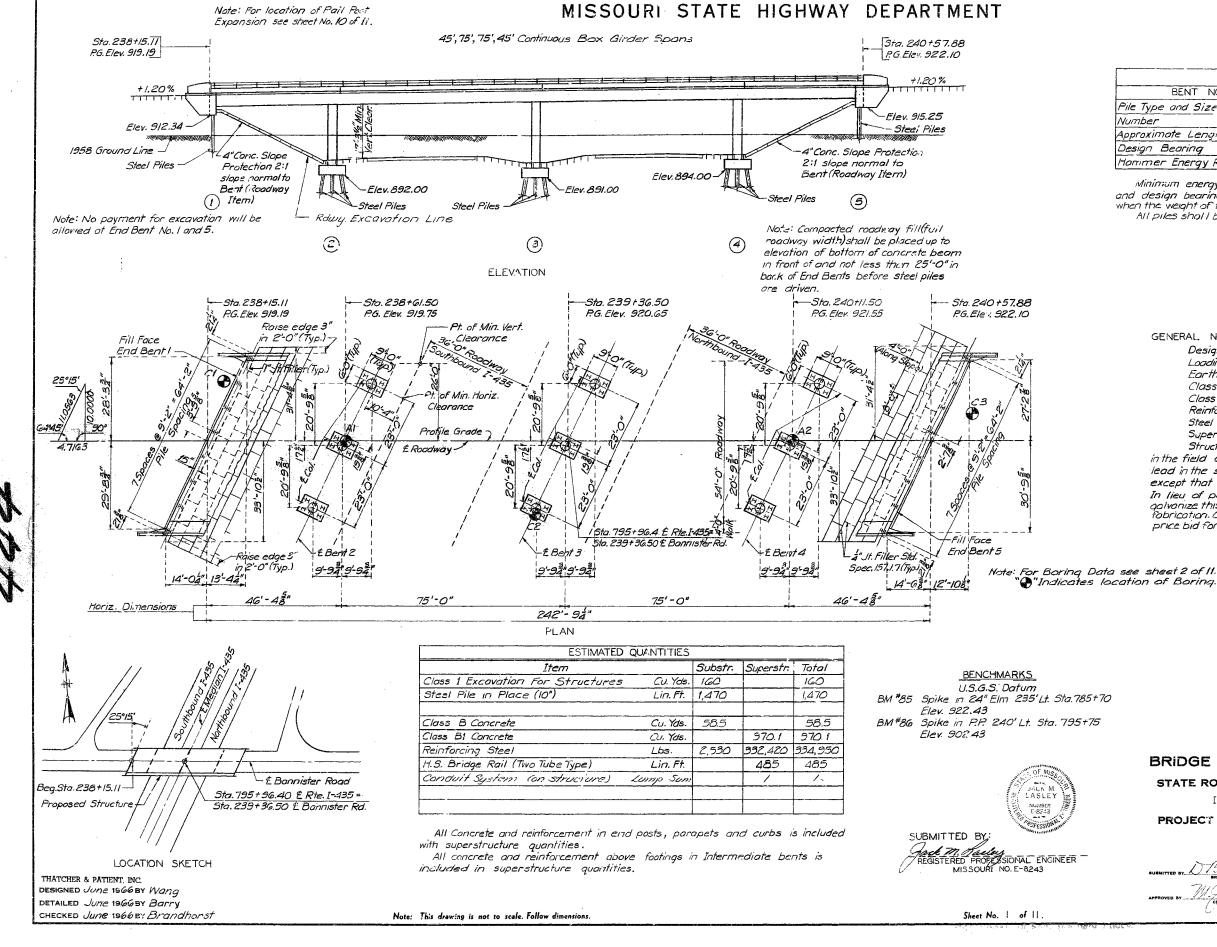
K



Minimum energy requirement of hammer based on plan length and design bearing value of piles. Increase by the factor  $(W_{tW})/2W$ when the weight of the rom (W) is less than the weight of the pile (w). All piles sholl be driven to procticol refusal

## A16433, Sht. 1

CED DOAD	67.475				,
FED. ROAD DIST, NO.	STATE	PROJ. NO.			SHEETS
5	MO.		19	146	

	PILE	DATA				
BENT NO.		1	2	3	4	5
le Type and Size		10BP42	CBP42	108P42	IOBP42	IOBP42
umber		8	18	18	18	8
proximate Length	<i>.</i> F <del>7</del> .	32	12	22	17	37
esign Bearing	Tons	.38	54	56	54	38
ommer Energy Required	Ft.Lbs	8,600	12,700	13,200	12,700	8,600

GENERAL NOTES: Design Specifications A.A.S.H.G.~ 1961 Loading HS2C-44 (15 \*!sq.ft. Future Wearing Surface) Earth120\*, Equivalent Fluid Pressure 30\* Class B Concrete (substructure) fc = 1,200 psi Class BI Concrete (superstructure) fc=1,600 psi Reinforcing Steel fs = 20,000 psi Steel Pile (A.S.T.M. A36-63T) fb = 9,000 psi Superstructure deck to be surface sealed. Structural Steel access doors shall be cleaned and painted in the field or may be cleaned and painted one coat of red lead in the shop with the two remaining coats applied in the field. except that final coat on access doors and frames shall be gray. In lieu of painting, the contractor may, if he prefers, galvanize this material. All galvanizing shall be done often fabrication. Cost of painting or galvanizing to be included in

price bid for other items.

3. . . . 3. . . . r = - 4 BRIDGE BANNISTER ROAD UNDERPASS STATE ROAD INTERSTATE ROUTE 435 IN KANSAS CITY PROJECT NO.1-IG-435-1(58)(RTE.I-435) STA. 795+96.4 COUNTY JACKSON DATE 12/6/26 factures STD. 54.00 CHIEF ENGINER DATE 12/3/22 A-1643

dia a

1

COMPLETE BILL OF REINFORCING STEEL BENDING SKETCHES & CUTTING DIAGRAMS NO. SIZE LENGTH MARK LOCATION NO. SIZE LENGTH MARK LOCATION BENDING SKETCHES & CUTTING DIAGRAMS NO. SIZE LENGTHMARM LOCATION INTERMEDIATE BENT NO4(Cont END PENT NO SUPERSTRUCTURE (Cont.) 32 "8 55'3" (GB Girders 48 14 36'9" (G9 " 8 5 56'3" AB1 4 78 31-6" AB2 4 78 40'9" AB3 6 "10 16-0" PB4 Beam 11 "10 600" PB5 ' 4 "6 31-0" PB1 " 154 % 12-3" L/2 " 2-98 2-114 6-6-4" 34'-68' ABI 29-98" ABZ <u>6-23</u> Beom 42" 13% 6-878 <u>39'07'</u> AB3 19'97' AB3 19'97' PB1 14'978' PB3 19'67' PB5 4 <sup>18</sup> 40<sup>.9</sup>" AB3 16 <sup>1</sup>6 34-9" AB4 134 <sup>15</sup> 10<sup>.3</sup>" []] 220 14 4'G" WI Diophrogms 8 15 299" WE " 11 8 76 286 W3 16 76 17-9 W4 END BENT NO.5 8 "8 36-3 ABI Beom 4 "8 31-6 ABE " 4 "8 40-9" AB3 " 16 16 34-9" AB3 " 16 16 34-9" AB4 " 134 "5 10-3" UI " ı ı 4 #4 9'0" VI Wings 10 #4 6'9" V2 " 6'6'4" 2'114 9'6" - 6'-2<del>'</del>4" - <u>6'-9</u>4" 9-0" ATTE: 2 PCI 4VI CUT 8 4V3 CUT 8 241 "5 G-3" CI CUMDS 13" 11 6-94" 10-24" 0 "4 25-0" C2 20 "4 38-0" C3 10 "4 38-0" C3 10 "4 23-9" C4 224 "5 4-9" C5 6 "6 24-0" C6 11 \_\_\_\_\_\_\_%" ABI AB2, AB3 11 FBI, PB3, PS5. H 3-28 2-52"UI 8-4" F2 ıţ. 2 % 14-3' TI 2 % 14-6" T2 11 3-10211 Ħ 2-48 10-34" 6-1134 11 INTERMEDIATE BENT NO.2 27 "7 8'9" FI Footings G "7 15'G" F2 " 18 "5 5'9" F3 " 24 "5 2'G" DI " U2 <u> 17-3"</u> 5H2 CUT IO 5H3, CUT 10 UCBG2 بے تحصیل UI,F2 46 3 13 9'6" PCI Columns ▲16 110 23'3' PC2 " 3p Ni šŗ. 3-84" 2-113 10 GB TT 2019" SI Boltom Sub "D 22-9" PC3 S.p 4 "5 8'3" R7 4 "5 8'3" R8 12 "5 8'3" R8 \* 8 # 11 п 8 11 21-6 PBI Beom à, 1000 12'4" 12/8GZ 4 "11 25'9" F62 12 "11 16'6" F89 6 "10 16'6" F84 11 GI 1 G2 11 1. 10 CO'0" PB8 4 16 31-0" PB1 154 16 12-3" UZ 12'8" 6-02"TI 12" 3-12" 4-44 6-178"TE 1: Sto.2: 22 " st TI, T2 .08" . 11 nr. 11 INTERMEDIATE BENT NO.3 38'0' RI7 27-9' RI8 10-6" RI9 INTERMEDIATE BENT NO 3 2.7 "7 8'-9" F1 EDITIOS C "7 15-C" F2 " 18 "5 5'-9" F3 " 24 "5 2'-6" D1 " C 10 25-3" FC1 COLUMNS 1C "10 25-3" FC4 " 8 "10 22-9" FC5 " 11 4 5 11 13 Blass #5 Clay Fill 1G 
 6
 11
 503
 50

 70
 "9
 596"
 514

 6
 "11
 596"
 514

 5
 "6
 216"
 515

 55
 "6
 216"
 516

 272
 "5
 28.9"
 517

 272
 "5
 33.0"
 518

 32
 "6
 5.9"
 59
 WI 8 900 4 "5 34-3" R20 [2]3 "5 5-0" R21 G3 Silty Cloy-28'9" W2 15-10" 27'6" W3 890 Scrt.weather-Ton SIND AN AMA 4 "11 CS 5 PDC 8 "10 C1-5" PB5 12 "145 15-3" PBG G "11 Co" PBT 5 "11 Co" PBT 23355 2-058C10 880 Interbaded Umestone <sup>c</sup> Shole Silty Shole 870 <u>pronn</u> 4-13 (9 11: # / 11 Q 132 1980 92 5 "11 GOO" PB3 4 "43 GOO" PB1 4 "6 31-0" PB1 154 °G 12-3" UZ 11 13C5 13C10 52C9 11 . 11 C1,C9 11 72 C5,C10 " 14 860 INTERMEDIATE BENT NO.4 27 \*7 8'9" FI Footings 6 \*7 15'6" FZ " 18 \*5 5'9" F3 " 24 \*5 2'6" D1 " 11 11 (Core 11 " 35) #11 32'6" 534 39 #11 17'6" 535 11 11 
 10
 13
 2"G"
 DI

 14
 "5
 2"G"
 DI

 610
 "
 "
 Columns

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 8
 "O
 25"9"
 RC2
 "

 \*
 16
 "O
 22"9"
 RC3
 "

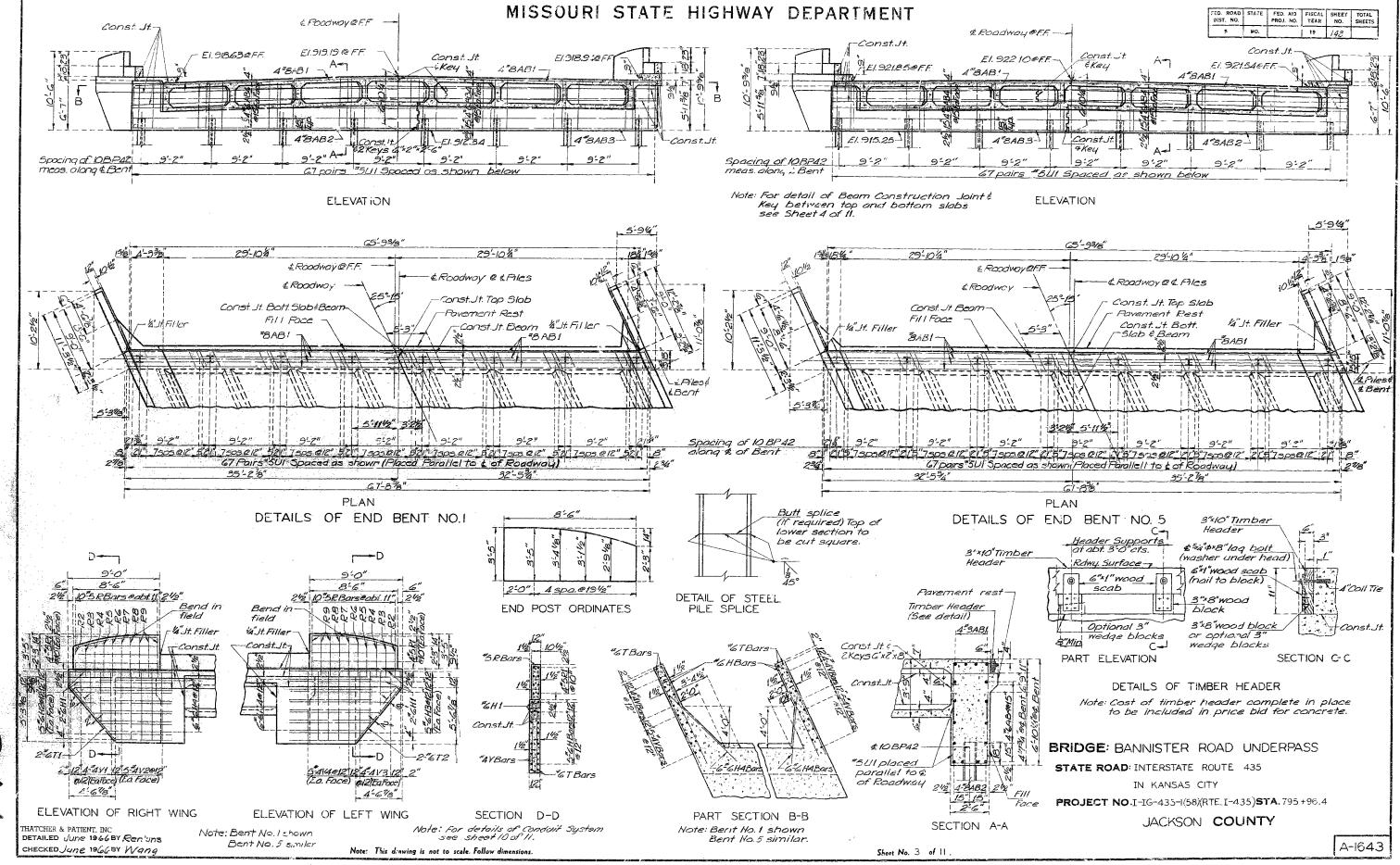
 \*
 16
 "O
 22"9"
 RC3
 "
 Na a a a a a a R2 THRU R9 72" 72 8 "|| 2/-G" PBI Beom 4 "|| 25-9" PB2 " 12 "11 "G-G" "B3 " 13"MOX. ZOK" 232" 32 #4 31-5" G5 32 "8 390" G6 20% 11 tt 32 1/ 380 GT \* spliced - 1:0' Plugo 2.5' splice = 3.5 , L. 68 68 3"R-1 HATCHER & PATIENT, INC. R10 R21 Drawn June 1966 by Rentins Checked June 1966 by Wang Note: This drawing is not to scale. Follow dimensions Sieet No. 2 of 11.

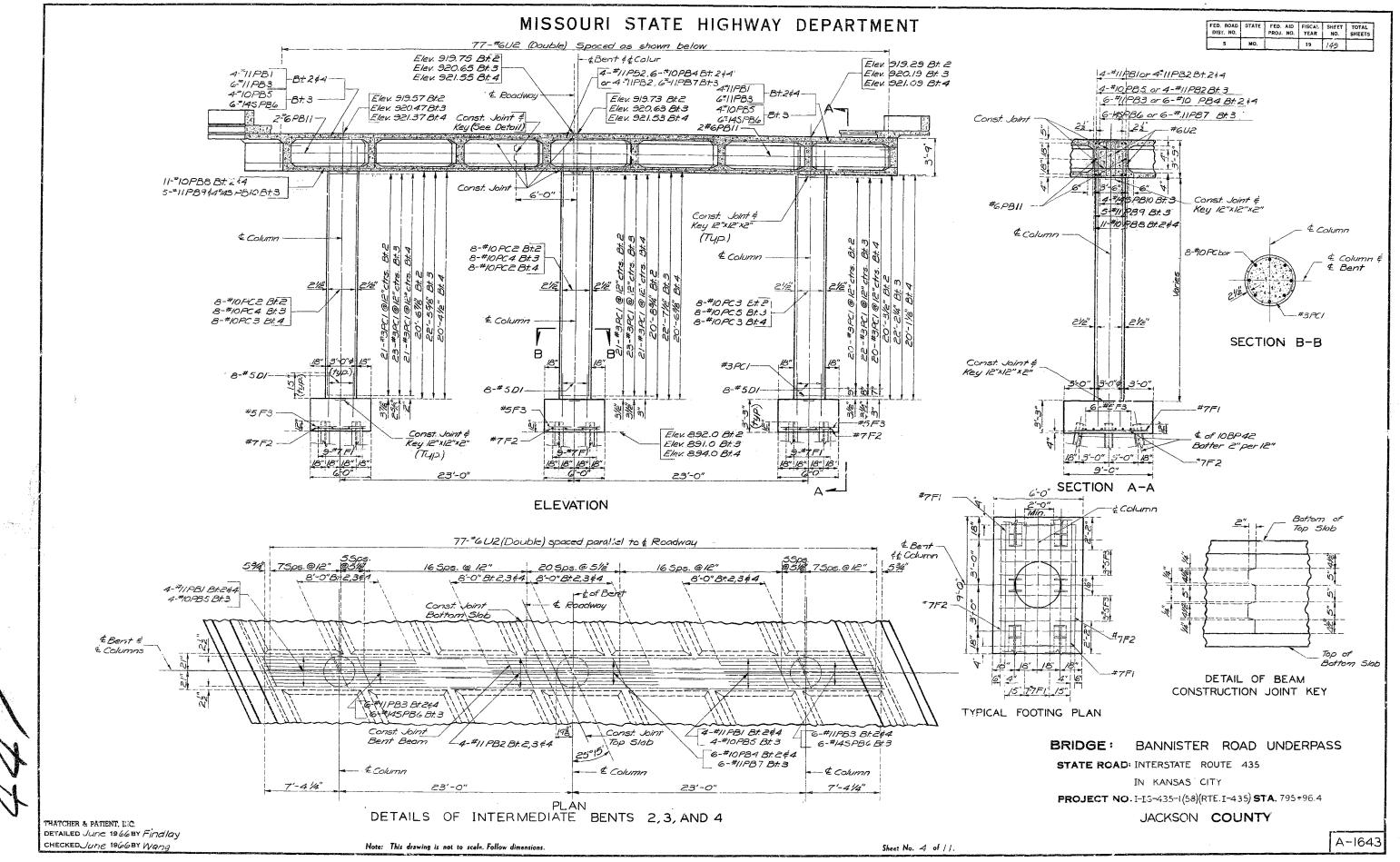
MISSOUR! STATE HIGHWAY DEPARTMENT

#### A16433, Sht. 2

an a						
	FED ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
	5	MO.		19	147	
			40+12	Sk	240	167
Lt. <u>4</u>	27'Rt.	Æ			10'Lt	
Bvernent 905.0 905.5	PCCPoverna	רלר	905.9		90	95.G
	П <u>ЭОг.7</u>	Η	FCC Pov ¢ Bidrs	'+	₽	
	Soft,Silt		SiltyCk	av 1.4	╆ <u>┾</u> ╌─	
Sulty Cloy	Cloy		few Bk	Irs		
	BleckSili	╌╞╎			H N	cothere
Firm Clay -	Weather				1/4	broken imestor
Shole - 8795	or Baulde	rs Y	i <u>381.1</u> Gray Shi	10-		ense
<u>8750</u>	Linestone	: `	197.0190			nestone 1 <u>9.7</u>
-Stiff Si ale	Hord Sh	ole			Sti	ff Shole
Hord Limestone	865.7					
	Limesto	72				· · · ·
					<u>_</u> 3	
A1	67	A2				
) (Auger) ((	_	AZ (Auga	cr)	(C	ore;	
		AZ (AUQC	sr)	(C	ore;	z
	.cz Core) NG DATA	AZ (AUQC	er)	(C	ore;	:
	NG DATA	(Auga	97)	(C	ore;	:
BORIN	NG DATA STER RC	(ruga DAD	er) UNDI	(C	ore;	2
BORIN BRIDGE BANNI STATE ROAD INTERS IN KANSA	NG DATA STER RC STATE ROUT	(2090 AD E 4:	r) UNDI 35	(C	ore;	:
BORIN BRIDGE BANNI STATE ROAD INTERS IN KANSA PROJECT NO.I-IG-435	NG DATA STER RC STATE ROUT S CITY S <sup>-1(58)</sup> (RTE I-2	(2092 AD E 4: TA. 79 (35)	r) UNDI 35	(C	ore;	:
BORIN BRIDGE BANNI STATE ROAD INTERS IN KANSA PROJECT NO.I-IG-435	NG DATA STER RC STATE ROUT	(2092 AD E 4: TA. 79 (35)	r) UNDI 35	(C	ore;	:
BORIN BRIDGE BANNI STATE ROAD INTERS IN KANSA PROJECT NO.I-IG-435	NG DATA STER RC STATE ROUT S CITY S <sup>-1(58)</sup> (RTE I-2	(2092 AD E 4: TA. 79 (35)	r) UNDI 35	(C	are; ass	164.3

