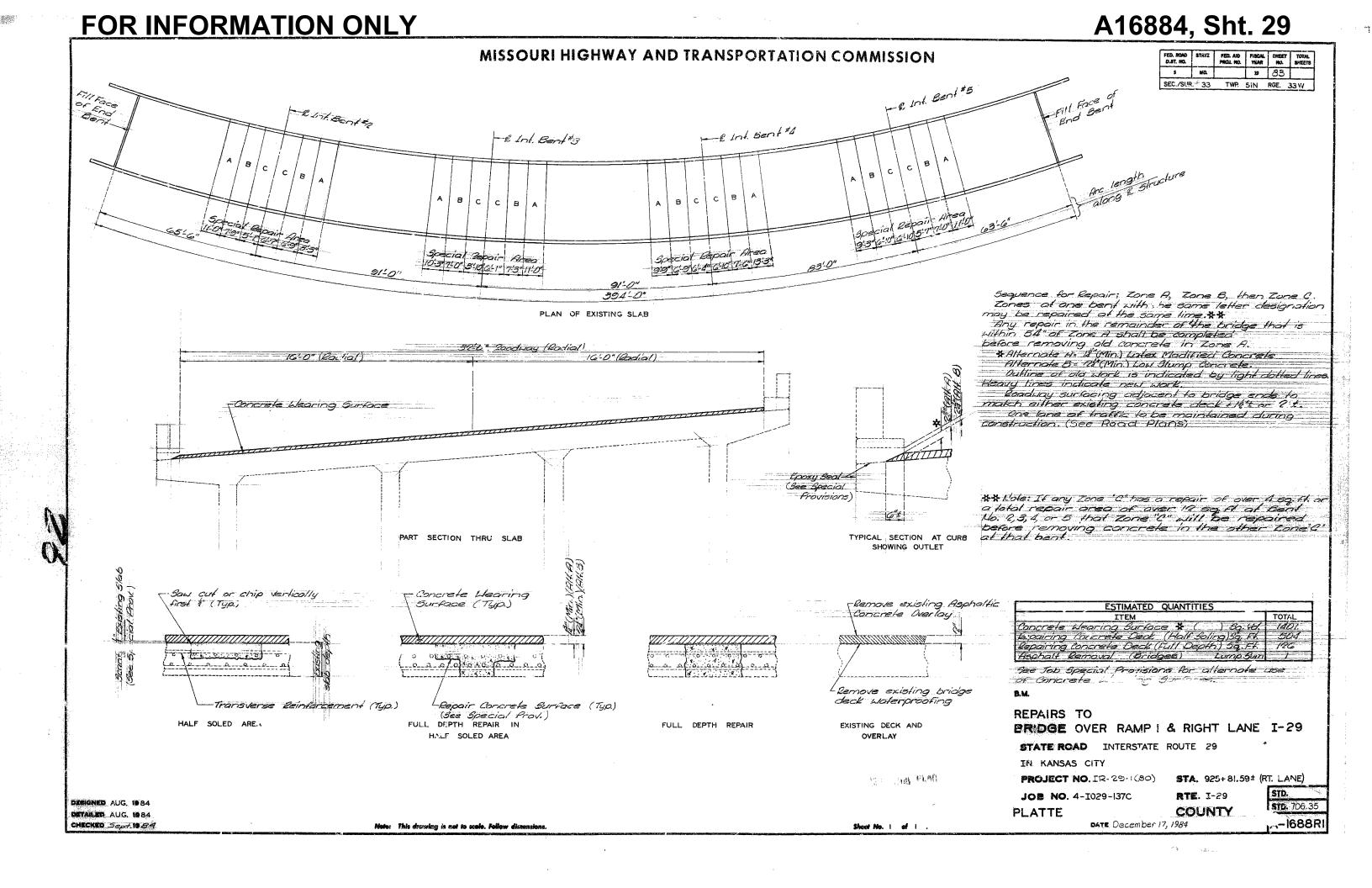
PLATTE A-1687R

4-I-29-137

300

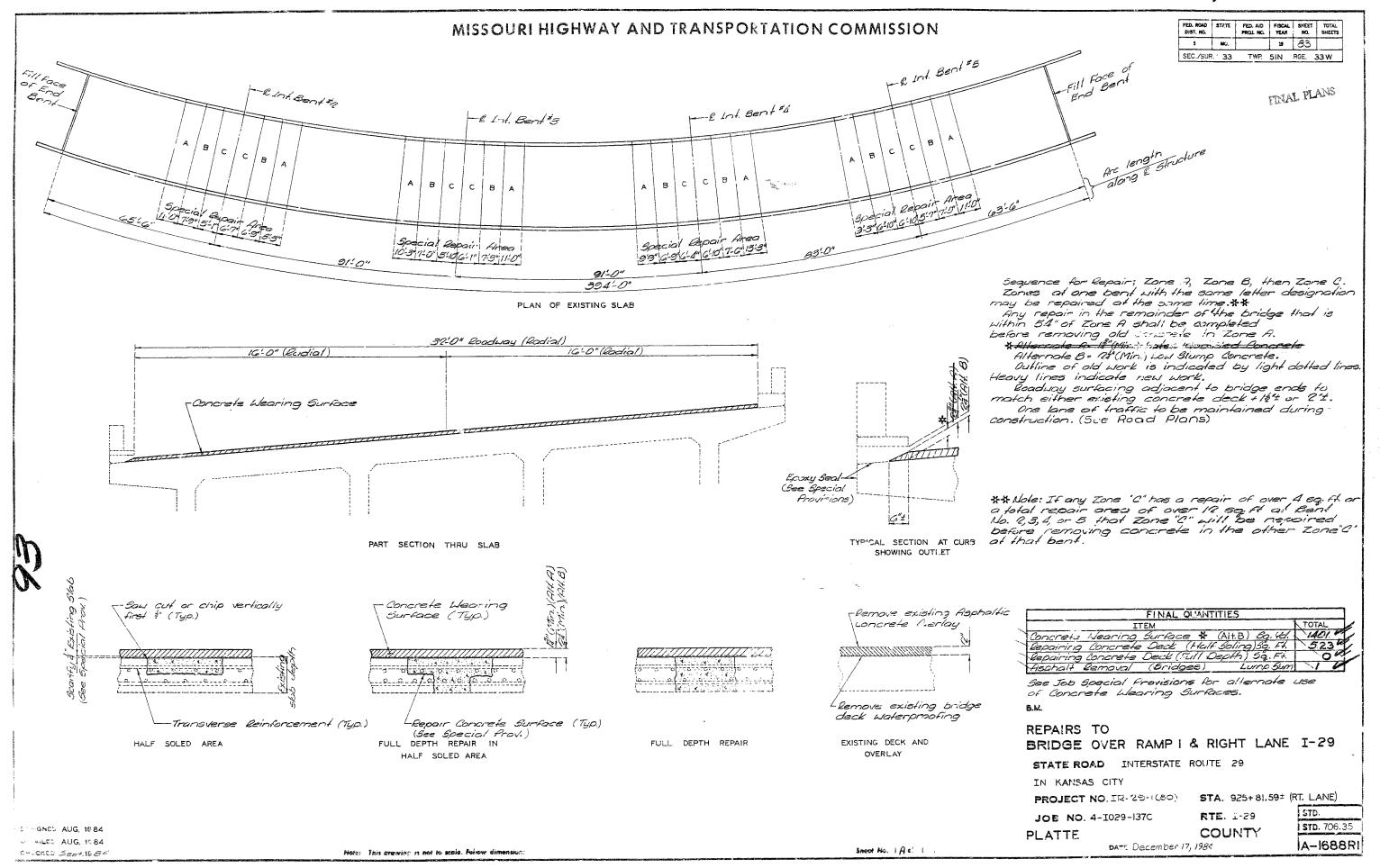
A-1688R

1 sf



# F FOR INFORMATION ONLY

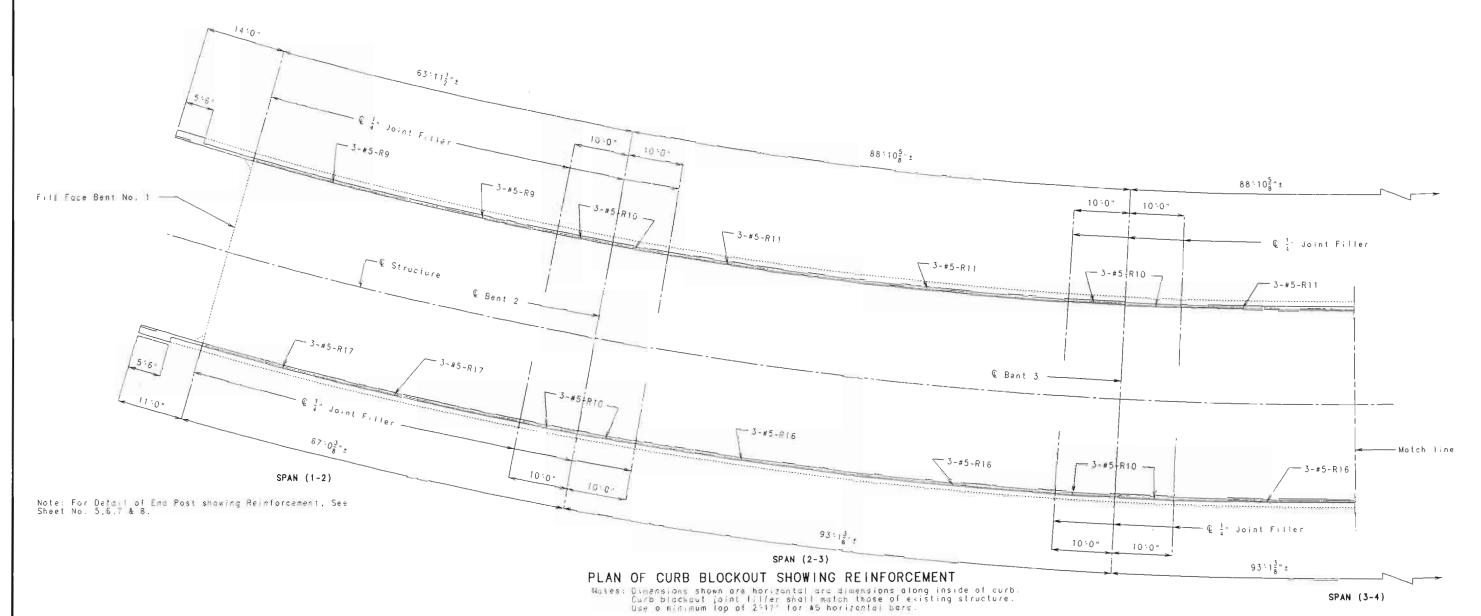
# A16884, Sht. 30



FOR INFORMATION ONLY

MISSOURI HIGHWAY AND TRANSPORTATION COMMISSION

SEC./SUR. 33 TWP, 51N RGE. 33W



#### NOTES FOR CURB BLOCKOUT

NOTES FOR CURB BLOCKOUT

Concrete in curb blockout shall be B1.

Meosurement of curb blockout is to the nearest finear foot measured at the gutter line from end of wing to end of wing.

All exposed edges at curb blockout shall have 1/2" radius or 3/8" bevel unless atherwise shown.

Payment for concrete and reinforcing steel in curb blockout complete in place shall be included in the contract unit price for the curb blockout per linear foot.

Cost of any concrete end past removal shall be considered completely covered in anit prices bid for curb blockout.

Embedment depth of resin anchor systems (vertical and horizontal) shall be a maximum of 6" into existing curb & paraget.

Adjust resin anchors in field, if necessary, to miss curb outlets.

	ESTIMATED QUANTI	TIES	
	/iTEM		TOTAL
CURB BLOCKOUT		LINGET.	840
			T
			1
			1

DESIGNED May 1997 DETAILED May 1997 CHECKED Oct. 1997

Bars bonded in old concrete not removed shall be clearly stripped and embedded into new concrete where possible. If length is ovariable, old bars shall extend into new concrete at least 40 diameters for smooth bars and 30 diameters for deformed bars, unless otherwise noted.

The contractor shall use one of the resin archar systems listed in the job special provisions for the curb blockout. These anchar systems shall be installed according to the manufacturer's special provisions, except as modified by the job special provisions and that an epoxy coated #5 grade 60 reinforcing bar as shown shall be substituted for the \$78 a threaded rod stud.