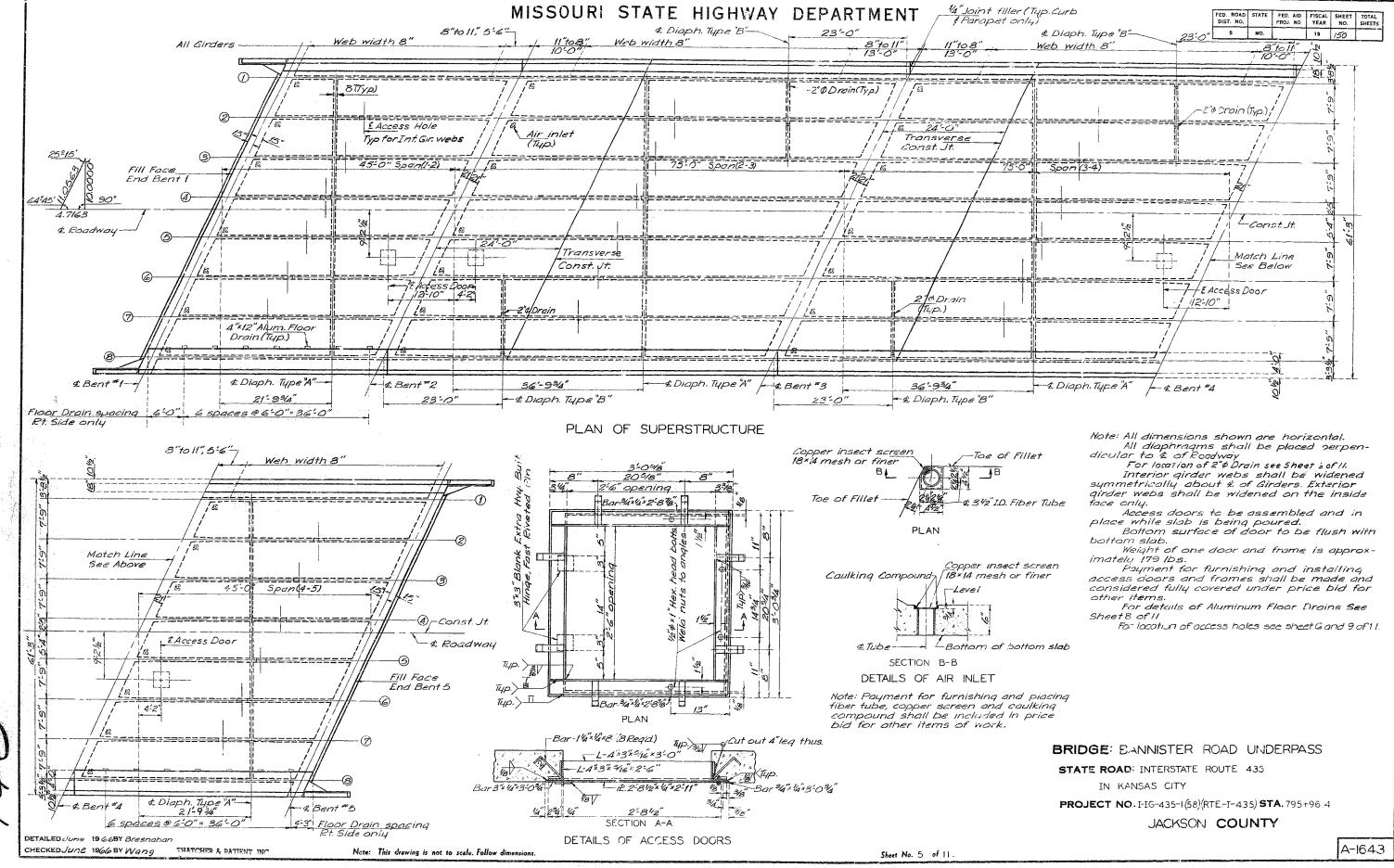
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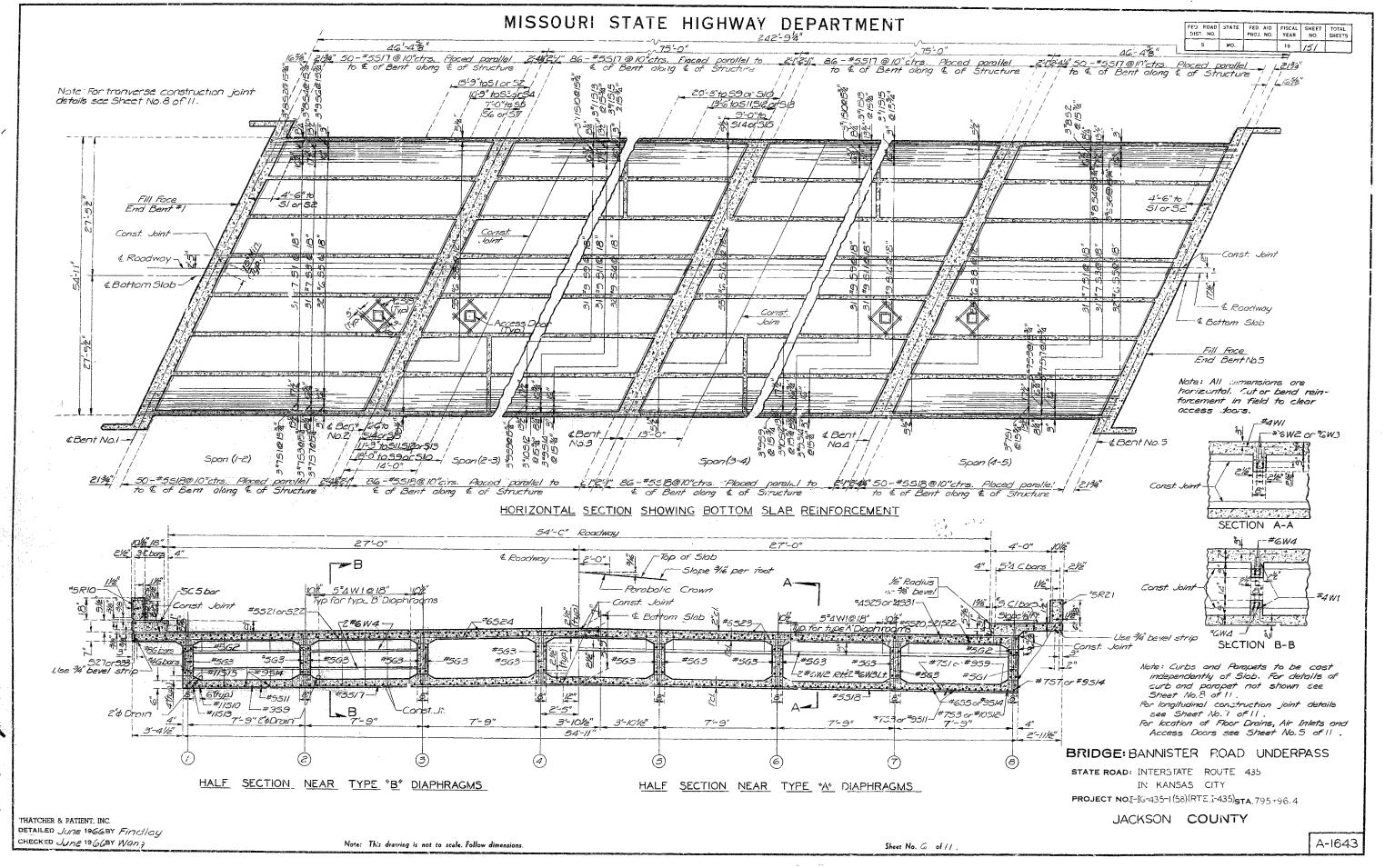




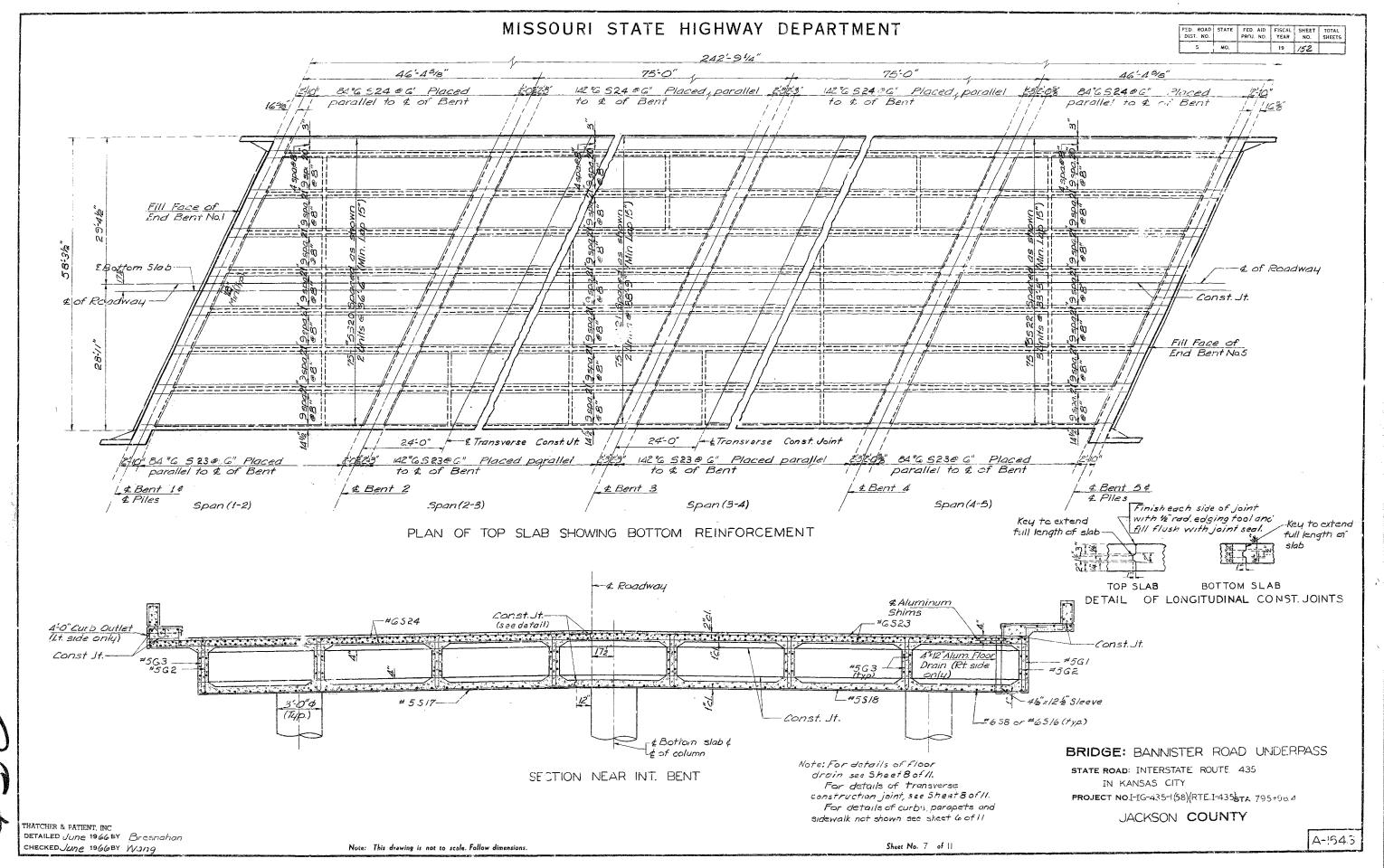
## A16433, Sht. 4

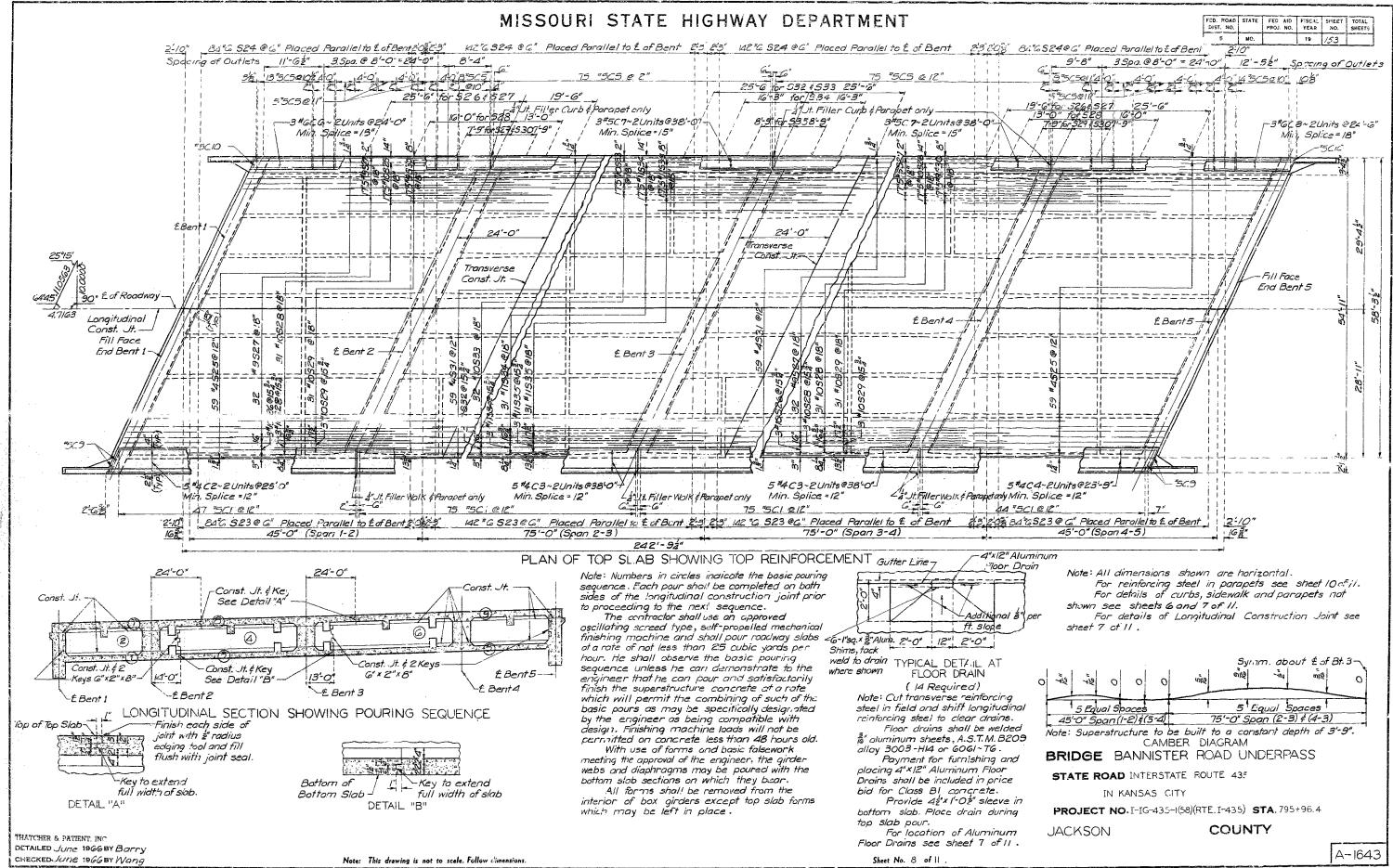
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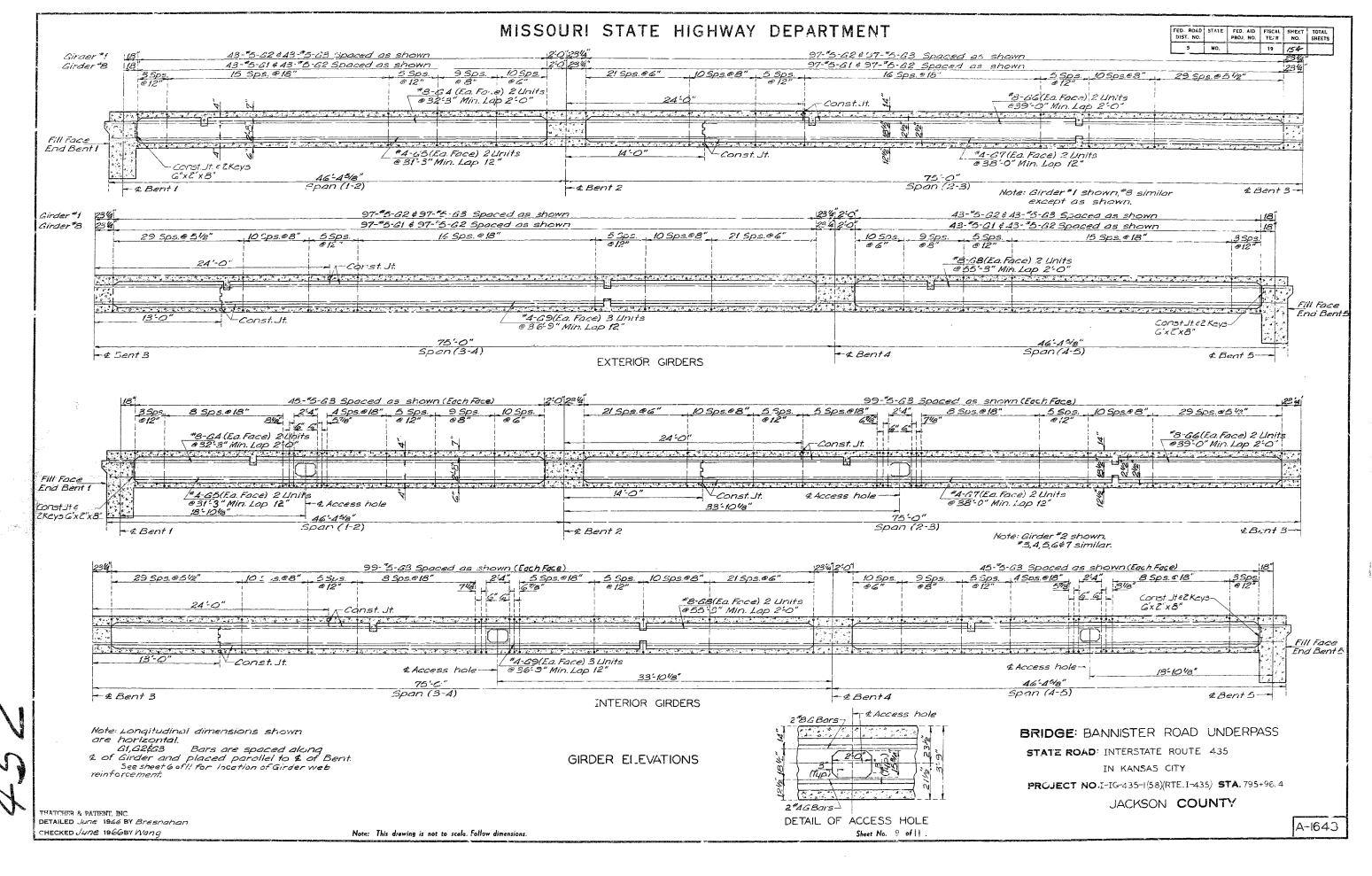


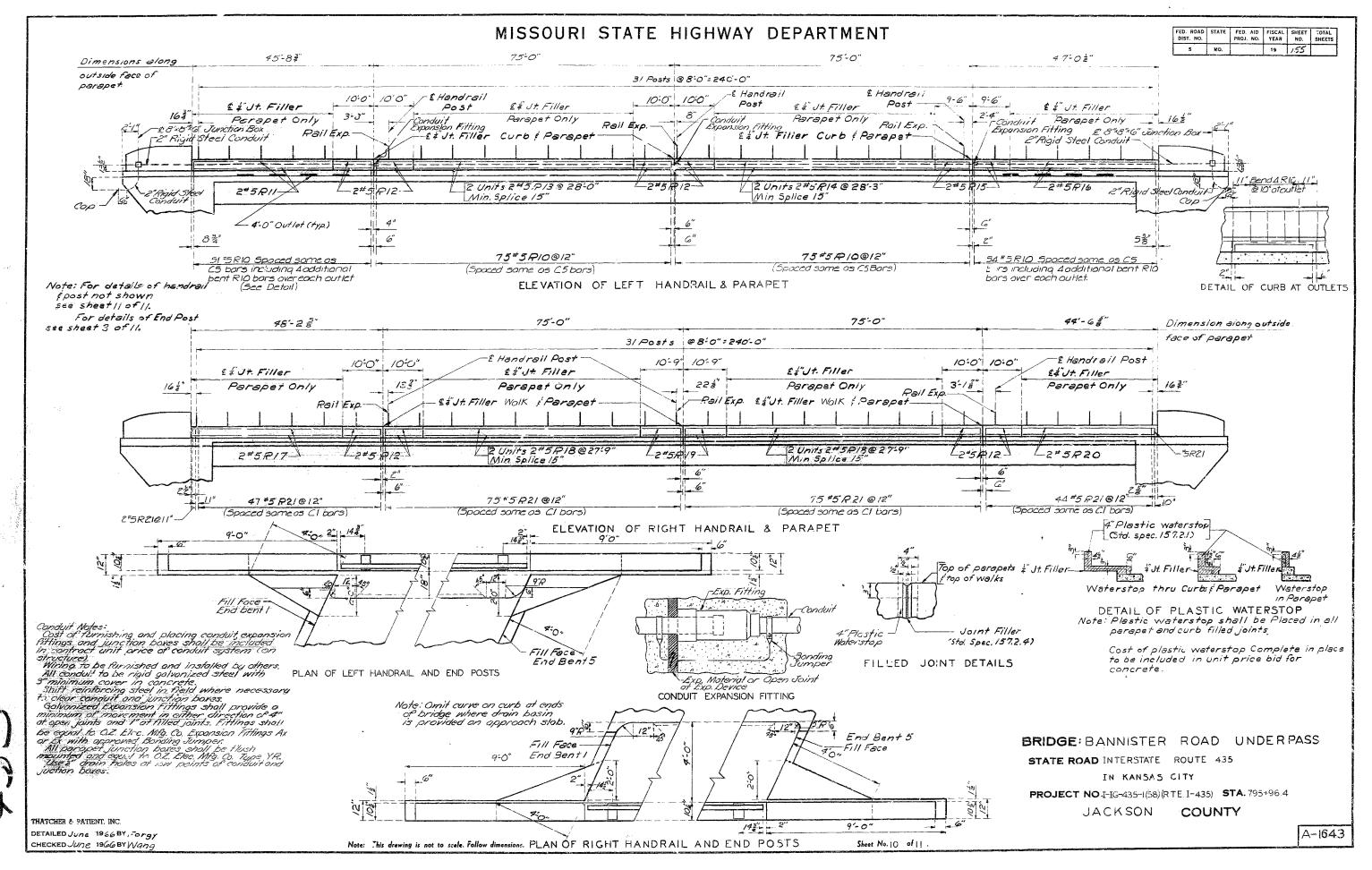
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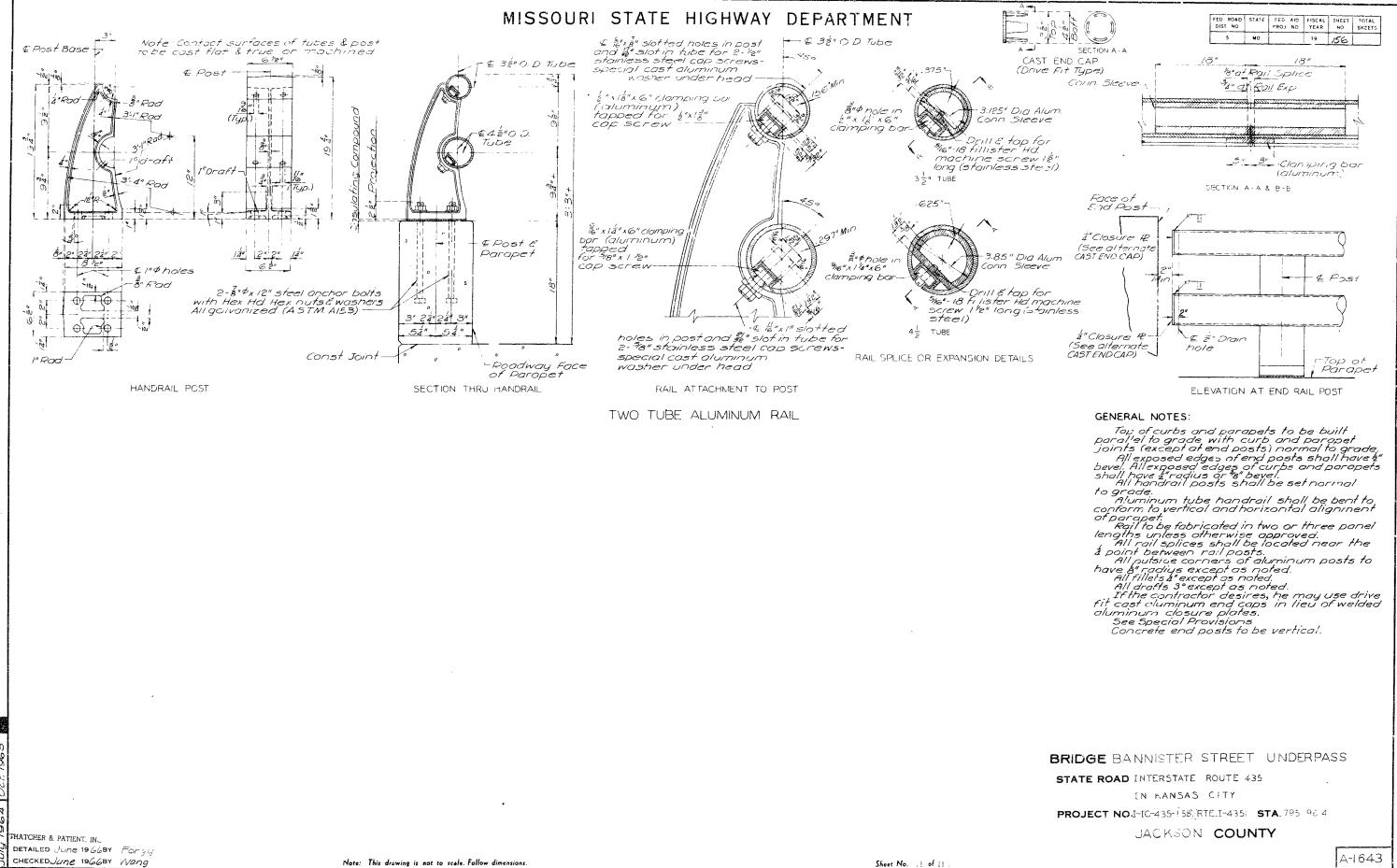