Cost of furnishing and installing the anchor systems complete in place shall be insluded in the price bid per linear foot of curb blockout

The 5/8" diameter resin anchor systems shall have a minimum ultimate pullout strength of 18.800 lbs. in concrete with fix = 4000 ps. . See special provisions. NOTE: THIS DRAWING IS NOT TO SCALE . FOLLOW DIMENSIONS

## GENERAL NOTES: DESIGN SPECIFICATIONS:

A.A.S.H.T.O.-1996 DESIGN UNIT STRESSES: Class B1 Concrete (Curb Blockout) f's=+000 ps; Reinforcing Steef (Grade 60) fy=60,000 ps; (GINT FILLER: All ident filler shall meet the requirements of Std. Spec. 1057.2.4, except as mated.

REINFORCING STEEL: Minimum clearance to reintereing Steat shall be 1-1/2" unless otherwise shown. OLO WORK: Outline of old work is indicated by light dashed lines. Heavy lines indicate new work-VERIFT DIMENSIONS:

Contractor shall varify dimensions in field pefore ordering new materials look new work. TRAFFIC HANDLING:

See roadway plans for traffic control suring construction.

SHEET NO. 1 OF 9



REPAIR TO: BRIDGE OVER RAMP 1 & RT. LANE I-29

STATE ROAD AT RTE. 1-29 & 1-635 INTERCHANGE

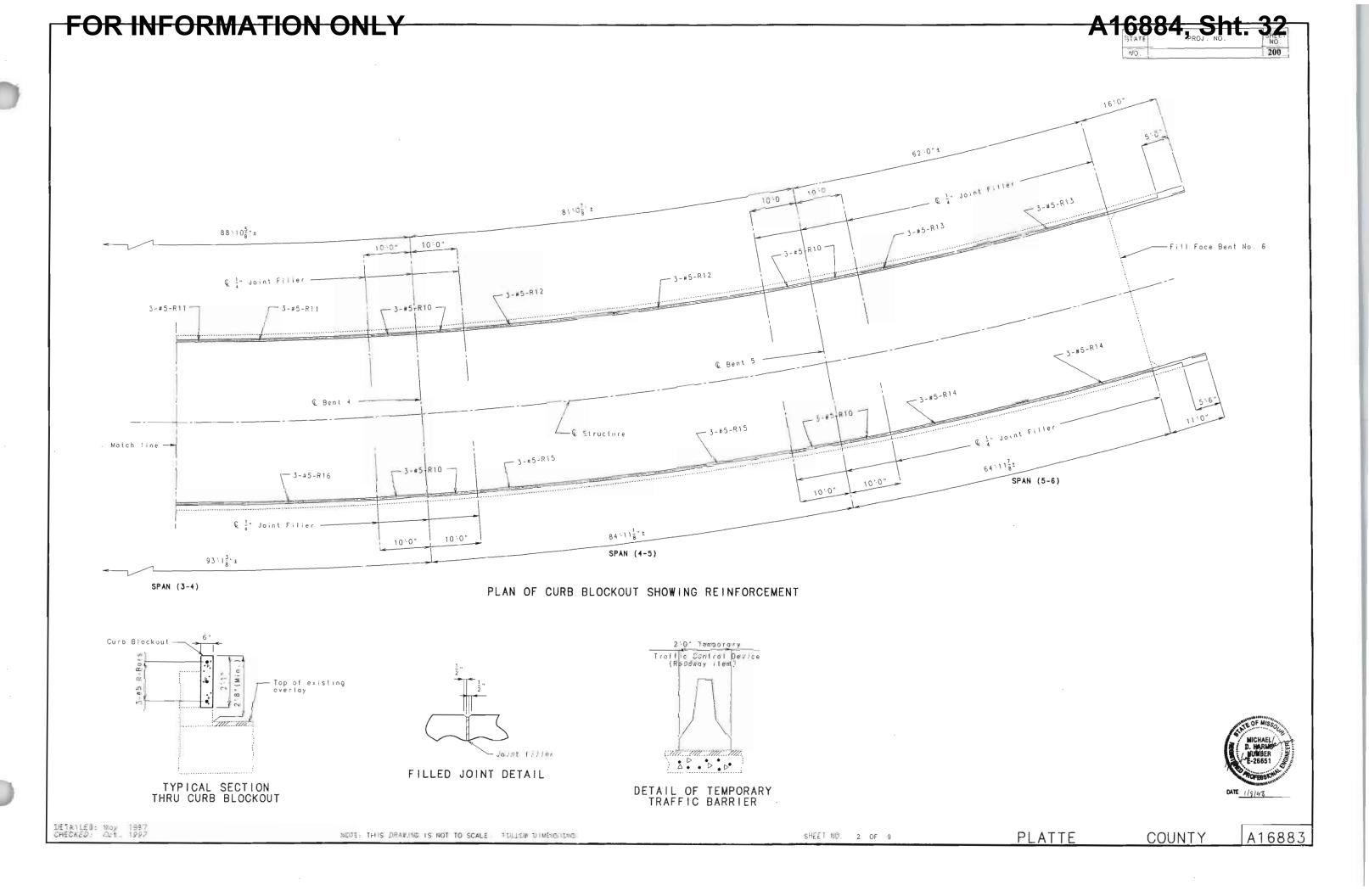
PROJECT NO. JOB NO. J411246 STA. 43+89.45 (MATCH EXIST RTE. RAMP 1-29

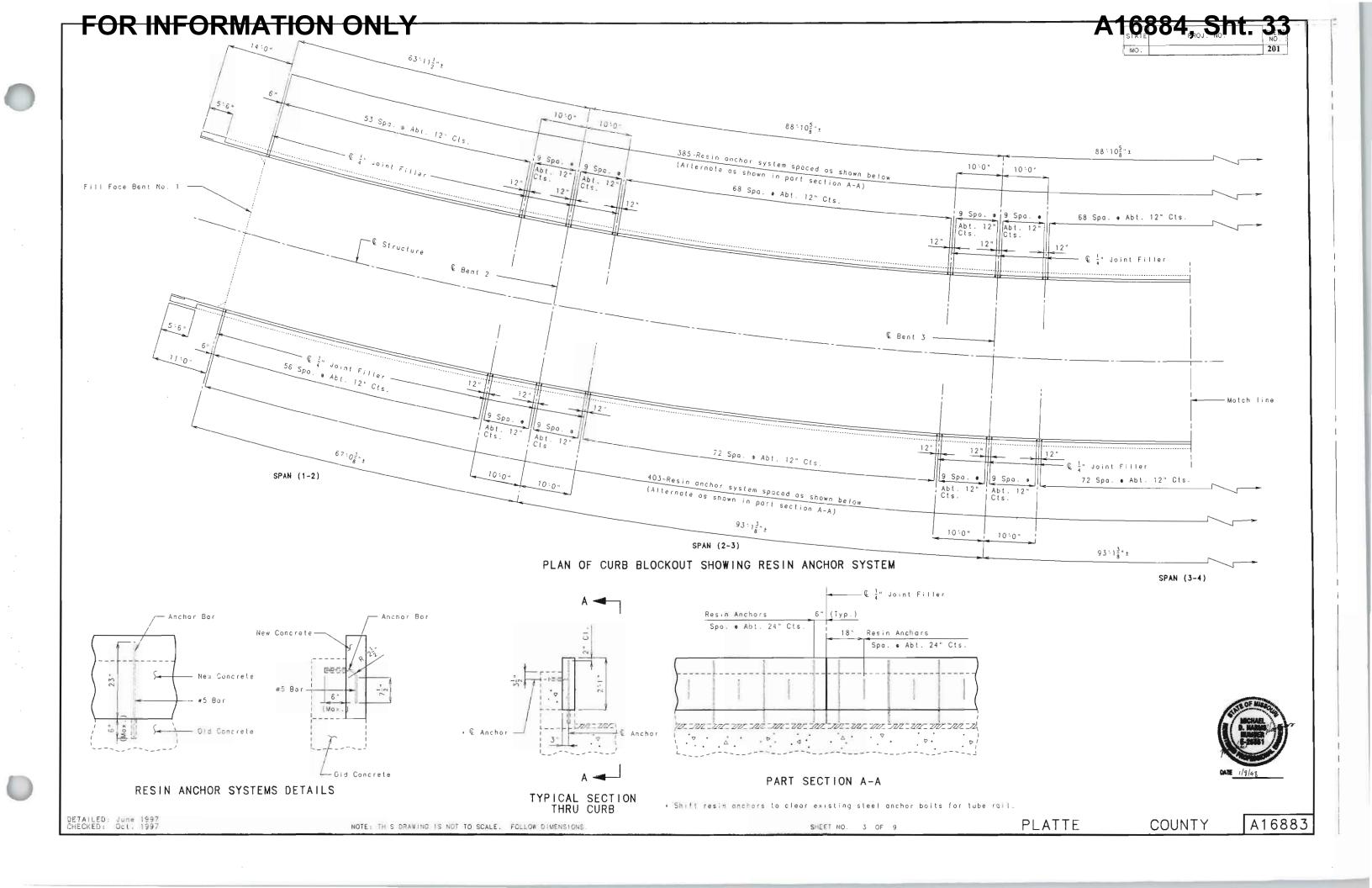
PLATTE

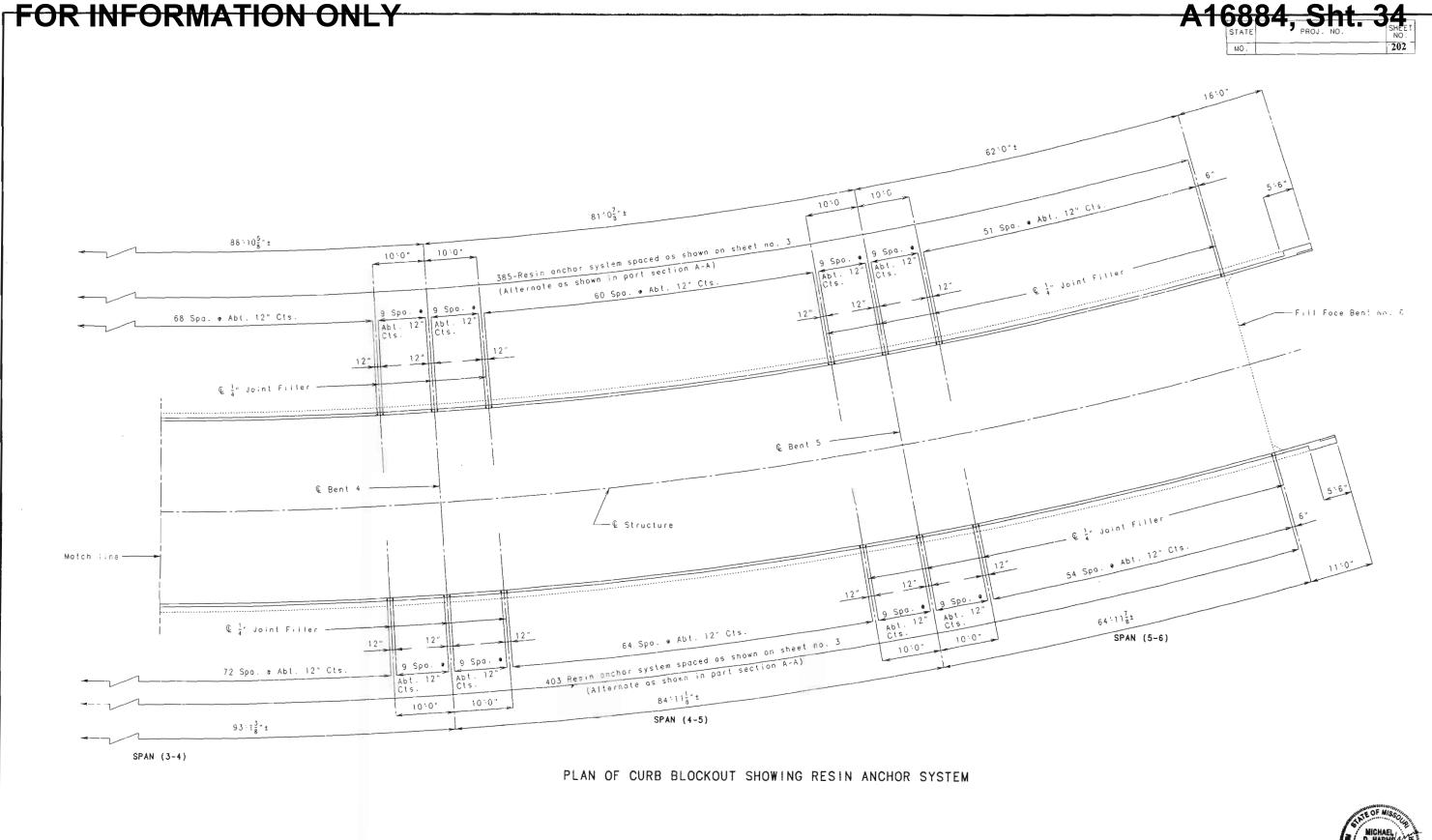
COUNTY JATE 1/27/98

STD. A16883

STD.