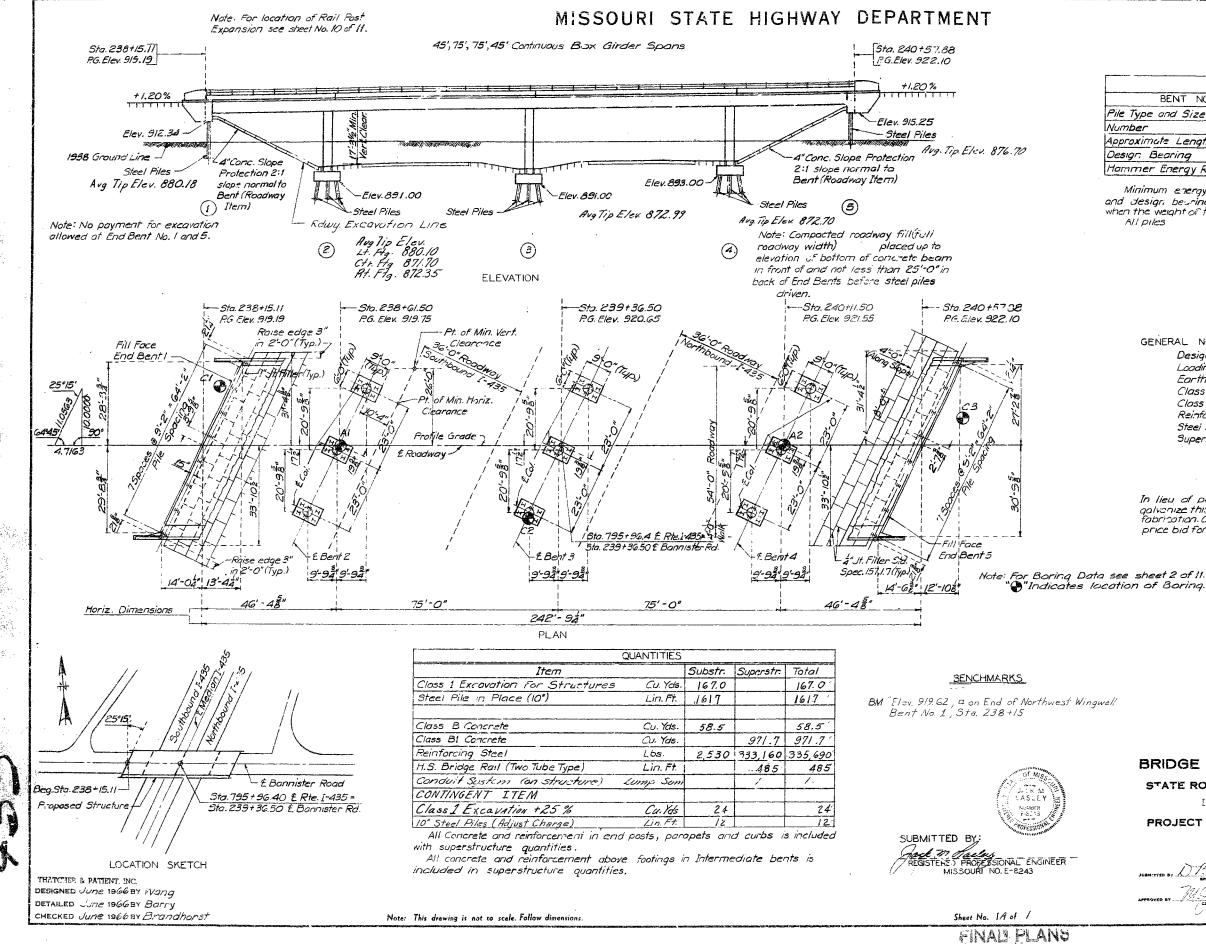












Pile Num Appr Desi Har

## A16433, Sht. 12

			D. ROAD	STATE				
			ST. NO.	STATE	FED. / PROJ.		SHEET NO.	TOTAL SHEETS
			5	MO.		19	14G	
					Ē	INAL	PLA	\S
	PILE	DATA						7
BENT NO.		1	2	T	3 .	4	5	-
e Type and Size		105-2	IOBP4	210	BP42	108:42	10 BP	42
mber		8	18	7	18	18	B	-
proximate Length	Ft.	34	12 20 1	R <del>.</del>	12	2!"	40	1
sign: Bearing	Toris	38	54	_	56	54	38	1
mmer Energy Required	Ft. Lbe	8600	12,70	0 1	3,200	12,700	8,60	0

Minimum energy requirement of harmmer based on plan length and Jesigri bearing value of piles Increase by the factor (W+w)/2W when the weight of the rorn (W) is less than the weight of the pile (w). All piles driven to prosticol refusal.

GENERAL NOTES:

Design Specifications A.A.S.H.O. ~ 1961 Looding H320-44 (15 #/sq.ft. Future Wearing Surface) Earth120#, Equivalent Fluid Pressure 30\* Class B Concrete (substructure) fc = 1,200 psi Closs BI Concrete (superstructure) fc=1,600 psi Reinforcing Steel fs = 20,000 psi Steel Pile (A.S.T.M. A36 G3T) fb = 9,000 psi Superstructure deck surface sealed.

In lieu of painting, the access doors, the contractor preferred to galvenize this moterial. All golvanizing done ofter fabrication. Cost of galyanizing included in price bid for other items

BRIDGE BANNISTER ROAD UNDERPASS

#### STATE ROAD INTERSTATE ROUTE 435 IN KANSAS CITY

PROJECT NO.1-IG-435-I(58)(RTE.I-435) STA. 795+96.4

#### JACKSON

#### COUNTY

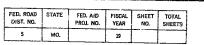
新家・5と1日

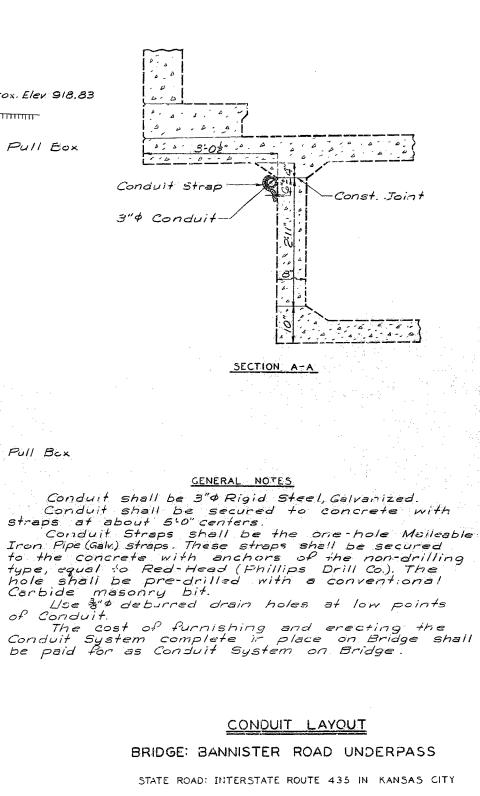
JUBMITYED BY DATA CONTACT SATE 12 1 Tag C St.	
APPROVED BY MAN COMPANY COMMENSION	STD. 54.0
CHIEF ENGINEER	A-164:

2 23

MISSOURI STATE HIGHWAY DEPARTMENT Approx. Elev. 921.96 A-3 Approx. Elev. 918.83 Conduit Strepminnin 001 To Full Box 3" 4 Conduit (Typ.) To Pull Box Bt.#5 Bt.#4 Bt.#3 B+#1 Bt.#2 Bek 2G1'± PARTIAL ELEVATION NORTH SIDE OF BRIDGE -Outside Face of Girder To Pull Bex -Outside Face of Parapet 3" & Conduit (Typ) To Full Box-268'± PARTIAL PLAN NORTH SIDE OF BRIDGE of Conduit. DETAILED MAY 1973 BY BRANDEL CHECKED MAY 1973 BY PATTERSON Note: This drawing is not to scale. Follow dimensions. Sheet No. 1 of 1.

## A16433, Sht. 13



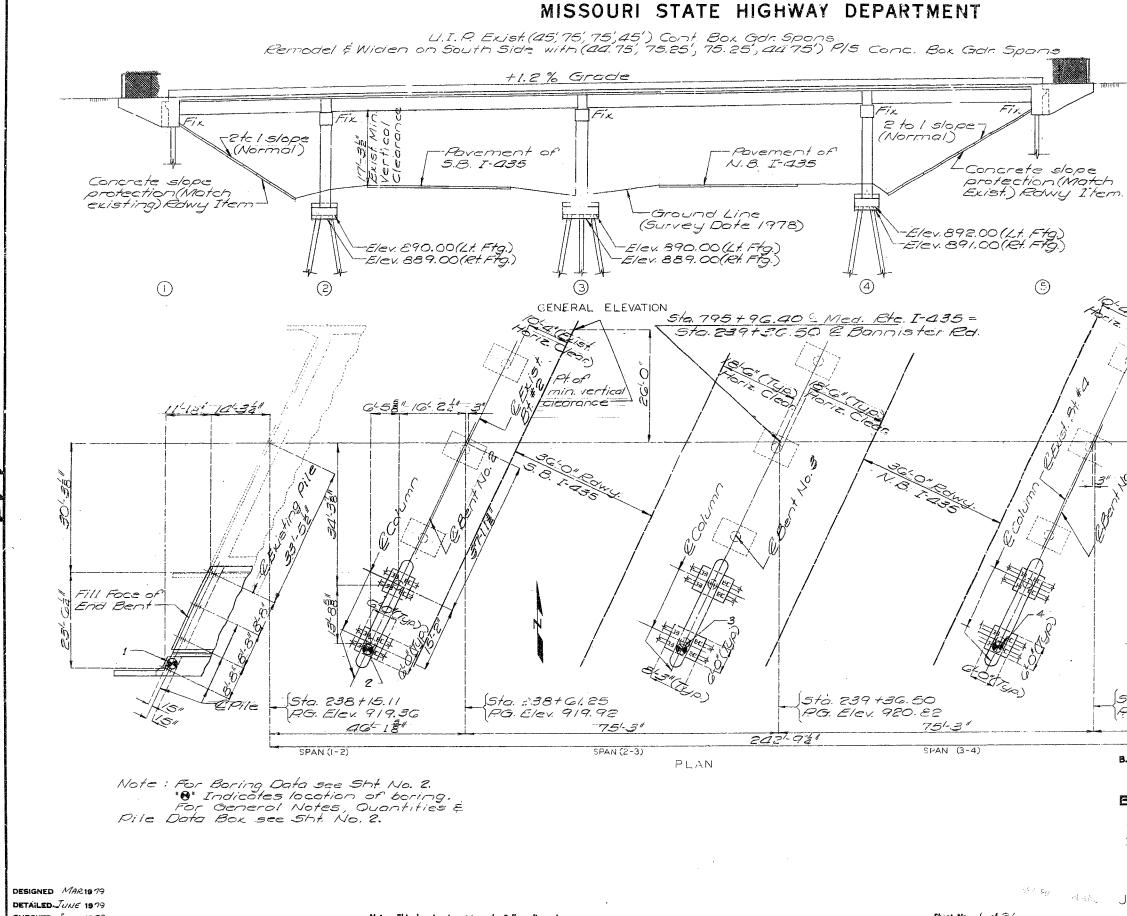


PROJECT NO. 4-U-435-53 (RTE. I-435) STA. 795+96.4

JACKSON

#### COUNTY

A-1643A



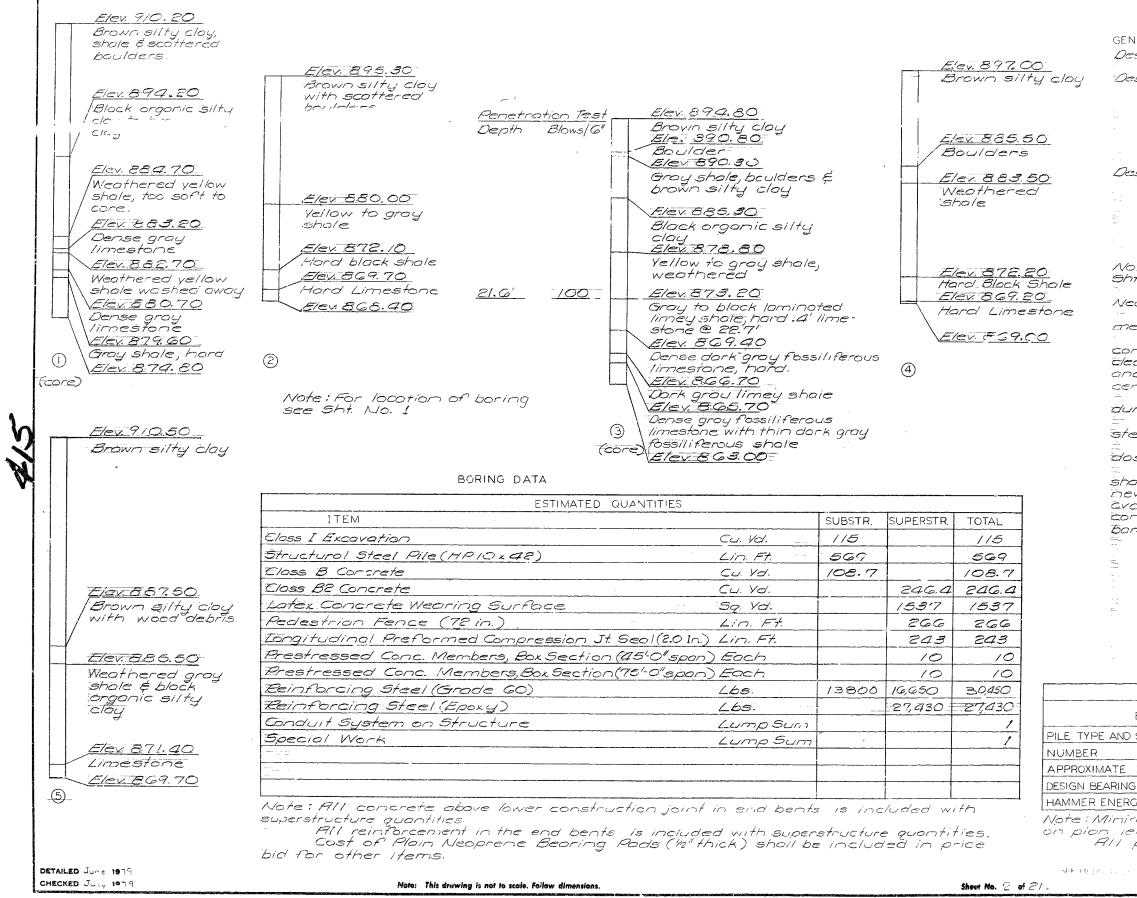
CHECKED JULY 1979

#### A16433, Sht. 14

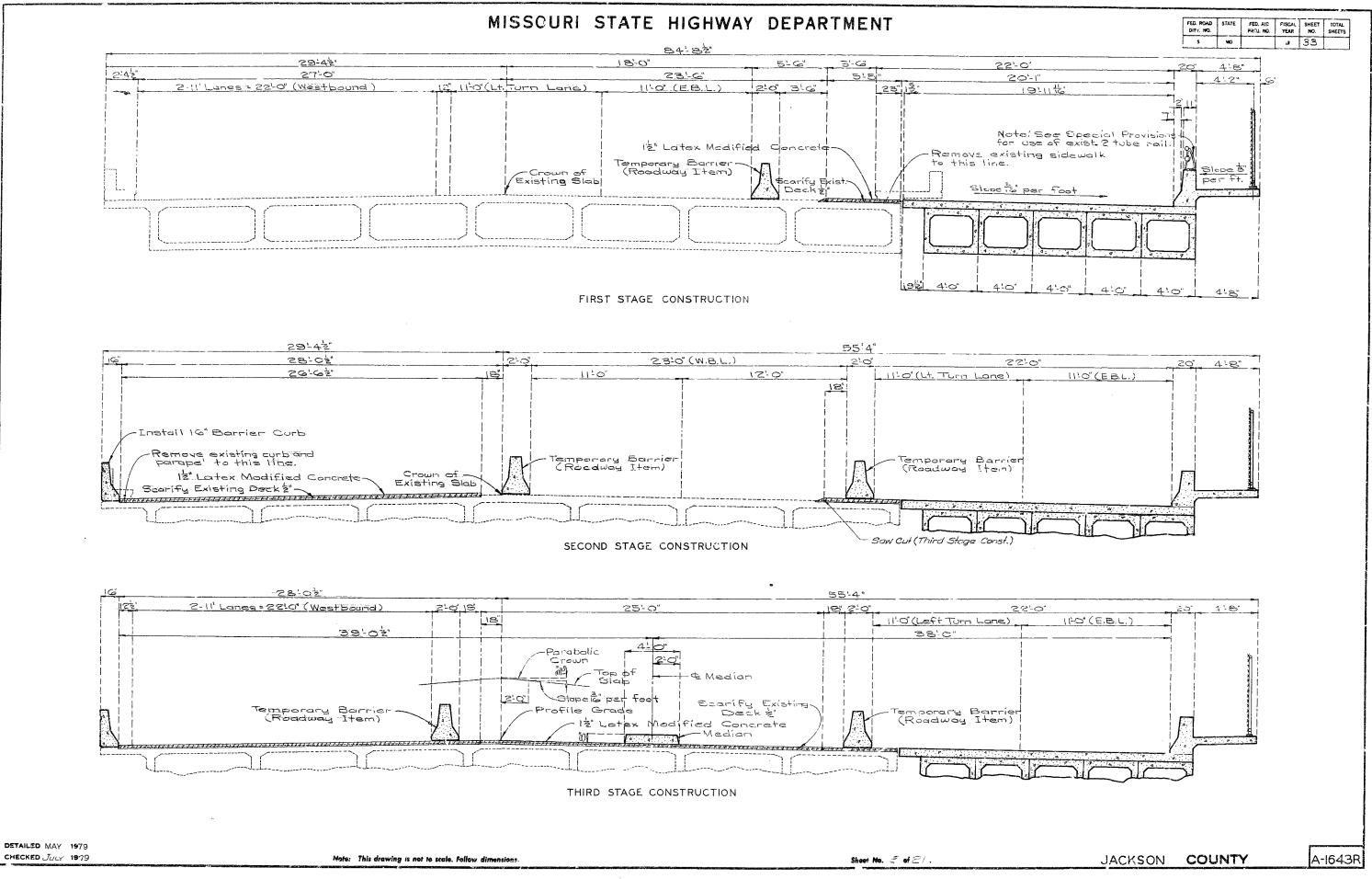
FED, ROAD	STATE	FED. AID	FISCAL	SHEET	TOTAL
DISC, NO.		PROJ. NO.	YAAR	NO.	SHEETS
5	MO.		وز	31	

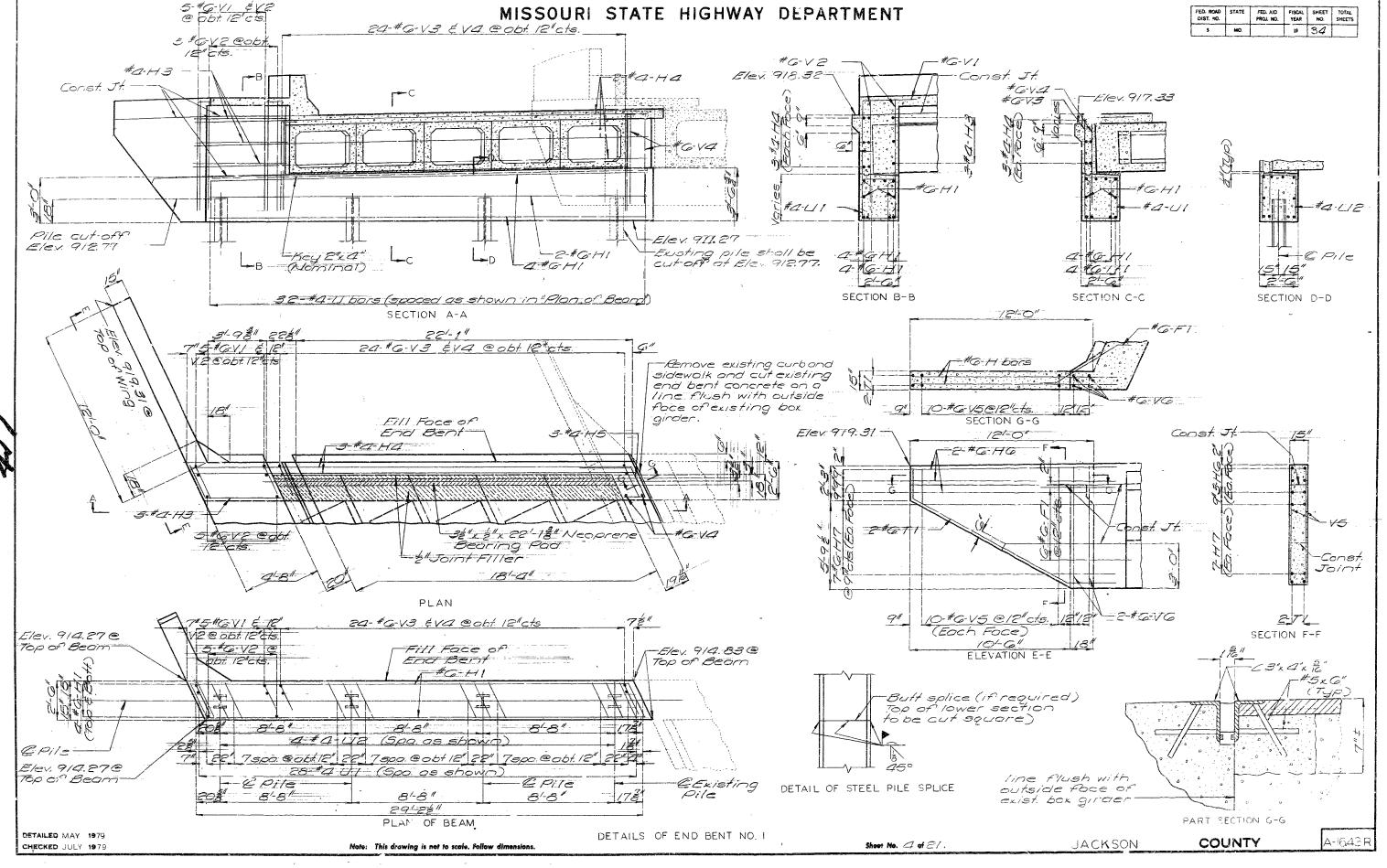
Note : Compacted roadway 17:11 shall be completed to the final roodway section and up to the elevation of the bottom of the concrete beam within the limits of the structure and for not less than 25' in back of the fill foce of the end bents before pilecre driven for any be. Stailing within the embankment section. -25°-15' 90° 464:25) 4.7163 Face of Fill End Bent 5ta. 240+11.75 5ta, 220+57.85 PG. Elev. 921.72 P.G. Elev. 922.27 46'- i 着 Horizontal Dimensions SPAN (4-5) B.M. No. 10 Elev. 719. GE = On N.W. Wingwall Bent No. 1 Lt. Sto. 238+15 BRIDGE: BANNISTER ROAD UNDERPASS STATE ROAD INTERSTATE ROUTE 435 IN KANSAS CITY STA. 7958 96,40 L. MED 141 PROJECT NO. STD. 314.0 RTE 1-435 JOB NO. 4-1435-407 State JACKSON STD. 706, 35 COUNTY Г. ТЕ 8-3/-79 A-1643

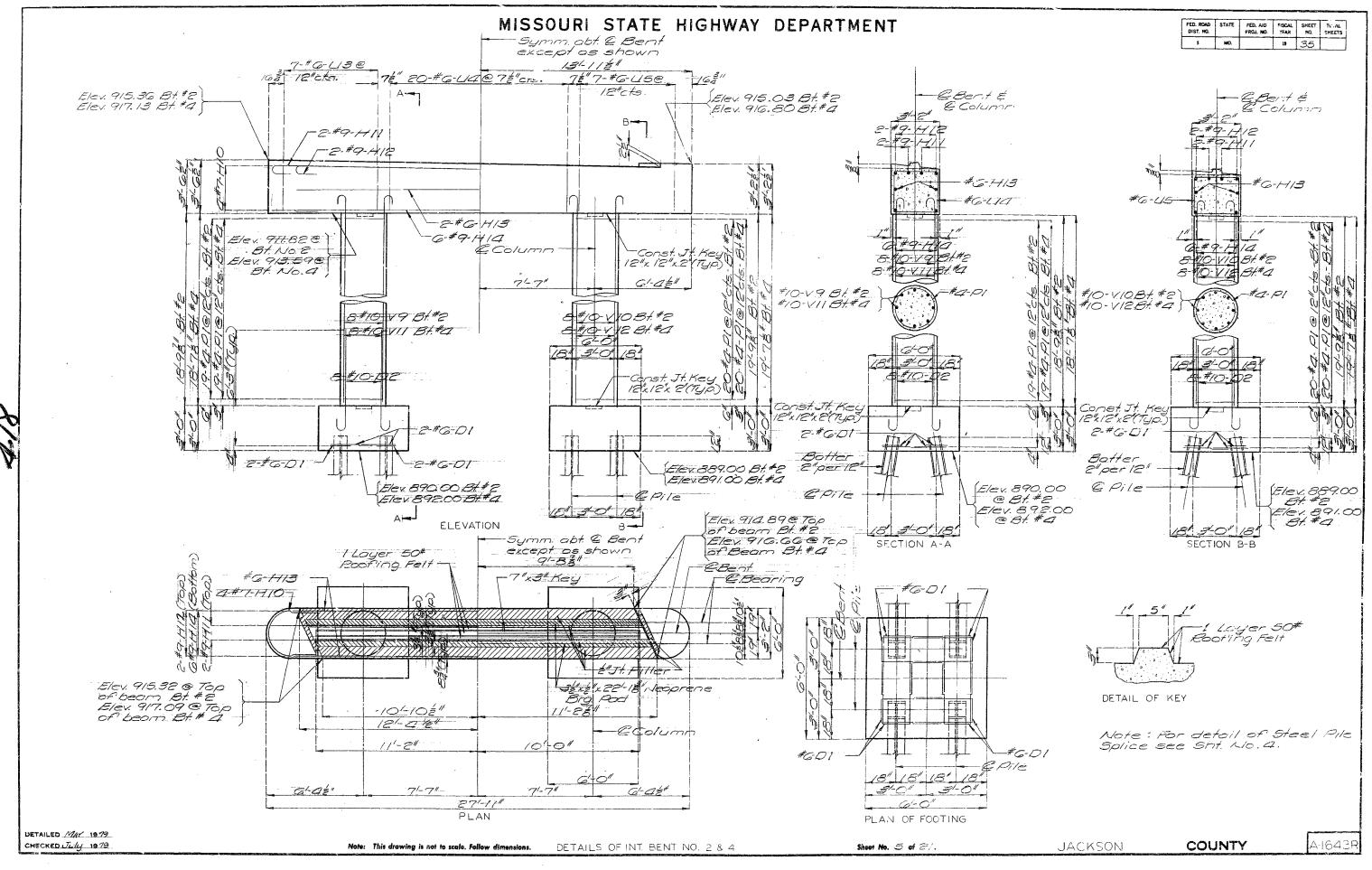
#### MISSOURI STATE HIGHWAY DEPARTMENT



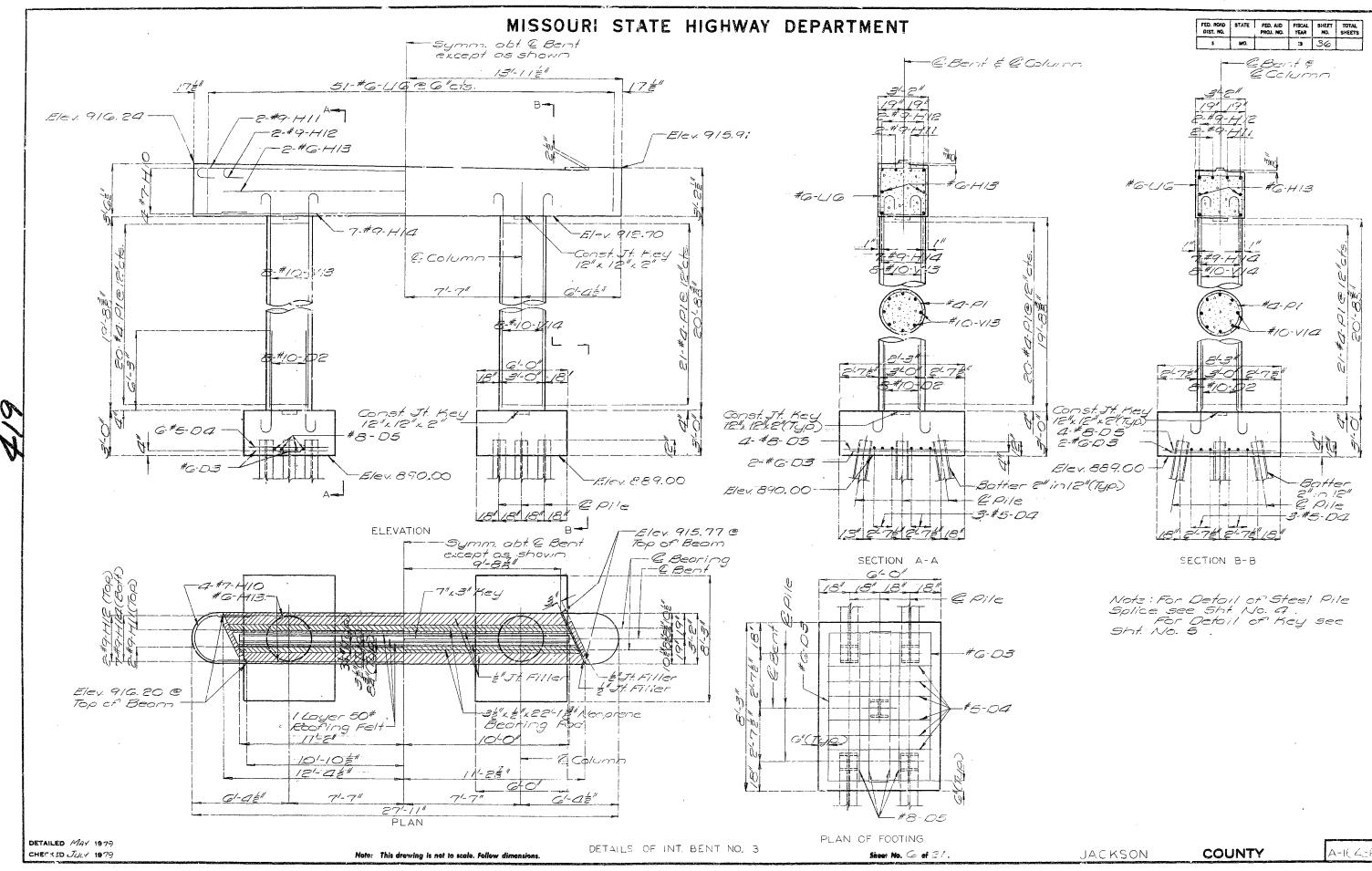
				FED. ROAD	STATE	FED. AID	FISCAL	l curre	1014
				DELT. NO.		PROJ. NO.	FISCAL YEAR	NO.	TOTAL
				¥	MO.		19	32	
NERAL NOTES	-								
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Ax/e.									
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comp	cs/4e		<u> </u>	unp	19.	sapp	/U/~;	i ed	
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sign Uni Class	t Str B Cr	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	5 ; = t ,= (	SULE	4-1	-+1.100	= ) ≁7	/ = -71	00 <i>psi</i>
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prest Reint	ress	ed bi	ox gir	der	5) <del>/</del>	C = Z	7 <i>0</i> 0	0 p.	s./.
Steel	Dile	79 S • Fh=	tee/ ( = 9m	000	'e G i	) A	y = C	0,0	00 p.s.i.
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ote: For F nt. No. 10.	-1257	ress	ea c	nrcle	r 5	tres	se	5 50	20
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coprene .	Põde	э.							
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False	work	over	r exi	stin	0 /c	nes	; =1	boil	be
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arance a a mini	ר י דכ ריז גרי	7-0" 1 n lot	tron terni	n arc I aler		n of 'e nce	xist	ing 40	10nes -01
ntered o	n ex	cistir	29 10	nes		,_~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	<i></i>		<u> </u>
ntered on existing lones. Traffic over structure to be maintained									
Minimum clearance to reinforcing									
zel shall be la" unless otherwise shown.									
Outlir	2e. ¢	f 0.'0	wor	K is	ind	icate	ed ,	641	ligint
shed lin Bors i	esii	Heov	y lir	nes :	indi	cot c	€ r	7ew	work.
ail be cle	anly	stri	ipped	d and	ר בי מר	mbe	dde	d in	770
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ailable, c ncrete of	d b	t ar	shal.	l'ex	ter	d ir	70	7. 7000	1
ncreie uf rei und 3									
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·	PILE	DATA	<u>\</u>						
		- DATA	<u></u>	1			T		<del>,</del>
BENT NO.	I	2	3-LT.	3-RT.	4-L	T.   4-	RT.   9	5-LT	5-RT
SIZE	HPIOx42	HPIOx42	HPIOx4	HPIO,4	HPIO	XAZ HPIC	x42h	PIOXA	HPIOx42
	3	8	5	5	4			1	2
LENGTH FT		11	21	15	20			26	38
· · · · · · · · · · · · · · · · · · ·			£3	E3	50				
G TONS		56		1				<u>47</u> 200	47
CY RQD FTLB	L	13200				$\infty$ $B2$		<u>2000</u> e c - k	
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pile sho	ii be	driv	en f		act	ical	re	FUS	01.
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JA	CKSC	)N	_	COL	INT	<u>Y</u>		A-	1643R
								-	





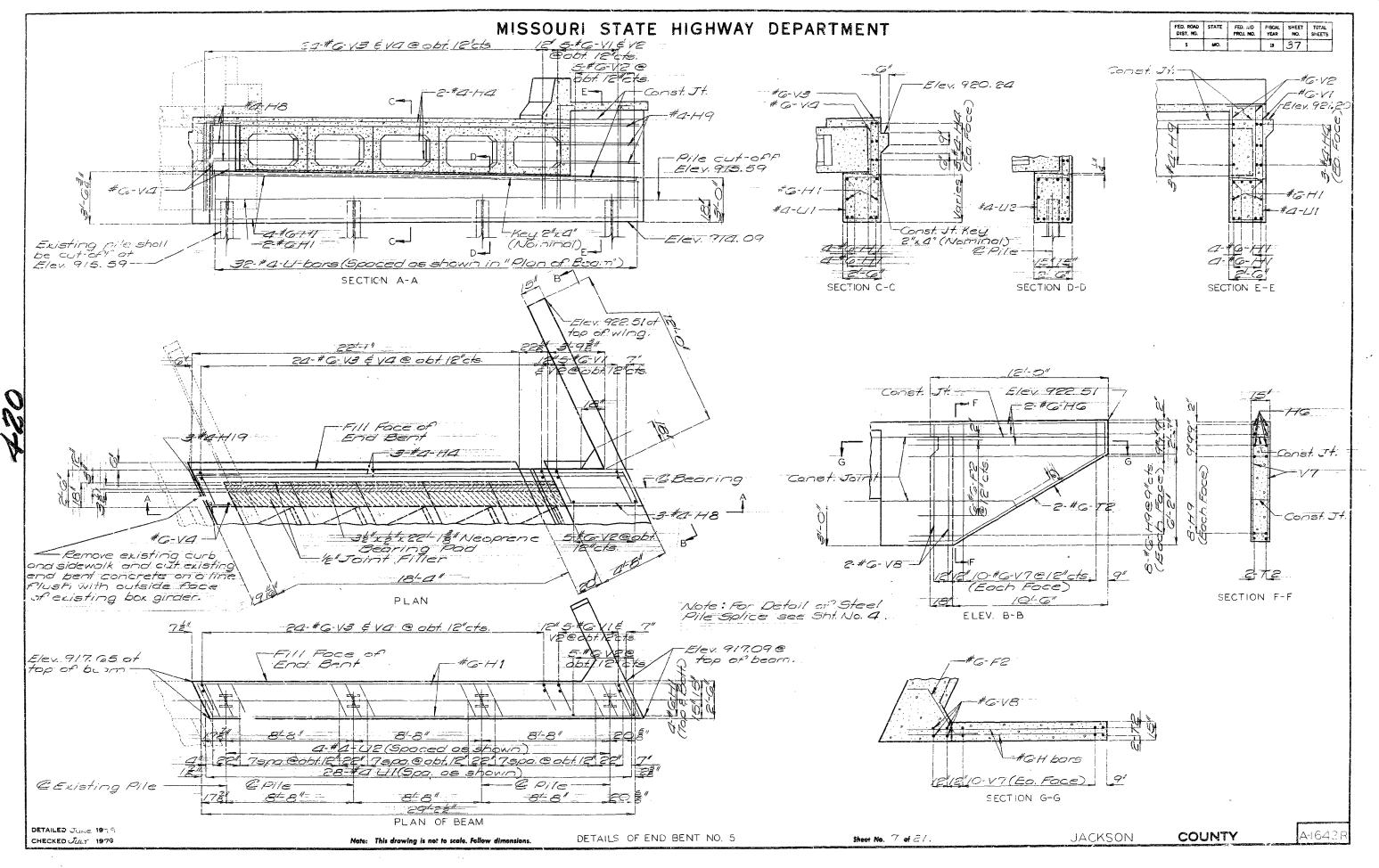


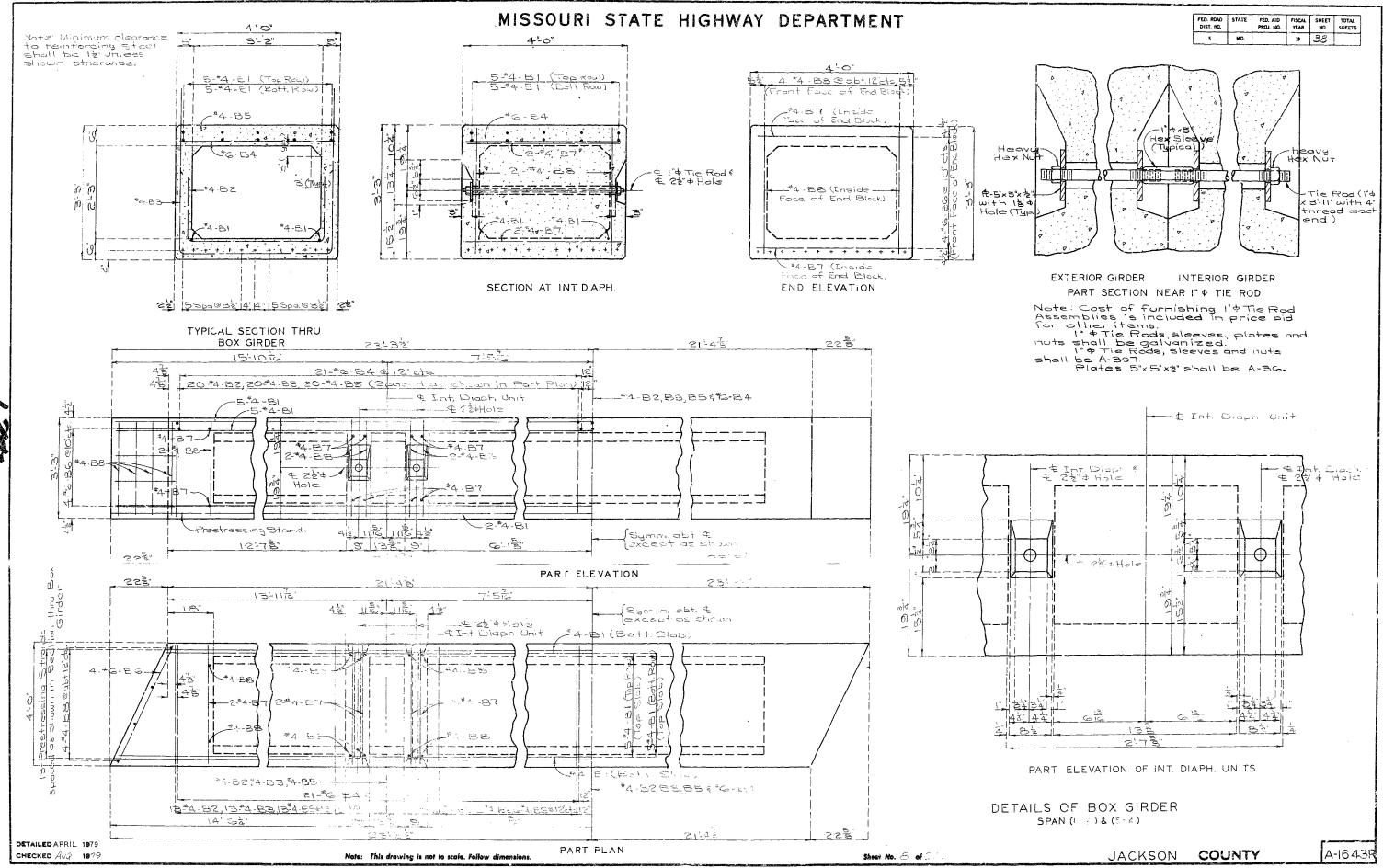
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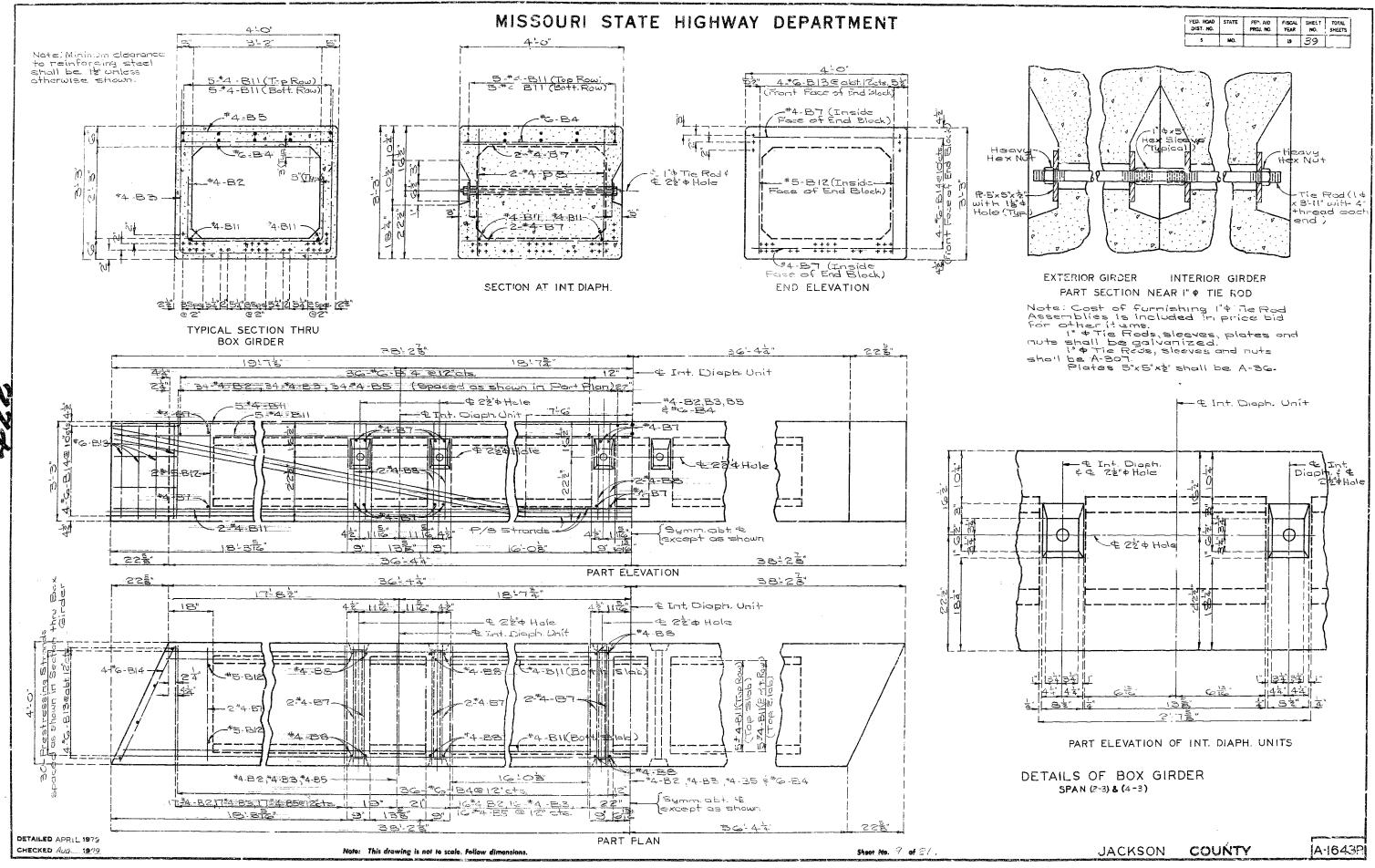