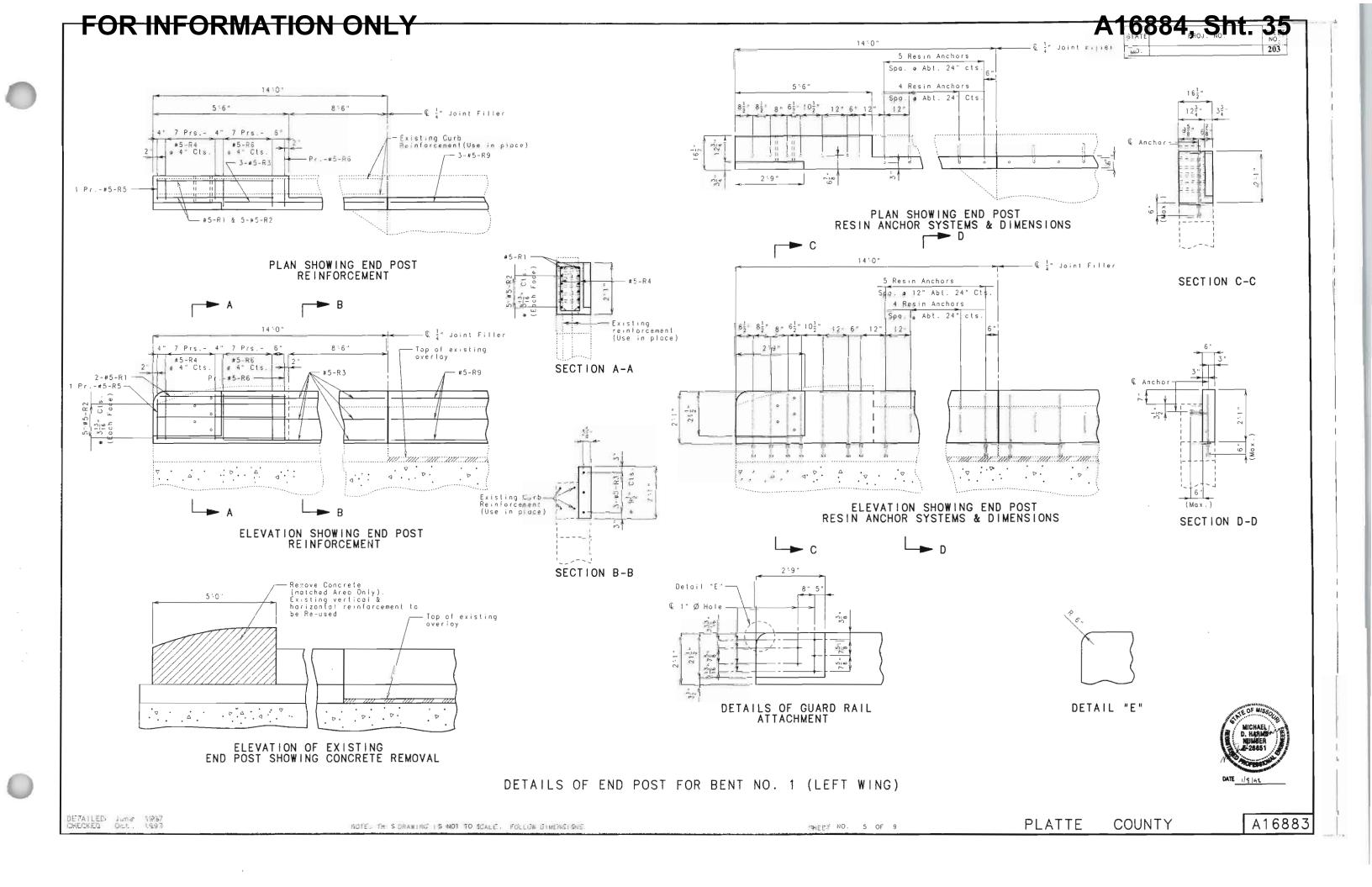
DETAILED: June 1997 CHECKED: Oct. 1997

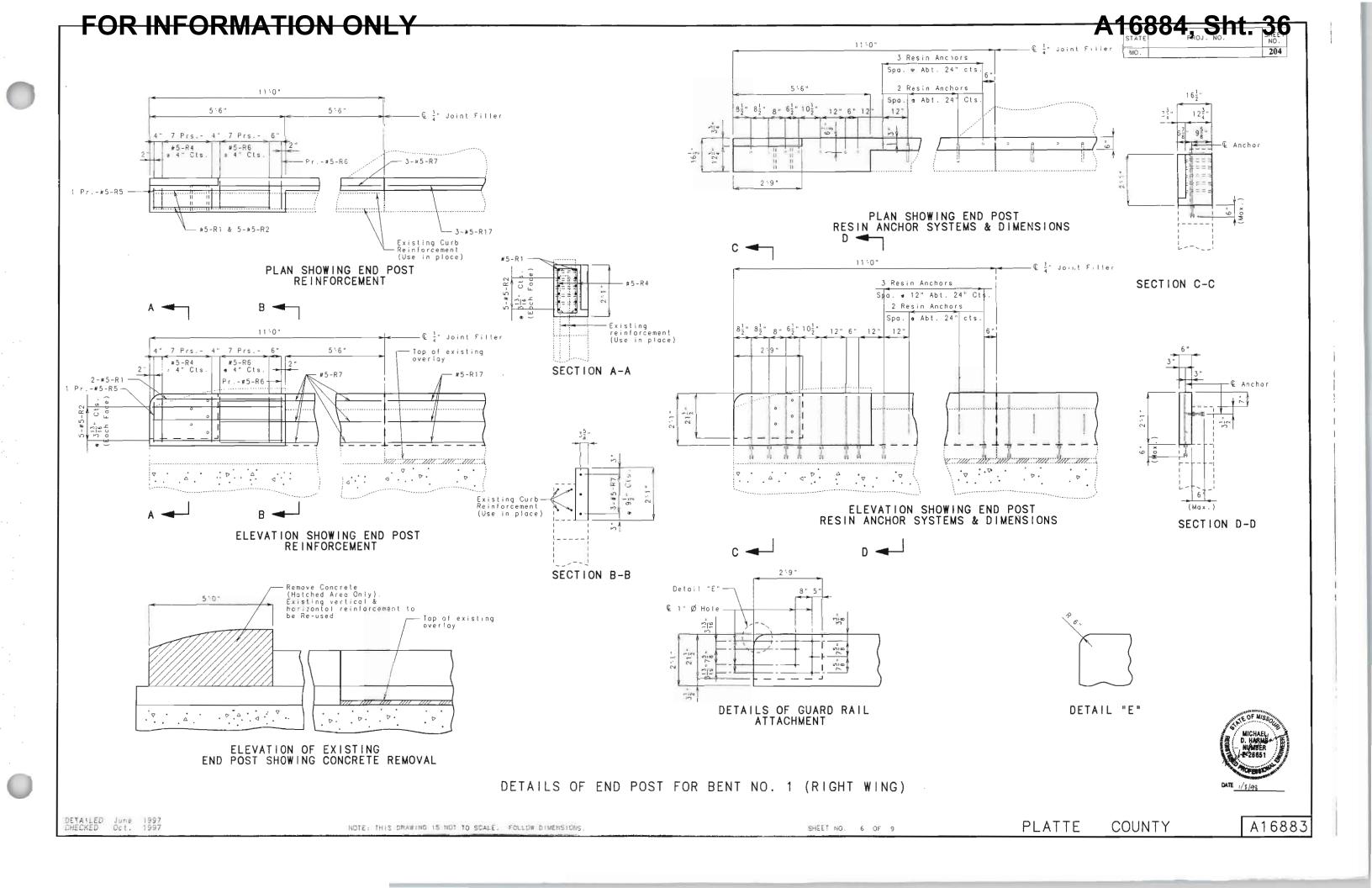
NOTE: THIS DRAWING IS NOT TO SCALE. FOLLOW DIMENSIONS.