#### A16433, Sht. 19

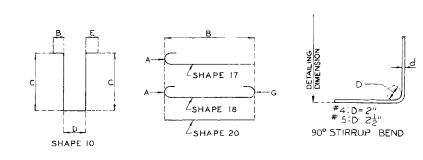
A-IC42R







COMPLETE BILL OF REINFORCING STEEL-EACH BOX GIRDER         MARK       DIMENSIONS       I I I I I I I I I I I I I I I I I I I
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2 0 2       0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
2+ 181       Box Girder       1/20       23 1.000       23 1.23 1.370       23 1.23 1.370       24 1.48?       25 1.23 1.370       41 1.94       41 1.94       41 1.94       41 1.94       41 1.94       41 1.94       41 1.94       41 1.94       41 1.11       326       56 0.00       12 1.11       11 1.1326       26 0.00       90° STI
41         483         Box Girder         HIDS         4         2000         3         90° STI           41         483         Box Girder         HIDS         4         2000         3         90° STI
43     GB4     Box Girder     H 20     3 9.000     3 9.242     SHAPE 10       41     485     Box Girder     H 105     20.000     3 9.000     7 1 6 11 189
E GBG Box Girder H20 4 2.000 4 2.000 4 2.000
20         437         Box Girder         H20         3         9.001         3         9.51         THAN 180 DEG, TO BE BRAN WITH SAME PROCEDURE           28         483         Box Girder         H20         3         0.000         3         0.50         0.56         As FOR 90 DEG, STANDARD HOURS.         As FOR 90 DEG, STANDARD HOURS.
WITH THE PROCEDURES AS SHOWN ON THIS SHEET. NOMINAL LENGTHS ARE BASED ON OUT TO OUT DIMENSIONS SHOWN IN BENDING DIAGRAMS AND



NOTE: ALL STANJARD HOOKS AND BENDS OTHER THAN 180 DEG. TO BE BENT WITH SAME PROCEDURE AS FOR 90 DEG. STANDARD HOOKS. HOOKS AND BENDS SHALL BE IN ACCORDANCE WITH THE PROCEDURES AS SHOWN ON THIS SHEET. NOMINAL LENGTHS ARE BASED ON OUT TO OUT DIMENSIONS SHOWN IN BENDING DIAGRAMS AND ARE LISTED FOR FABRICATORS USE.

S-STIRRUP

	COMPLETE BILL OF REINFORCING STEEL-EACH BOX GIRDER																				
								DIMENSIONS								INAL GTH	TUAL NG TH	H			
REQ'D	NO. Lu X	LOCATION	DE 60	SHAPE NO	STR.	EACH		В	i 	С		D		E		F	н	ĸ	NOMINAL LENGTH	ACTU	WEIGH
Ň	SIZE MAR		GRA	STI	SUB	NON	FT.	IN.	Fi	IN.	FT.	IN.	FΤ.	۴N.	FT.	IN.	FT. IN.	FT. IN.	FT. IN	FT. IN	LBS.
		SPAN (2-3)(4-3)					]		ĺ										1	Ι	
69	482	Box Girder	HI	05						22.500	З	6.000							73	7 1	326
60	483			υs	LI				4	2.000	З	9.000							121	11111	549
73	664		142				Э	9.000											39	3 3	411
69	465	Fox Girder	ΗI	٥S						20.000	З	9.000							71	61	319
			$\downarrow$		4	+													ļ		
58	4B7		HZ		$ \downarrow$	Ļ	3	3,000					ļ						39	39	05
24	468		H2	_		1	3	0.000										1	30	30	48
:24	4B11		42				38	0.660										l	38 0	38 0	603
4	5B12		HZ		<u> </u>		3	0.000											30	30	13
8	GBI3		HZ				Э	0,000											30	30	36
8	GB14	Box Girder	14/2	0			4	5'000											42	42	50

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#### A16433, Sht. 23

FED. ROAD			FISCAL	SHEET	TOTAL
DIST NO.			YEAR	NO.	SHEETS
	мо	_	19	40	

1

GENERAL NOTES-PRESTRESSED BOX GIRDERS

DESIGN UNIT STRESSES: NON-PRESTRESSED REINFORCING STEEL. is - 24,000 psi CONCRETE FOR PRESTRESSED GIRDERS ULTIMATE COMPRESSIVE STRESS 5 000 p.s.i (A-1) MINIMUM COMPRESSIVE STRENGTH AT TRANSFER 4.000 p.s.i.

PRESTRESSING TENDONS SHALL && HIGH STRENGTH UNCDATED SEVEN-WIRE STRESS RELIEVED STRANDS FOR PRESTRESSED CONCRETE CONFORMING TO A S.T.M. A-416 EXCEPT THAT NOMINAL DIAMETER OF STRANDS = \," AND NOMINAL AREA = 0 153 SQ. IN. AND MINIMUM ULT!MATE STRENGTH = 41.300 HDs. (270 ks))

THE WETHOD AND SEQUENCE OF RELEASING STRANDS SHALL BE SHOWN ON THE SHOP DRAWINGS

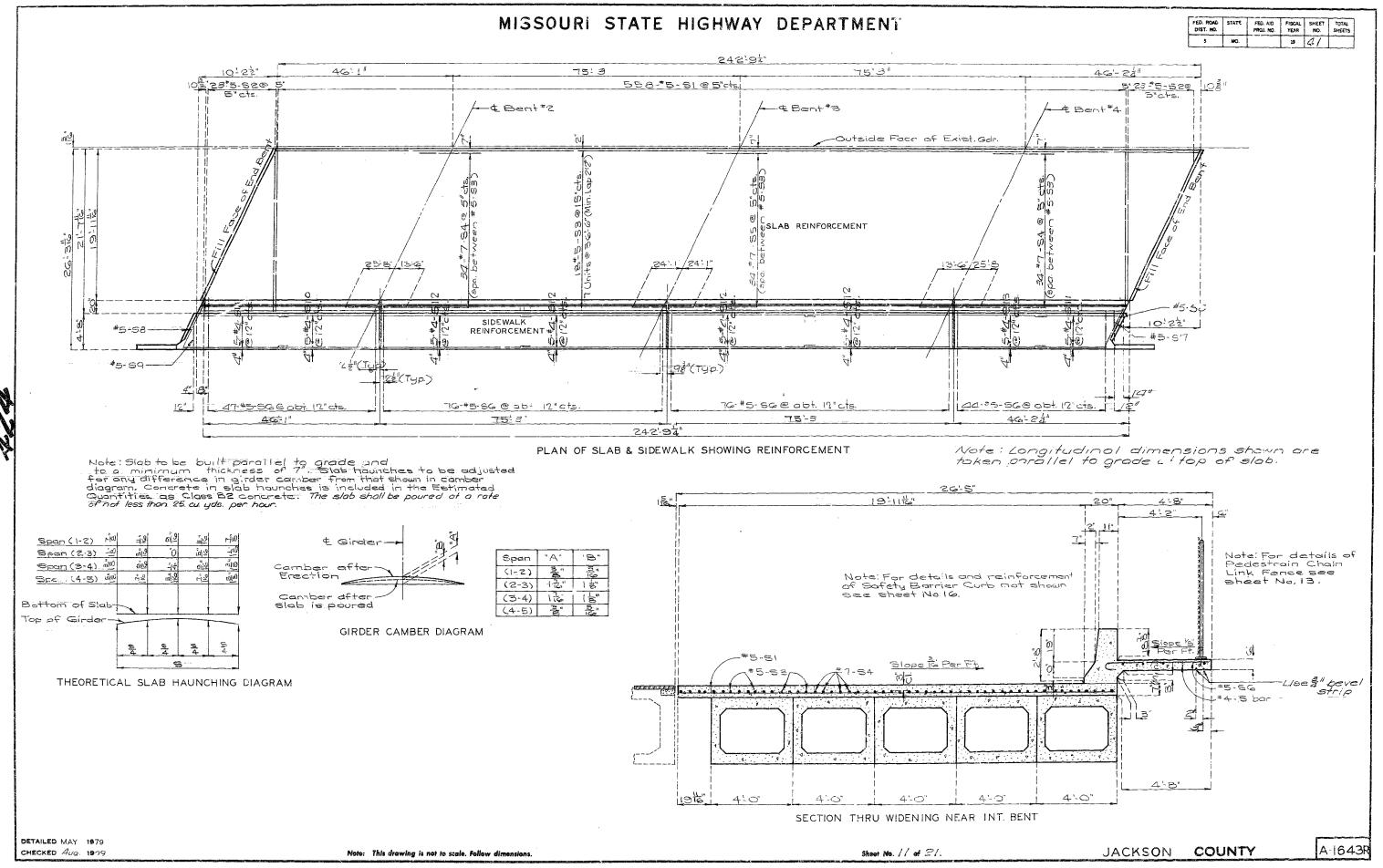
(+) INDICATES PRESTRESSED STRANDS

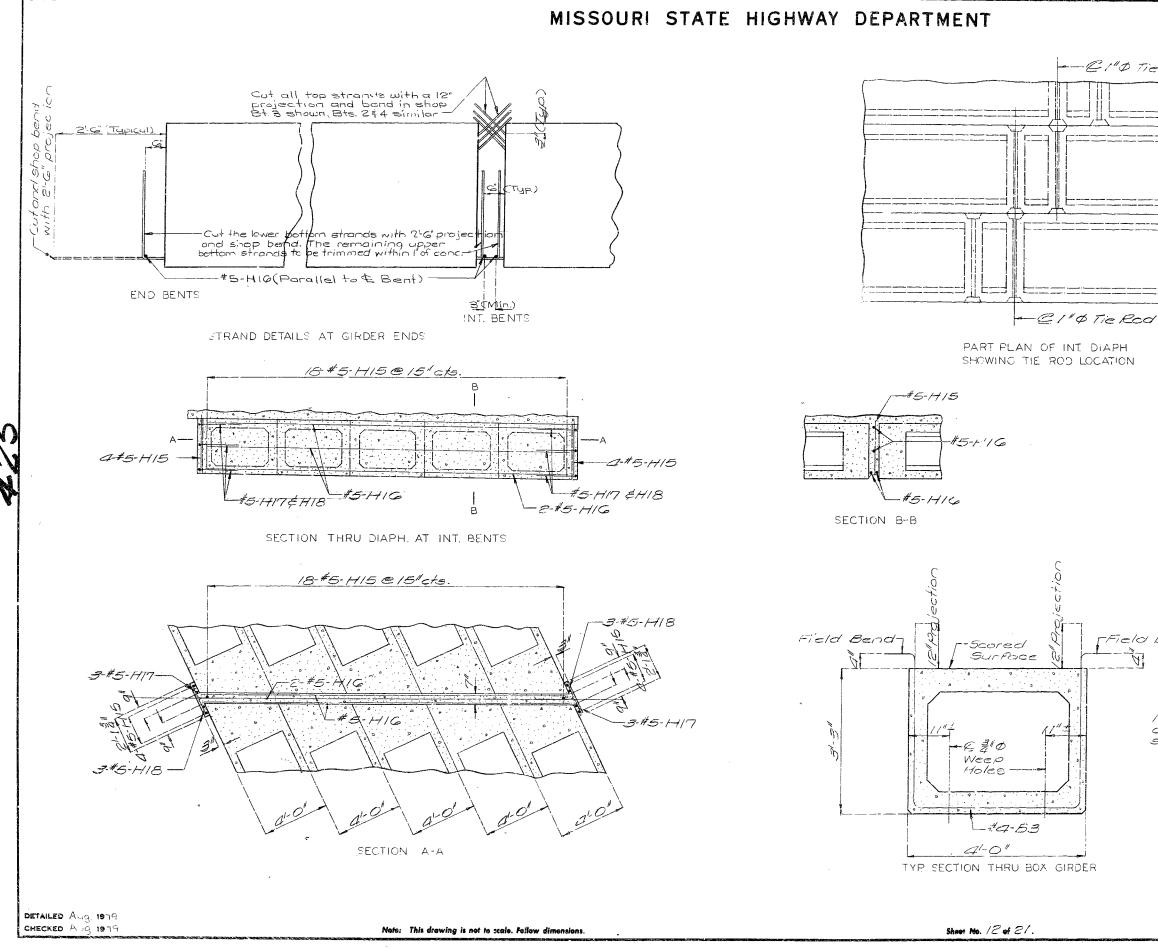
USE 13 STRANDS. INITIAL PRESTRESS FORCE = 376 Kips. (45'Score) USE 30 STRANDS. INITIAL PRESTRESS FORCE = 376 Kips. (75'Score) Two weep holes shall be provided at lower end of Each vido. THEY SHALL EXTEND THRU FORMS AND BE KEPT OPEN AT ALL TIMES

JACKSON

COUNTY

A-1643F





# A16433, Sht. 25

FED. RY/D DIST. NO.	STATE	FED. AID PRGJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
5	MO.		19	42	

12

Ø	Tie	Re	d	
				=4
				=
				(
				=
				(

Field Bend T.

Note: 📲 Ø Weep Holes to Le centered between prestrussed strands.

COUNTY

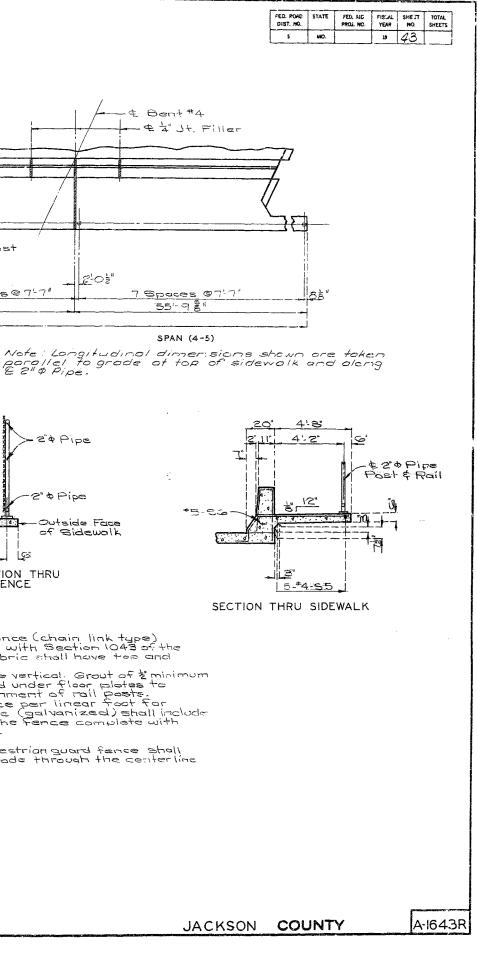
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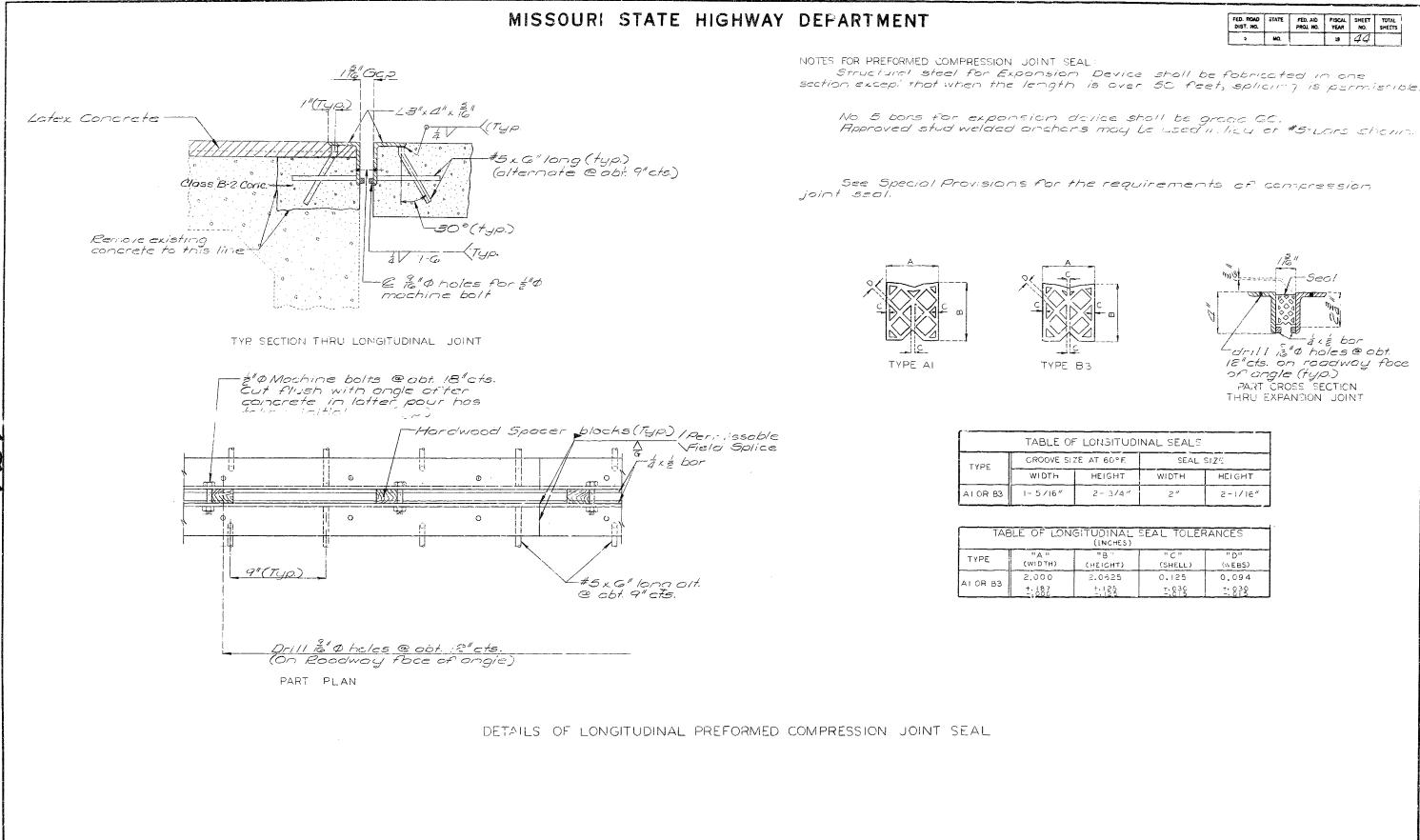
6

#### MISSOURI STATE HIGHWAY DEPARTMENT E Bent#2 & Bent #3 et it. Filler Et Jt Filler E Fence Post (Typ. E Pull Post & Pull Post except as noted) 172" 102 8 Spaces @ 7-7 4 Spaces @ 7-7 7: 17" 5 Spaces @7'-7' 5 Space 4 Spaces @ 7-7" <u>a</u>-75'3" <u>60'5</u>} 75'3' 266-94 SPAN (3-4) SPAN (1-2) SPA N (2-3) PLAN OF SIDEWALK SHOWING FENCE POST SPACING 4"Gap for splice Provide Ventholes at all 2"40.D. x10" -Fabric ties e abt 15"cts. internatic closed joints for galvanizing. € 2°¢Pipe z' + Pipe 24 -<́т<u>ч</u>р *W* End Post pectyp.) G'Fabric Stretcher Bal-Band Stretcher Bar (Typ.) Stretcher Ъ 2"4 Pipe (Tgp.) Bar Band (Typ.) ัง่ Stretcher Bar Je.YV 2" & Pipe -z"& Rod Outside Face 19 of Sidewalk Grout (2 Mini) (Typ)-- & 2" & Fence Fost ≺тур. 6 Floor 12-T'x 6"x2"(Typ) J.V END OF FENCE PULL POST SECTION THRU PART ELEVATION OF PEDESTRAIN CHAIN LINK FENCE FENCE E 2" & Pipe Post a V Note: Note: Pedestrian guard Fence (chain link type) shall be in accordance with Section 1043 of the Std. Spec., except all fabric shall have top and bottom edges. Knuckled. All rail posts shall be vertical. Grout of 2 minimum thickness shall be placed under floor plates to provide for vertical alignment of rail posts. The contract unit price per linear foot for pedestrian guard fence (galvanized) shall include furnishing and receting the fence complete with anchor bolts and washers. Floor A-7"×6"×2" - 2 2 & Pipe Fost E Grout (2"Min.) in V Measurement of pedestrian guard fence shall be taken parallel to grade through the centerline of posts. 2" PHON ×9" R-71×61×2"-۵. - 2 2-2" + U bolts with hex nuts ; washers (all galv.) Weld --14 ې slotted holes RAIL POST CONNECTION (TYPICAL) for 2" & U bolts DETAILED MAY 1979 PLAN OF FLOOR PLATE CHECKED JULY 1979

Note: This drawing is not to scale. Follow dimensions

Sheet No. 13 of 21





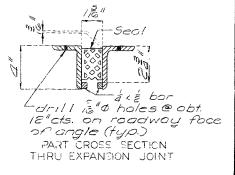
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# A16433, Sht. 27

FED. ROAD	STATE	FED. AID	FISCAL	SHEET	TOTAL
DIST. NO.		PROJ. NO.	YEAR	NO.	SHEETS
\$	MO.		19	44	

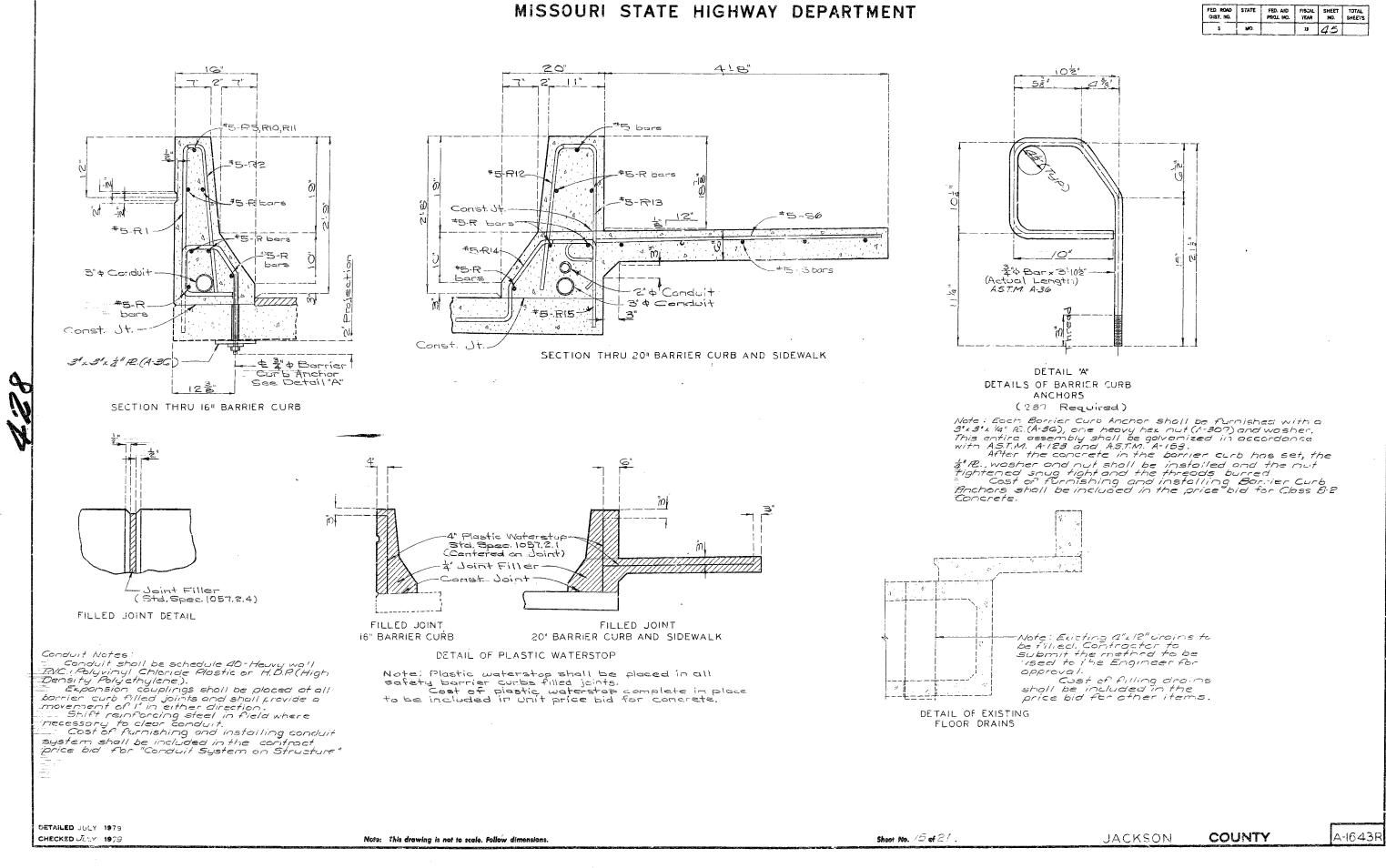
Structurel steel for Exponsion Device shall be fobricated in one

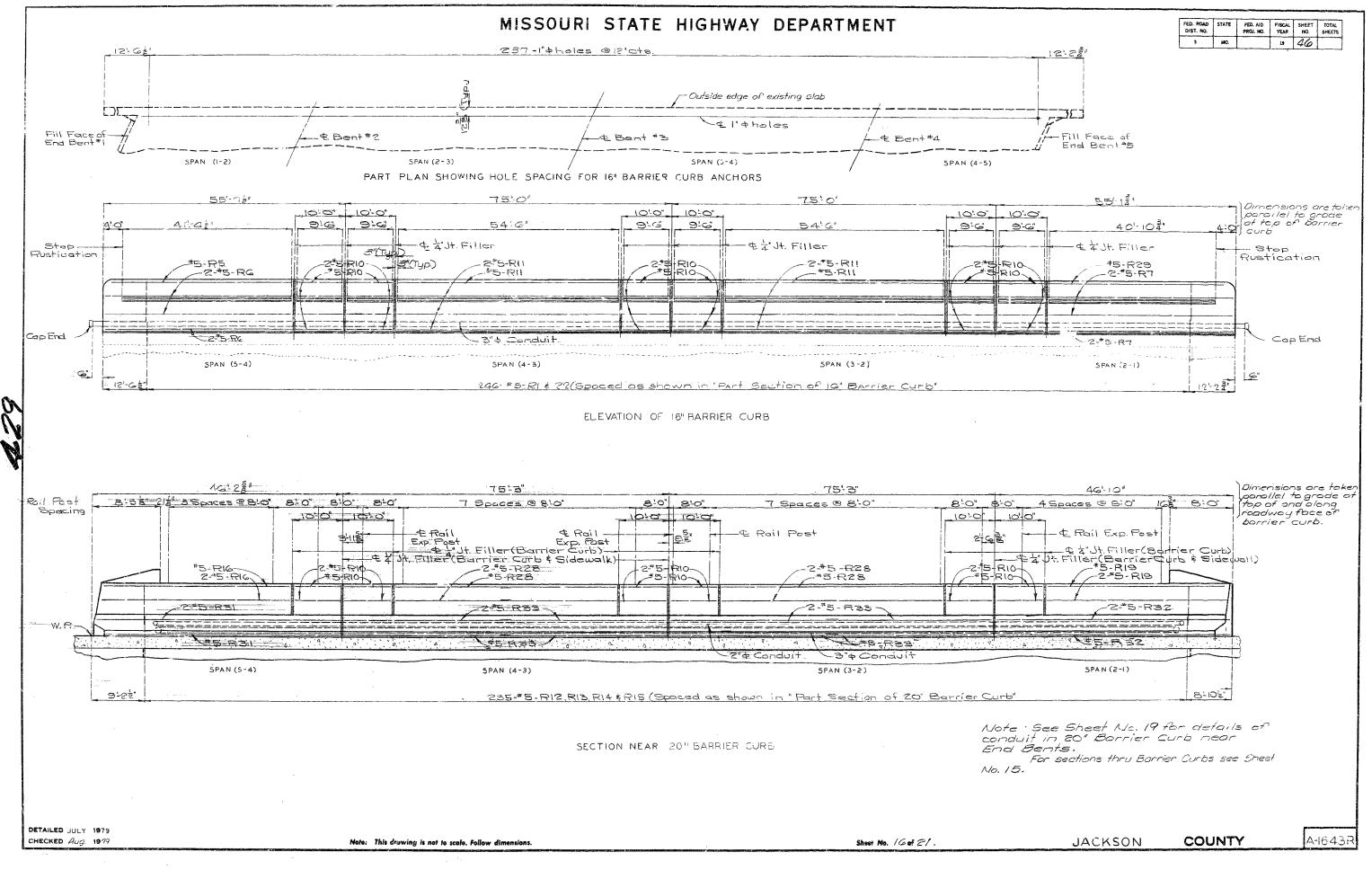
Approved stud welded anchers may be used in lieu of #5-wars shown.

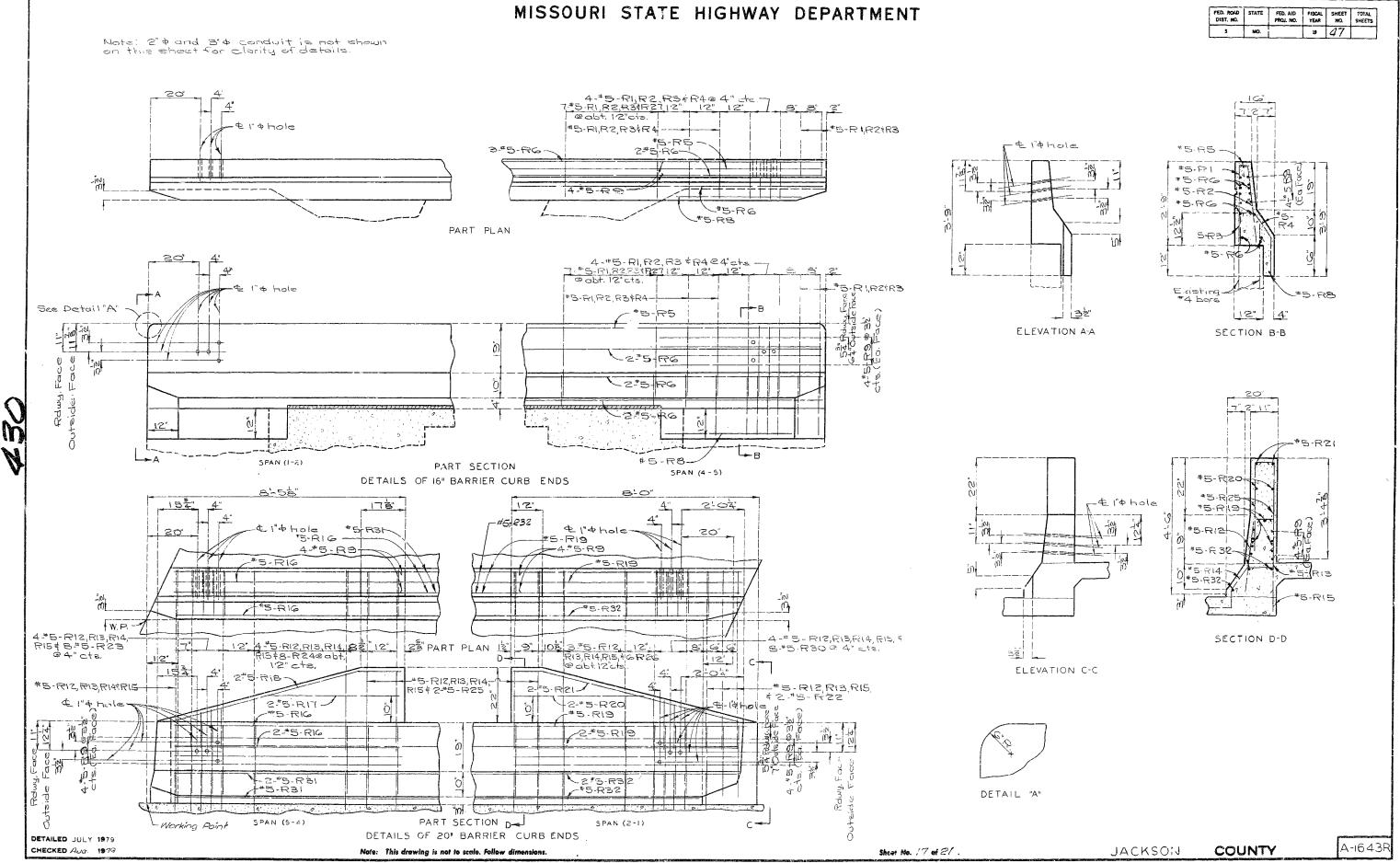


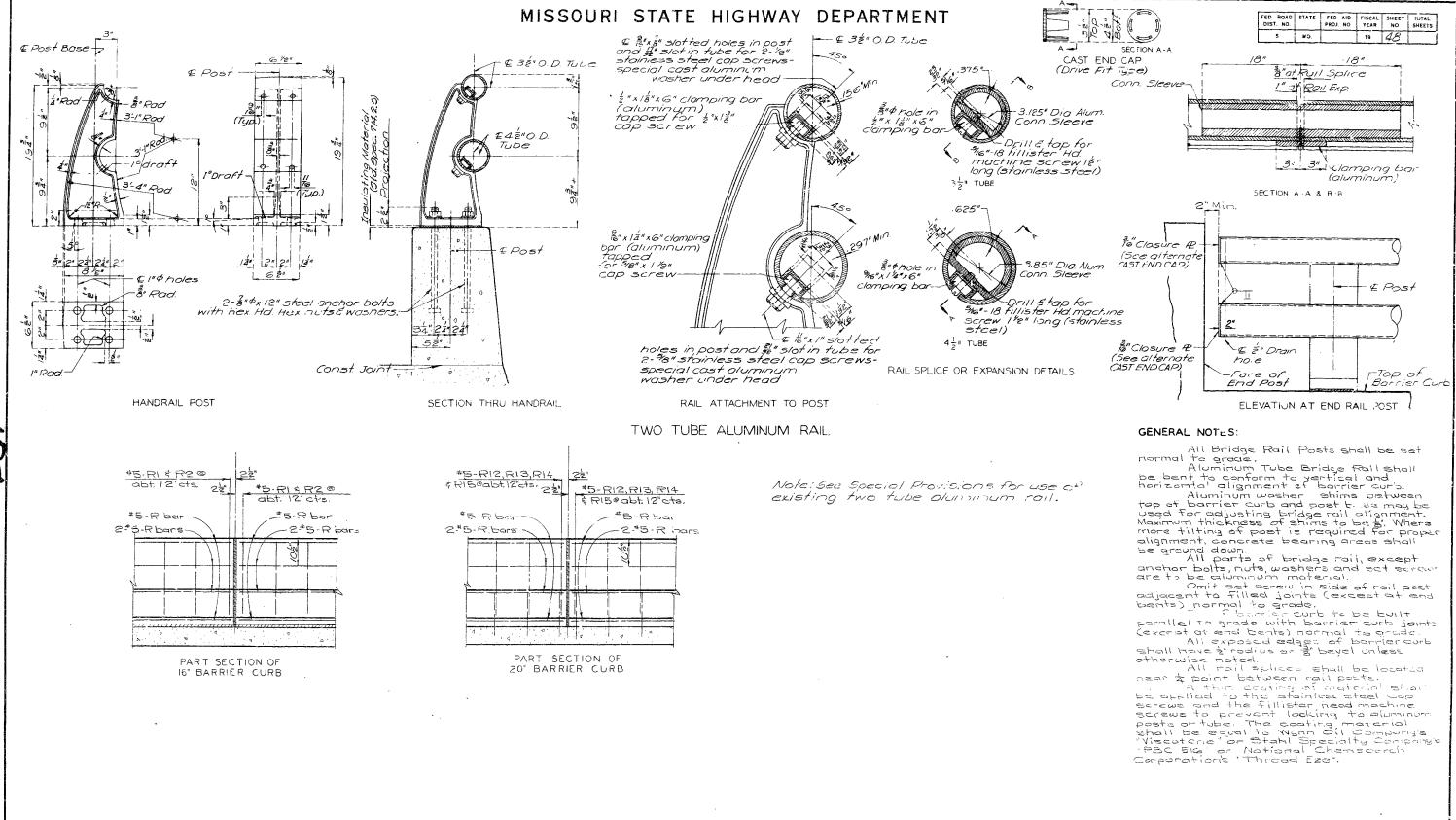
E OF	ELONGITUDI	NAL SEALS	
VE SI	ZE AT 60°F.	SEAL	SIZK
н	HEIGHT	WIDTH	HEIGHT
6″	2-3/4"	2"	2-1/16"

LON	GITUDINAL (INCHES)	SEAL TOLER	ANCES
0	"B"	"C"	"D"
	(HEIGHT)	(SHELL)	(webs)
	2.0625	0.125	0.094
	+.125	T.030	+.030
	125	T.015	015









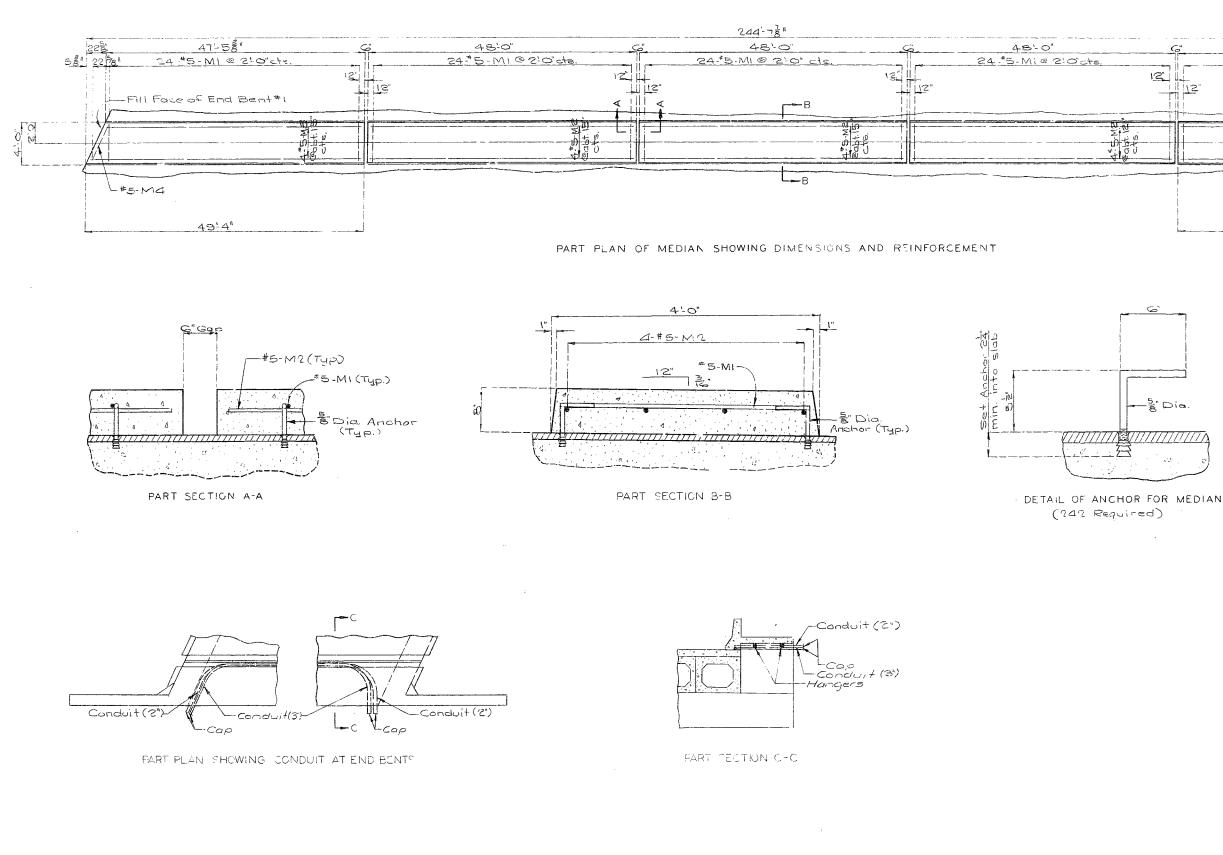
#### A16433, Sht. 31

JACKSON COUNTY

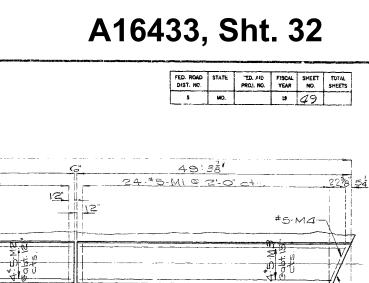
A-164

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#### MISSOURI STATE HIGHWAY DEPARTMENT



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Note: Anchors Shall be of the multi-set, steel flange drop in type. Cost of furnishing and installing hook anchor bolt assemblies shall be included in price bid for Class B2 Concrete.

Fill Fuse of End Bent #5

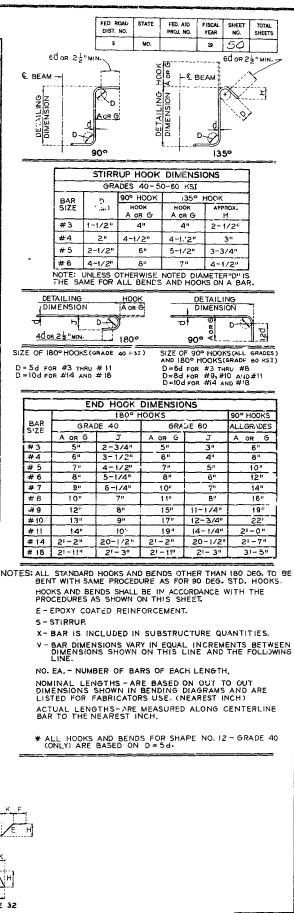
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COUNTY JACKSON