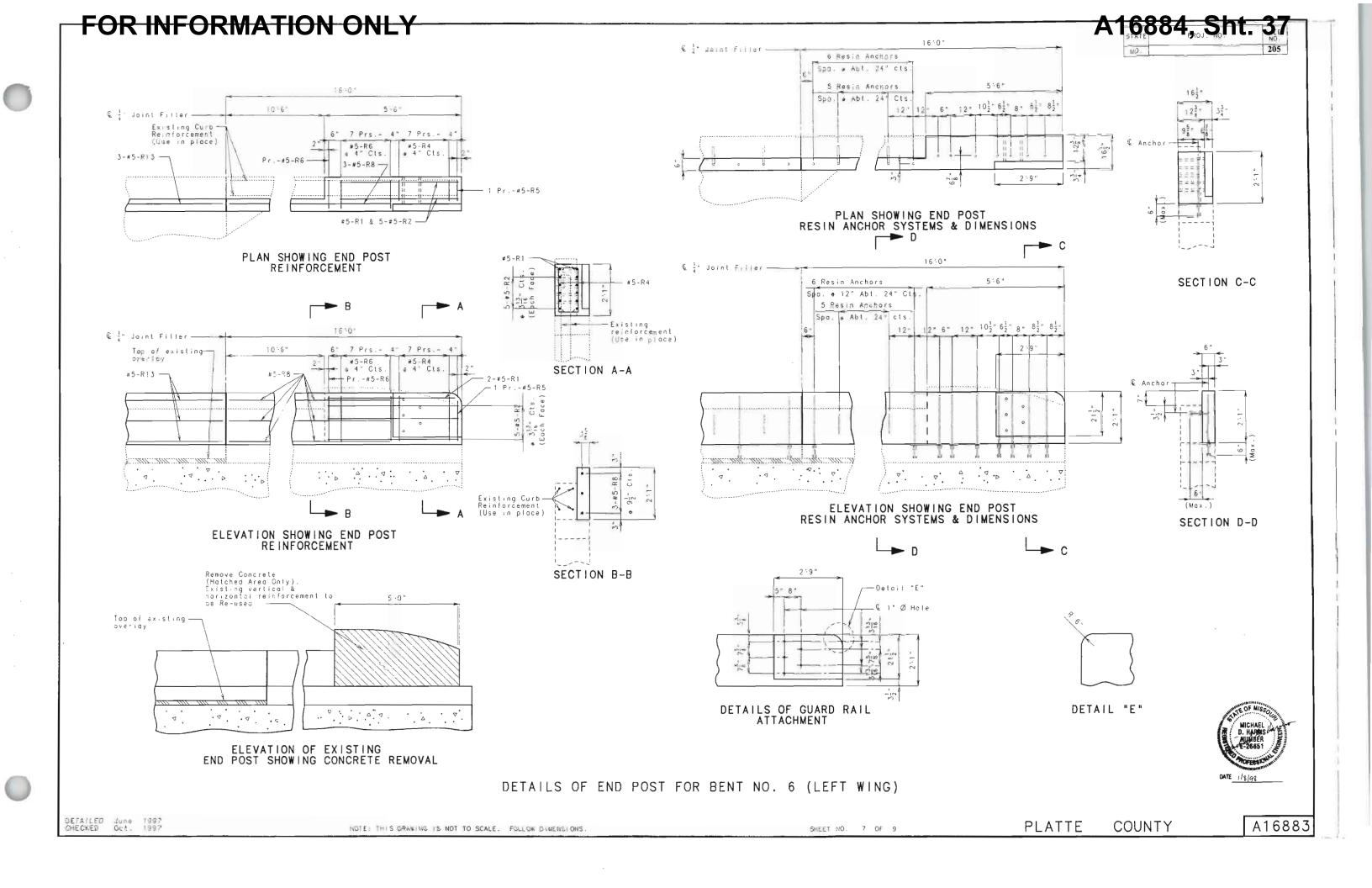
SHEET NO. 4 OF 9 PLATTE

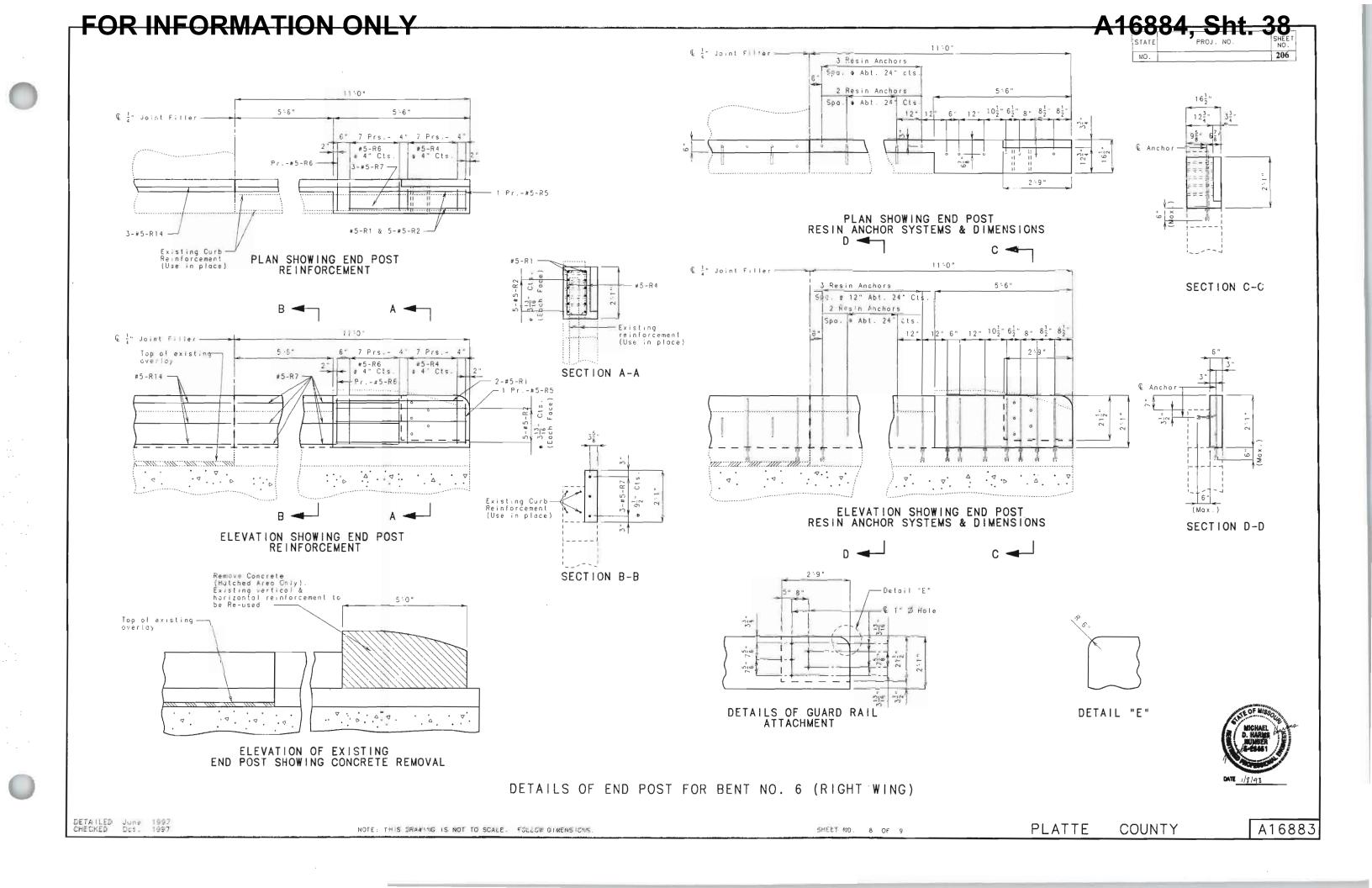
COUNTY

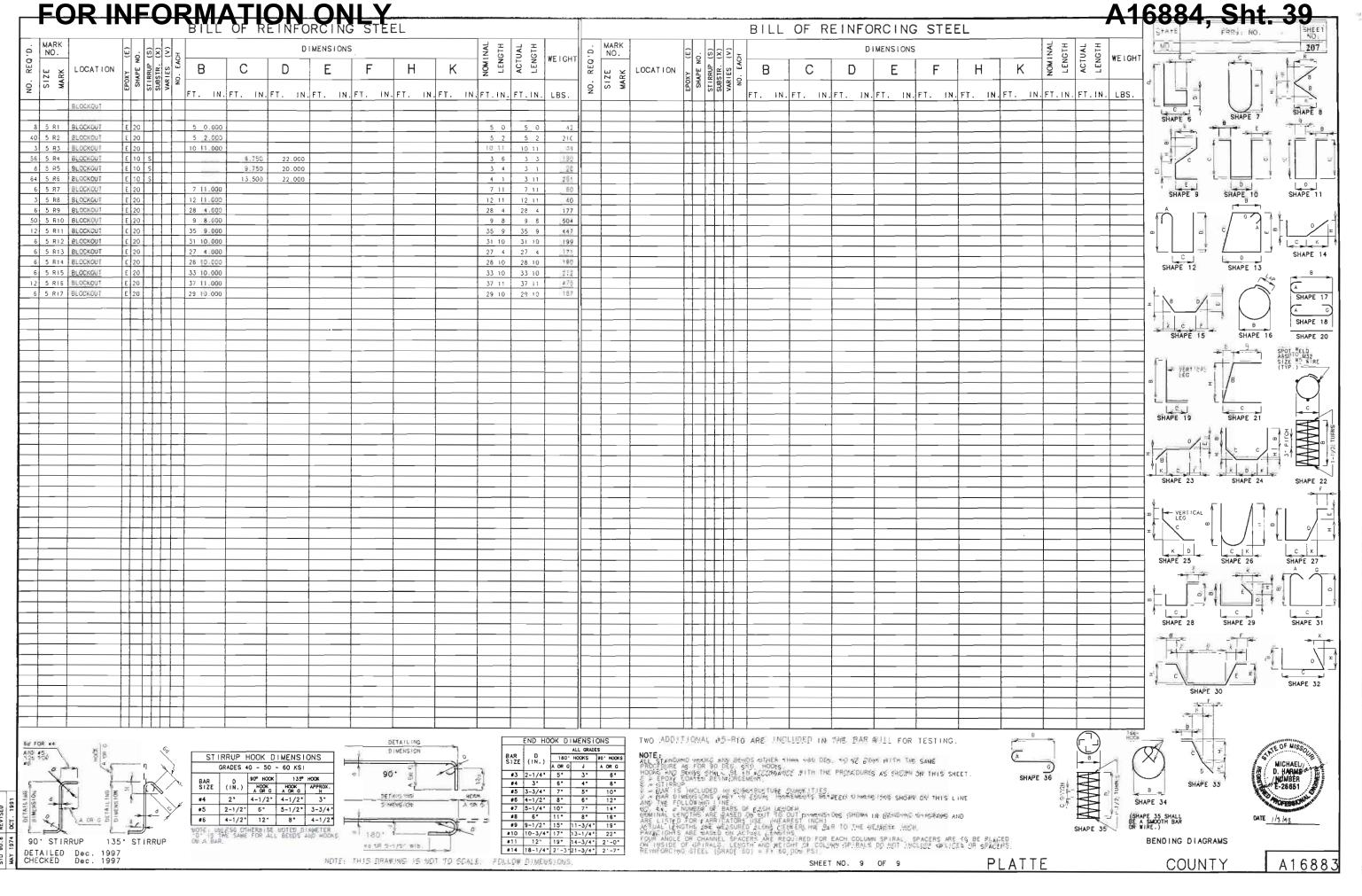
A16883











MISSOURI HIGHWAY AND TRANSPORTATION COMMISSION ACIM-ACIMG-29-1(99) 199 SEC./SUR. 33 TWP. 51N RGE. 33W Contract ID. 980424-09-OVK certify that this plan sheet accur-onfiguration and location of the 14:0" 63 - 11 1 " ± i" Joint Filler — 10:0" 10:0" 88'-10<sup>5</sup>"± 88'10<sup>5</sup>"± 3-#5-R9 Fill Face Bent No. 1 -10'0" 10'0" -3-#5-R11  $\mathbb{Q} \frac{1}{4}$ " Joint Filler -@ Structure 7-3-#5-R11 7-3-#5 R10 \_ 3-#5-R11 © Bent 2 — 3-#5-R17 -3-#5-R17 5:6" € Bent 3 ---Joint Filler. -3-#5-R16 67:03"± -Match line 3-#5-R16 3-#5+R10 3-#5-R16 SPAN (1-2) 10:0" Note: For Detail of End Post showing Reinforcement, See Sheet No. 5,6,7 & 8. · 🖟 🕯 "Joint Filler 93:13"± 10'0" 10'0"

#### NOTES FOR CURB BLOCKOUT

Concrete in curb blockout shall be B1.

Measurement of curb blockout is to the nearest linear foot measured at the gutter line from end of wing to end of wing.

All exposed edges of curb blockout shall have 1/2" radius or 3/8" bevel unless otherwise shown.

Payment for concrete and reinforcing steel in curb blockout complete in place shall be included in the contract unit price for the curb blockout per linear foot.