A-1643F

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REQD.	<u>NO.</u>	LOCATION	PE NO	SUP C	E ACH	В	l c		D	E		F	н		к	NOMINAL LENG TH	ACTUAL. LENGTH	WEIGHT	REQD.	NO.		192	1	ц 3 8	c	D	E	F	н	ĸ	NOMINAL LENGTH	LENGTH
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_		SUBSTRUCTURE	$\left  \right $	+++	┝┤		┢- ──	_											╢──	2 671	WING	25 <u>S</u>	╞╢╇	2 0.500	11 5.375	3 4.000			5 6.000	10 0.500	16 10 1	6 9
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	7H10 9H11		1			4 0.000 27 6.000	2 9.5			<u>†                                    </u>						96 300	30 O	31	3 20	6 6V4 0 6V5	BACKWALL	20 20	<u> </u>	5 1.000 2 2 5.000	<u> </u>	<u> </u>					51	5 1 2 5
	9H12 6H13		18 20	X X		25 7.000						·	+			28 1 25 7		382 154		+ 6V6	INCR = 6.500 IN	20	$\mathbb{H}$	7 4.000				<b> </b>			74	
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14	603	BEAM INCR = 0.375 IN		s x v	2	2 11.000			11.000							13 6 13 4		272	<u>-</u>	<u> </u>	INCR = 4.000 IN END BENY NO 5	┝╋╌┾╴	┟┼┼		3 3.000	2 6.000					9 0	8 10
40	604	BEAM INCR = 0.375 IN		S X V	2	2 11.000	3 1.6		11.000	3 1.		-				13 3 12 10		750		6F2	BEAM & WING BEAM	23 S 20		14.250	3 7.500	14.250	12.000	7.750	12.000	7.750	6 0	
14	605	BEAM		s x v		2 11.000	2 10.7	50 2	11.000	2 10.	750					12 10	12 4			5 4H4	BACKWALL	20		28 11.000							28 11 2 28 11 2	
8 1	079	INCR = 0.500 IN COLUMN BT. 2	17	x		2 11.000 20 5.000	2 9.3	75 2	11.009	2 9.	375		┼──			12 7 21 10		251 752		5 6H6 5 4H19	WING BACKWALL	20 21 S		13 6.000	22.000	18.000		<u> </u>	2 3.000	12.500	13 6 1 5 10	
	0V10	COLUMN BT. 2 COLUMN BT. 4	17	x	1 1	21 5.000 20 2.000				ļ			<u> </u>			22 10 1 21 7 1		786		6H9	BACKWALL	21 \$		2 5.750 2 3 11.500	4 10.000				2 3-000	12.500	7 4	
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16 1		FOOTING & COLUMN	17	X		7 10.000	3 6.00		*			-			1	9 3 14 3 1		631		402	BEAM			1 2 6.000		2 6.000	2 9.000				11 3 1	
12	403 504	FOOTING FOOTING	10			5 9.000	3 0.04		3.000							5 9		84 72		402	INCR = 0.500 IN BEAM	10 S			3 3,750 2 9,000	2 6.000	3 3.750				12 5 1. 8 0	
8	805	FOOTING	_	×		8 0.000										8 0	P. 0	171	-	671	INCR = 4.000 IN BACKHALL	19 5		7 5.000	5 3.000 4 3.000	2 6.000					90 11 81	
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	6H13 9H14		20	X		25 7.000		1					t	-+-	Ì	25 7 2	25 7	77 609	1 20	647	WING INCR = 7.000 IN	20	-14-	2 4.500							2 5 2	
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51	5U6	BEAM	13			2 11.000	3 3,12	25 2	11.000	3 3.	125					13 6 1	3 1			5H17 5H18	DIAPH	23 5 21 S	_	10.750	23,000				9.750 10.875	4-625	2 10 2	
8 1		INCR = 0.250 IN COLUMN	17			2 11.000 21 3.000	2 9.25	50 2	11.000	2 9.	250					12 7 1 22 6 2		964					11								<u> </u>	
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+			┽┼	┽┼┨	-+									_ <u> </u>		<del> </del> -			8	5M3	MEDIAN INCR = 6.625 IN	1	V i	2 47 3.500 48 11.500							47 4 4	
-		SUPERSTRUCTURE		$\prod$	T			1											2	5H4	MEDIAN	20	-	3 21.000							3 11 3	3 11
1		END BENT NO 1						+-			Ť								- I j	5R1	16 BARRIER CURB			2 7.000	3.500						2 11 2	
6	6F1	BEAM & WING	23	5	-+	14-250	5 6.75	50	14.250	7.	750	12.000	7.	750	12.006	7 11	7 11	71		5R2 5R3	16 BARRIER CURB 16 BARRIER CURB	15 S	++	2 7.125	3.500				2 7.000	3.000	2 11 2	2 10 16
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1	4113	BACKWALL					4 10.00				_								6	586	16 BARRIER CURB	25		45 4.000							45 4 45	5 4
6	4H4	BACKWALL	20		2	28 11.000		+					2 3.0			28 11 2	1	15 116	· · · · · · · · · · · · · · · · · · ·	5R7 5R0	16 BARRIER CURB 16 BAFRIER CURB			44 10.000 3 9.000							44 10 44	3 9
		BACKWALL WING	21 2	s		2 5.750 13 6.000	18.00	00	22.000	<u> </u>			2 3.0	900	12.500	5 10 13 6 1		11		5R9	END PUSTS BARRIER CURBS	20	╉╋	7 5.000							757	
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		SHAPE 8 SHA	PE 9	SHA	PE I	O SHAP	ц •ЕП ж	¥SHAF	CI PE 12 SI	D HAPE 13	н	N <sup>B</sup>		2 E	в	VERTICAL LEG		SHAF	E 18 2			PE 22	B	C C H	SHAL	PE 25 SHA	PE26 SH	<u>C K</u> 1498 27	SHAPE 28	SHAPE	29 <sup>B</sup>	Рв
												K		F		C		SHAF	E 20	2	SHAPE 21		L.L	K D K								PE 31 S
DETA		AUG <b>19</b> 79 Alg <b>19</b> 79										56	APE 15		SHAP	- C 13				BEND	NG DIAGRAMS		5	HAPE 24							SUAL	
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#### A16433, Sht. 33



ACKSON

COUNTY

A-1643R

SHAPE C

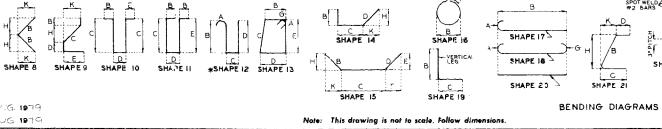
SHAPE 7

DETAILED AUG. 1979

CHECKED AUG 1979

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	u X	LOCATION	γXQ	PE	4 D B C B C B	SUBSTR.	EAC	L	В		С	D		E			F	н		к	NOM	ACTUAL	WEIGHT
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4		SIDEWALK		17	+	+		5	2.500										-		5 10	510	147
		SIDEWALK	1	17	1			<u> </u>	11.000												6 6	6 6	1
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		SPOT WELD #2 BARS	~																				



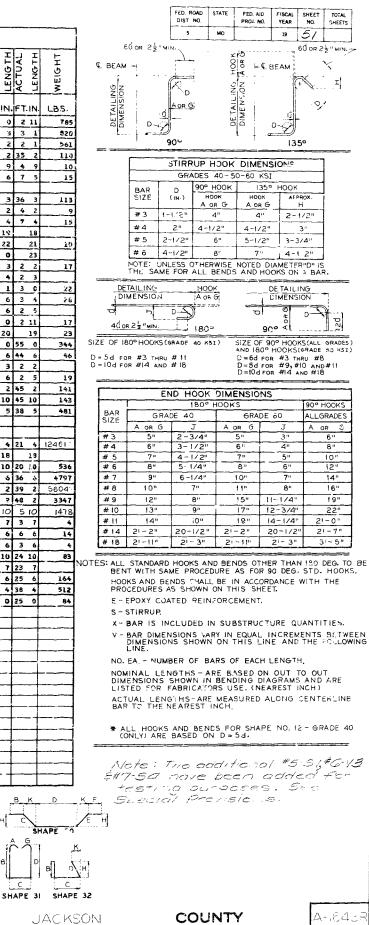


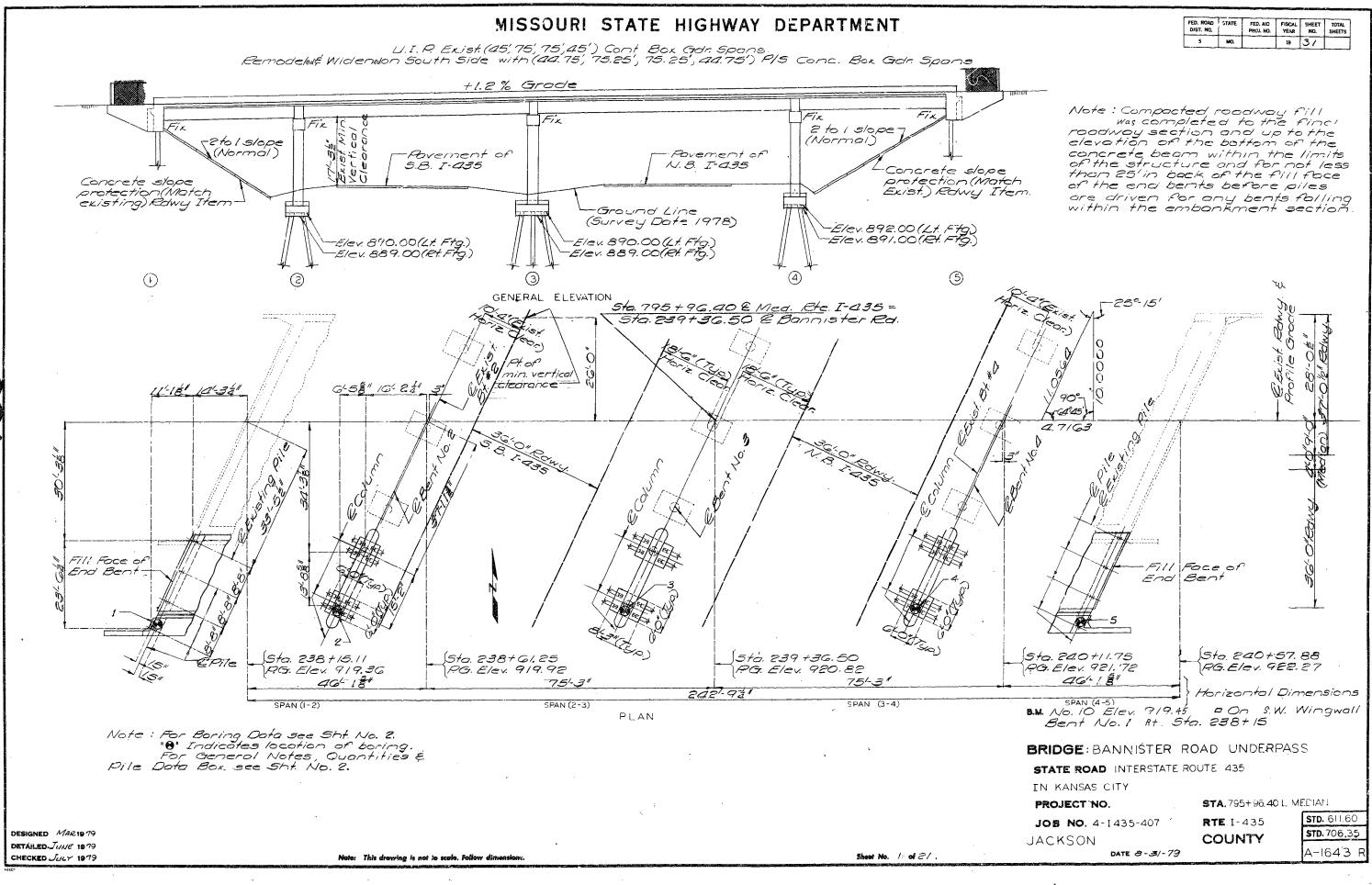
SHAPE 25 SHAPE 26 SHAPE 27 SHAPE 28 SHAPE 29

SHAPE 22

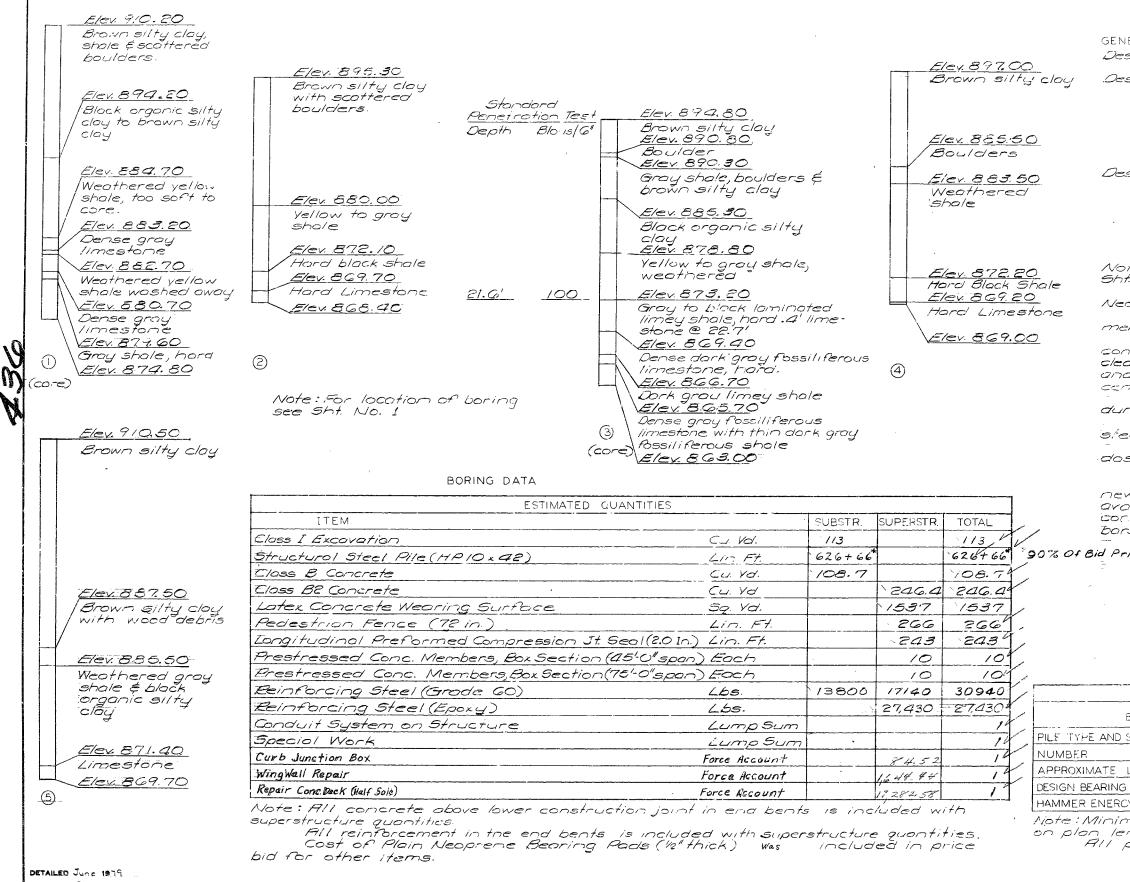
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SHAPE 24





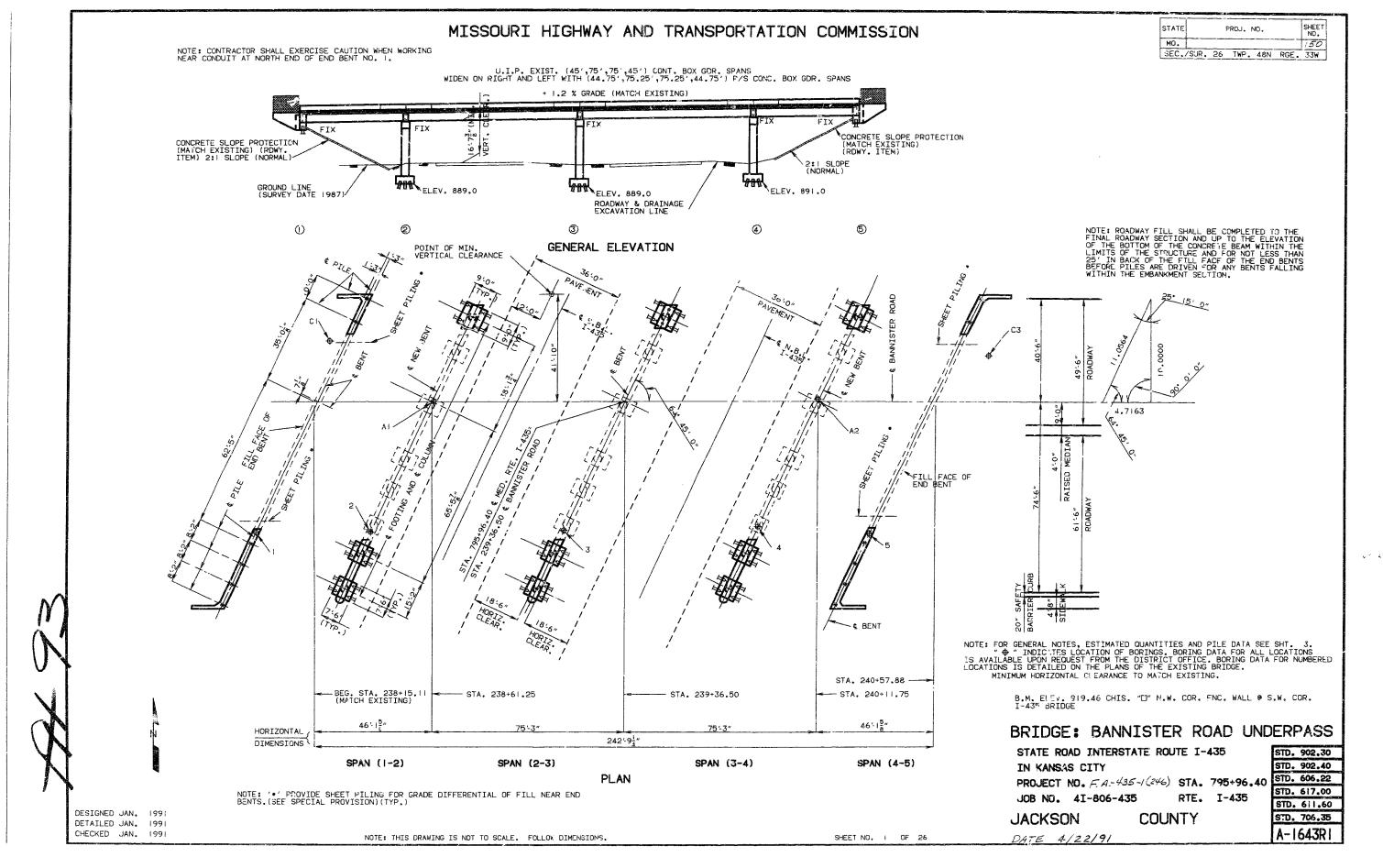
#### MISSOURI STATE HIGHWAY DEPARTMENT

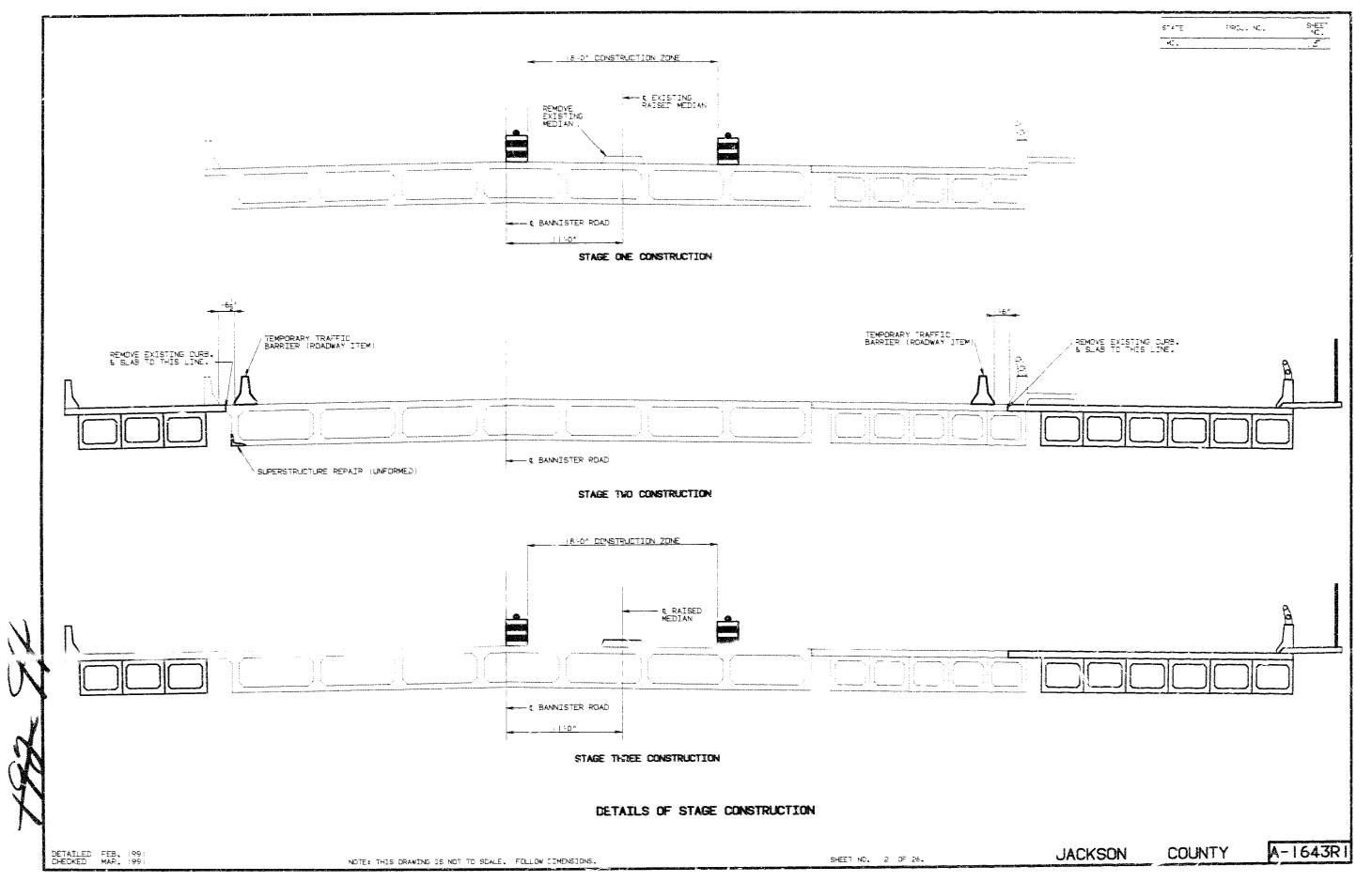


CHECKED JULY 1979

Note: This drawing is not to scale. Follow dimensions

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BENT NO.	1	2	3-L.T.	З-RT.	4-L	.T. 4	-RT.	5-LT	5-RT
SIZE	HP!Ox42	HPIOx42		HPIOx40	1190	×42 HP	10,42	HPiOx4	eHPIOx42
	3	8	5 N	5	4		2	1	20
LENGTH FI 5 TONS		11 56	21 £3	15  E3	20 EG		6	<u>26</u> 47	32
CY ROD. FT.L.B.	paco	13200	12500	12,500	13,20	2013	200	19600	10,500
num en ength on pile w	ergy d de las	requ sian driv	iirer beo en t	nent ring to pr	0f 70 20f	' hai Nue Nca	07 07 1 re	er i pili efus	bosed es. sol.
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TTFM		SUBSTR.	SUPERSTR	INTAL
REMOVA' & STORAGE OF EXIST. BRIDGE RAIL	LIN. FT.	100001	226	226
CURB REMOVAL (BRIDGES)	LIN. FT.	<u> </u>	747	747
PARTIAL REMOVAL OF SUBSTR. CONC.	LUMP SUM	T	+	1
FARTIAL REMOVAL OF EXIST. BRIDGE DECK	SQ. FT.	1	1184	: 184
CLASS   EXCAVATION	CU. YD.	330	1	330
SHEET PILING	LUMP SUM	1		1
(72") PEDESTRIAN FENCE (STRUCTURES)	LIN. FT.	1	263	263
STRUCTURAL STEEL PILES (10")	LIN. FT.	444		444
TRUCTURAL STEEL PILES (12")	LIN. FT.	740	1	740
CLASS B CONCRETE (SUBSTR.)	CU. YD.	190.1		190.i
SUBSTR. REPAIR (UNFORMED)	SQ. FT.	70		70
SUPSTR, REPAIR (UNFORMED)	SQ. FT.	1	10	10
CLASS B2 CONCRETE (SUPSTR) CONC ON BOX O	SDR CU, YD	1	295.6	295.0
16" SAFETY BARRIER CURB	LIN. FT.		264	264
20" SAFETY BARRIER CURB	LIN. FT.	T	242	242
RAISED MEDIAN BARRIER	SQ. FT.	Ī	971	971
SIDEWALK (BRIDGES)	SQ. FT.		1133	1133
PLAIN NEOPRENE BEARING PADS	EACH		18	18
AMINATED NEOPRENE BEARING PADS	EACH		54	54
P/S CUNC. BOX GDR. (45' SPAN)	EACH		18	18
VS CONC. BOX GDR. (75' SPAN)	EACH	1	18	18
REINFORCING STEEL (BRIDGES)	POUND	25740	8260	34000
CONDUIT SYSTEM ON STRUCTUPE	LUMP SUM			1
EINFORCING STEEL (EPOXY COATED)	POUND		65690	65690
RIDGE RAIL RELOCATED	LUMP SUM			1
			<u>  </u>	
······································				

NOTE: ALL CONCPETE ABOVE LOWER CONSTRUCTION JOINT IN END BENTS IS INCLUDED WITH SUPERSTRUCTURE QUANTITIES.