Cost of any concrete end post removal shall be considered completely covered in unit prices bid for curb blockout.

Embedment depth of resin anchor systems (vertical and horizontal) shall be a maximum of 6" into existing curb & parapet.

Adjust resin anchors in field, if necessary, to miss curb outlets.

FOR INFORMATION ONLY

	FINAL	QUANTITIES		
	ITEM			TOTAL
CURB BLOCKOUT			LIN.FT.	`840 1
	CURB BLOCKOUT	ITEM		ITEM

NOTES:

Bars bonded in old concrete not removed shall be clearly stripped and embedded into new concrete where possible. If length is available, old bars shall extend into new concrete at least 40 diameters for smooth bars and 30 diameters for deformed bars, unless otherwise noted.

The contractor shall use one of the resin anchor systems listed in the job special provisions for the curb blockout. These anchor systems shall be installed according to the manufacturer's specifications, except as modified by the job special provisions and that an epoxy coated #5 grade 60 reinforcing bar as shown shall be substituted for the 5/8 % threaded rod stud.

Cost of furnishing and installing the anchor systems complete in place shall be included in the price bid per linear foot of curb blockout.

The 5/8" dicmeter resin anchor systems shall have a minimum ultimate pullout strength of 18,800 lbs. in concrete with f'c = 4000 si. See special provisions.

## GENERAL NOTES:

SPAN (2-3)

Notes: Dimensions shown are horizontal arc dimensions along inside of curb. Curb blockout joint filler shall match those of existing structure. Use a minimum lap of 2'11" for #5 horizontal bars.

PLAN OF CURB BLOCKOUT SHOWING REINFORCEMENT

DESIGN SPECIFICATIONS:
A.A.S.H.T.O.-1996
DESIGN UNIT STRESSES:
Class B1 Concrete (Curb Blockout) f'c=4000 psi
Reinforcing Steel (Grade 60) fy=60,000 psi
JOINT FILLER:
All joint filler shall meet the requirements
of Std. Spec. 1057.2.4, except as noted.
REINFORCING STEEL:

Minimum clearance to reinforcing steel shall be 1-1/2" unless otherwise shown.

OLD WORK:

Outline of old work is indicated by light dashed lines. Heavy lines indicate new work.

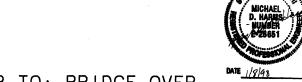
VERIFY DIMENSIONS:

Contractor shall verify dimensions in field before ordering new materials.icate new work.

TRAFFIC HANDLING:

See roadway plans for traffic control during construction.

SHEET NO. 1 OF 9



SPAN (3-4)

REPAIR TO: BRIDGE OVER RAMP 1 & RT. LANE 1-29

STATE ROAD AT RTE. 1-29 & 1-635 INTERCHANGE

93'-18"±

PROJECT NO.
JOB NO. J411246
PLATTE

STA. 43+89.45(MATCH

RTE. RAMP 1-29 STD.

COUNTY

DATE 1/27/98

std.
A16883

DESIGNED May 1997 DETAILED May 1997 CHECKED Oct. 1997

NOTE: THIS DRAWING IS NOT TO SCALE. FOLLOW DIMENSIONS