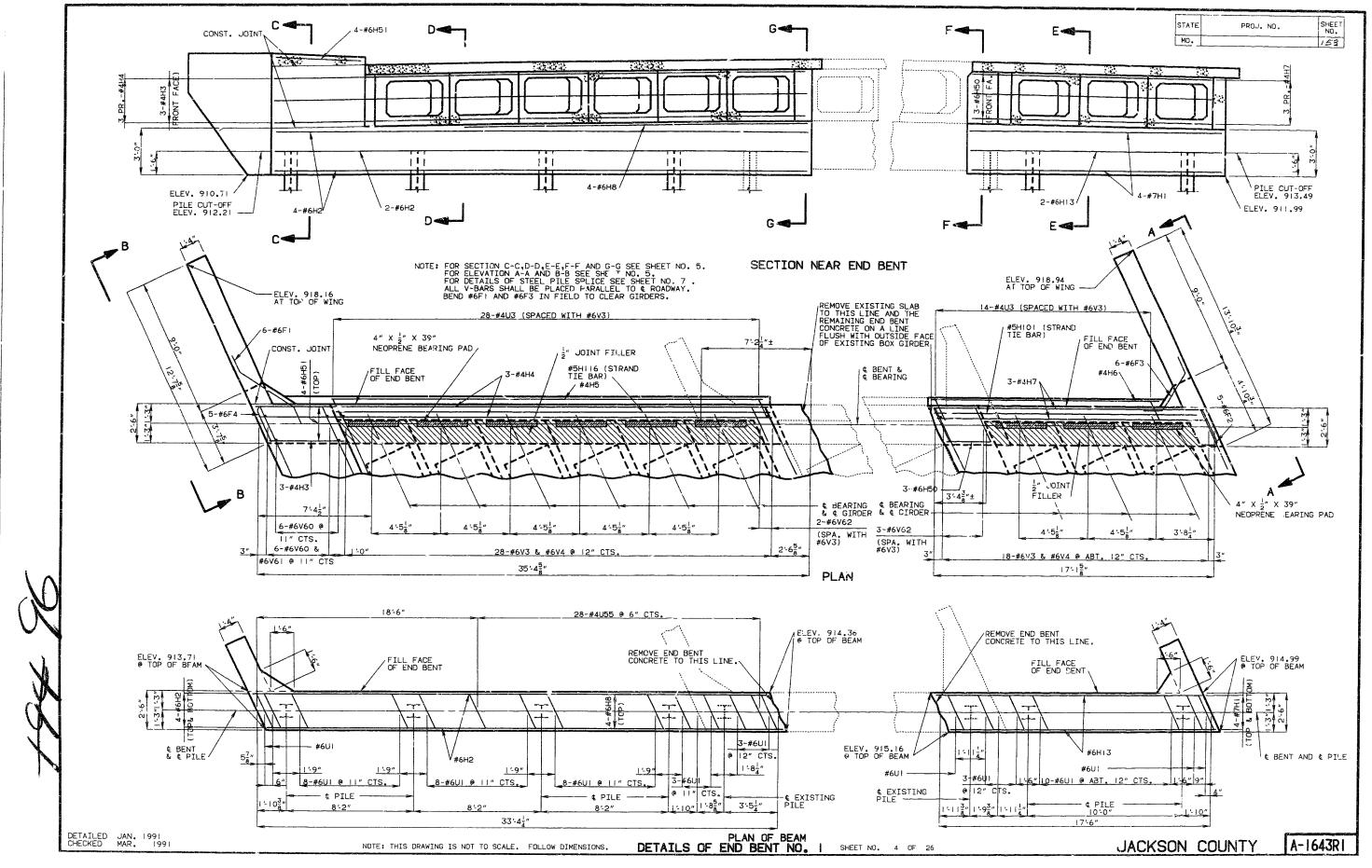
WITH SUPERSTRUCTURE QUANTITIES. ALL REINFORCEMENT IN THE END BENTS IS INCLUDED WITH SUPERSTRUCTURE QUANTITIES. THE COST & FURNISHING. FABRICATING AND INSTALLING NEOPRENE BEARING PADS, COMPLETE-IN-PLACE, WILL BE PAID FOR AT THE CONTRACT UNIT PRICE FOR PLAIN AND LAMINATED NEOPRENE BEARING PADS PER EACH. ALL CONCRETE AND REINFORCING STELL IN THE SIDEWALK ARE INCLUDED IN THE SUPERSTRUCTURE QUANTITIES FOR SIDEWALKS. ALL CONCRETE AND REINFORCING STELL IN THE RAISED MEDIAN BARRIER ARE INCLUDED IN THE SUPERSTRUCTURE QUANTITIES FOR RAISED MEDIAN BARRIER, COST OF FURNISHING AND INSTALLING RESIN ANCHOR SYSTEMS, COMPLETE IN PLACE, SHALL BE INCLUDED IN THE UNIT PRICE BID FOR CONCRETE.

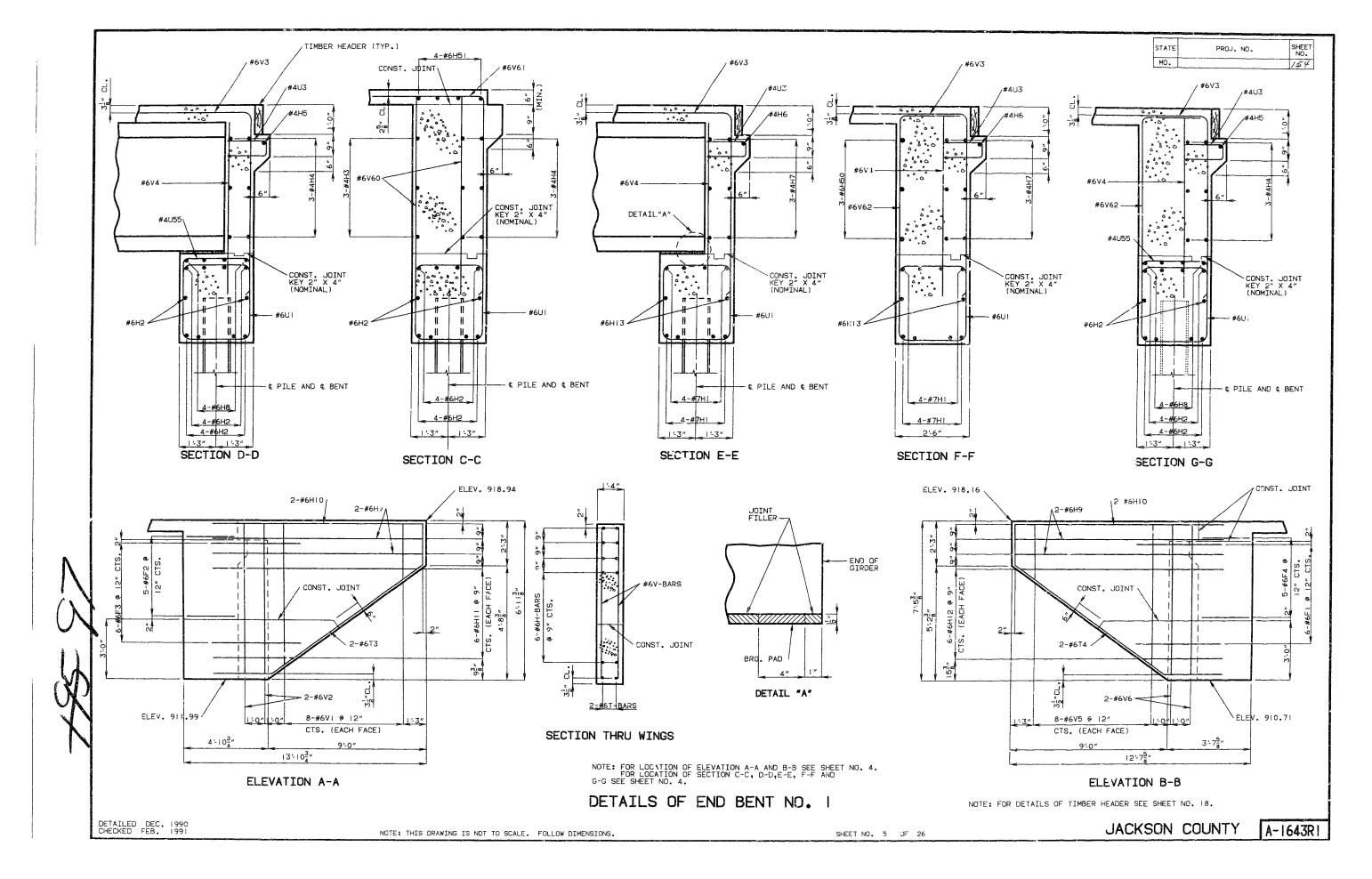
	PILE	E DATA								
BENT NUMBER	I LT.	I RT.	2 LT.	2 RT.	3 LT.	3 RT.	4 LT.	4 RT.	5 LT.	5 RT.
PILE TYPE AND SIZE	HPI0X42	HP10X42	HP12X53	HP12X53	HP12X53	HP12X53	HP12X53	HP12X53	HP10X42	HPI0X42
NUMBER	2	4	5	8	6	10	5	8	2	4
APPROXIMATE LENGTH (FT.)	32	32	17	17	17	7	19	19	42	42
DESIGN BEARING (TONS)	52	52	60	69	61	64	60	69	52	52
HAMMER ENERGY REQ'D. (FT/LBS)	11500	11500	14100	16200	14400	16400	14100	16200	11500	11500

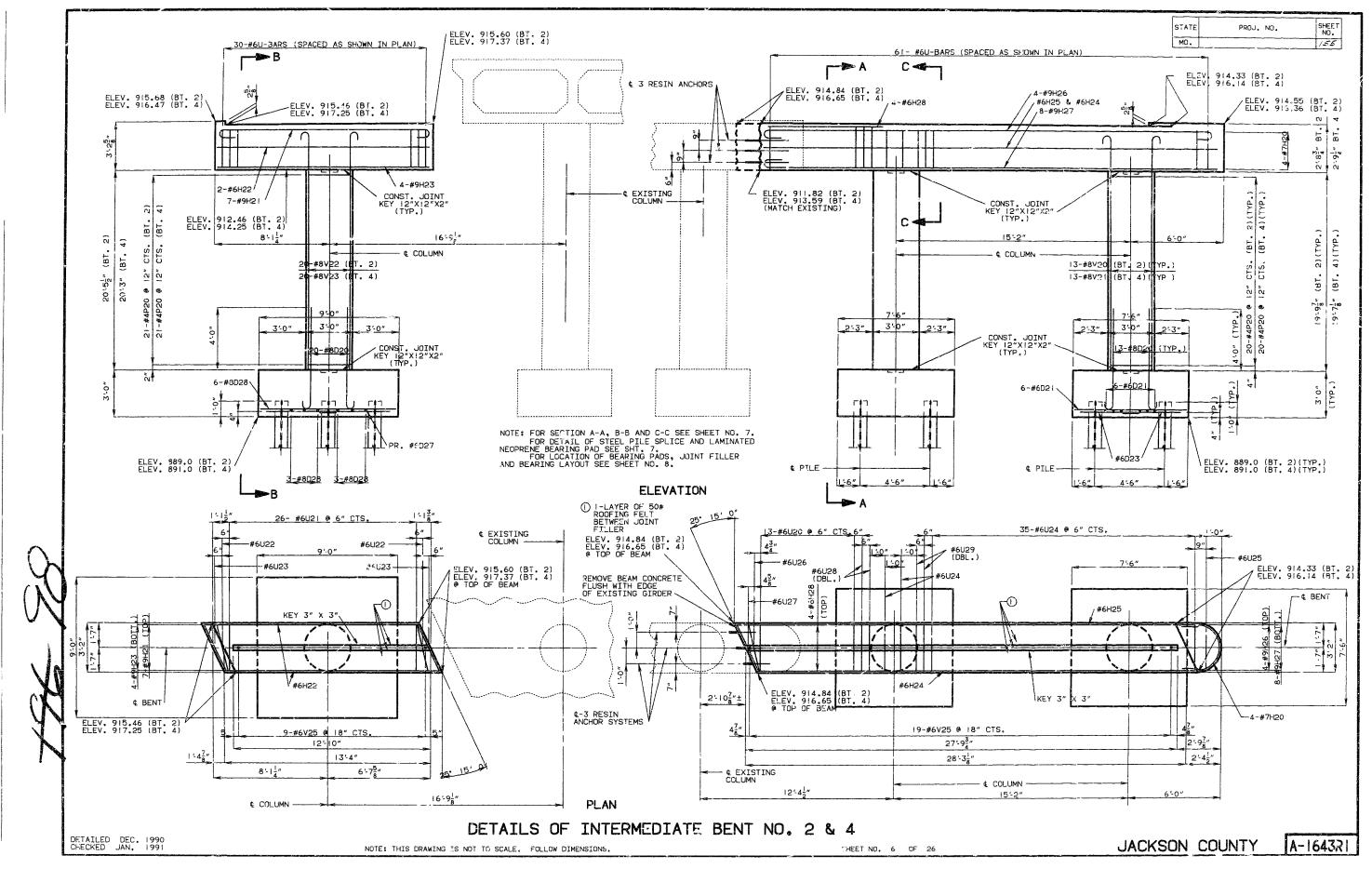
MINIMUM ENERGY REQUIREMENT OF HAMMER BASED ON PLAN LENGTH AND DESIGN BEARING VALUE OF PILES. ALL PILE SHALL BE DRIVEN TO PRACTICAL REFUSAL.

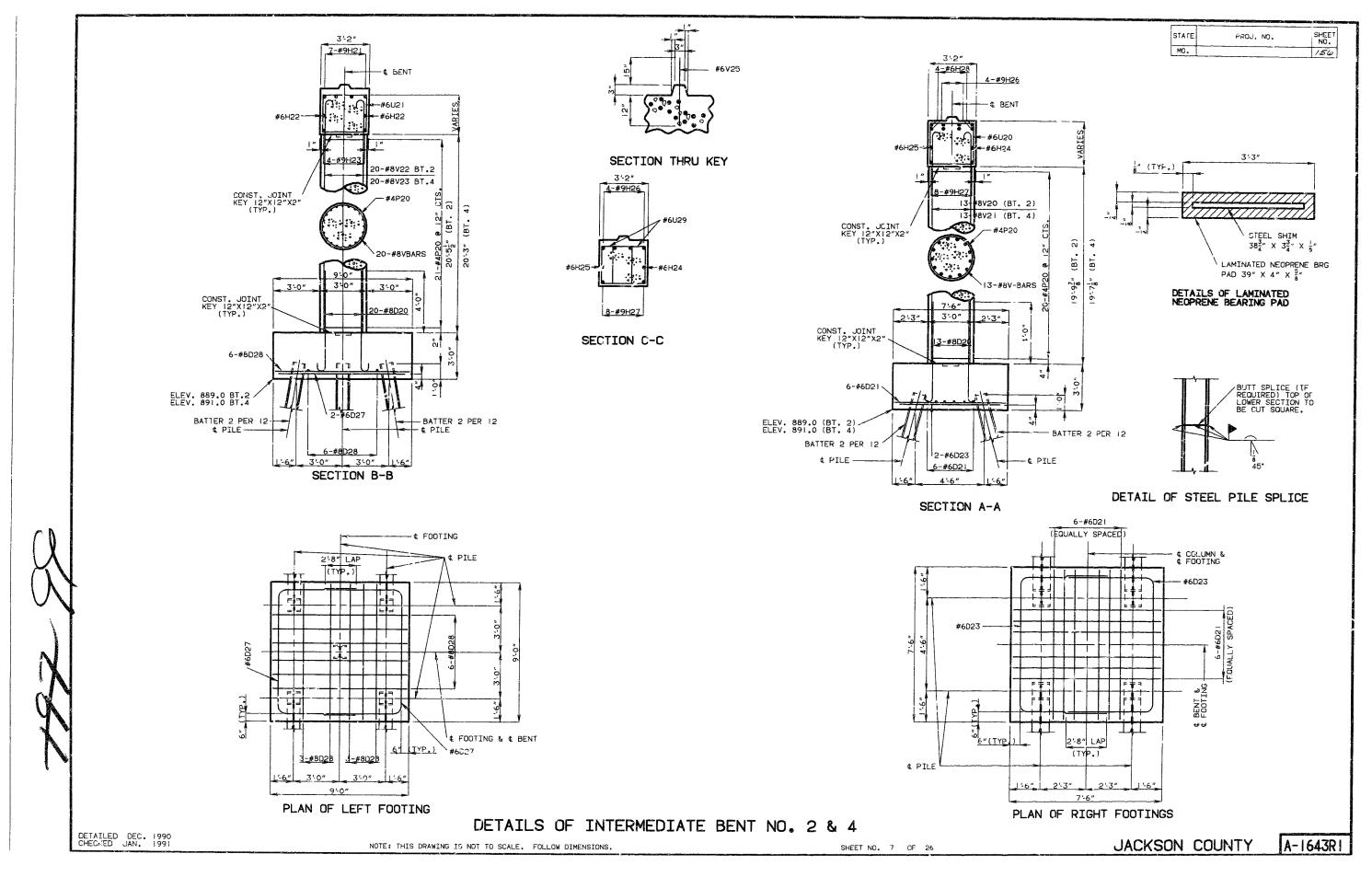
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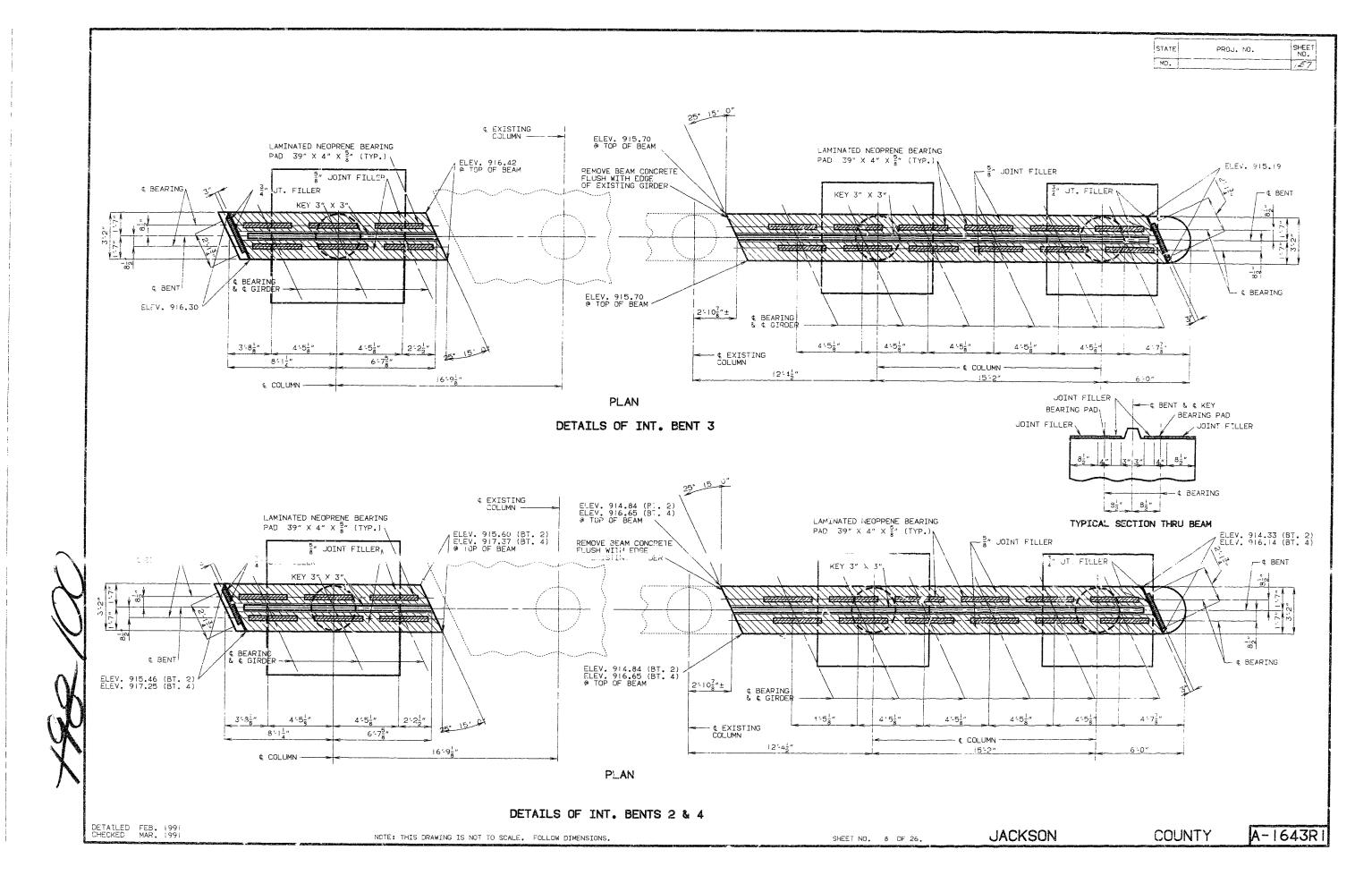
	STATE	PROJ. NU.	SHEET NO.
	MD.		152
ERAL NOTES:			
SN SPECIFICATIONS: A.A.S.H.T.OI	989 LOAD	FACTOR DESIGN.	
DESIGN PERFORM	ANCE CATEGO	PECIFICATIONS F	OR SEISMIC
-44 35# FUTURE WEARING SURFACE FIED 24,000# TANDEM AXLE			
H 120#/CU, FI., EGUIVALENT FLUID RSTRUCTURE: SIMPLY-SUPPORTED, NON	-COMPOSITE		
DNTINUOUS COMPOSITE FOR LIVE LOAD GN UNIT STRESSES:	•		
CLASS B CONCRETE (SUBSTRUCTURE)	-'C=3,000 P	SI	
CLASS B2 CONCRETE (SUPERSTRUCTUR MEDIAN BARRIER AND SAFFTY BA	E, EXCEPT P RRIER CURB)	RESTRESSED GDRS F'C=4,000 PSI	., RAISED
CLASS BI CONCRETE (SAFETY BARRIE	R CURB & RA	ISED MEDIAN BAR	RIER)
REINFORCING STEEL (GRADE 60) FY STEEL PILE FB=9,000 PSI	=60,000 PSI	, - , - , , , , , , , , , , , , , ,	
FOR PRESTRESSED GIADER STRESSES,		13 & 14,	
RENE BEARING PADS: BEARINGS SHALL	MEET THE RE		PADS.
STD. SPEC. 1057.2.4, EXCEPT AS NO FORCING STEEL: MINIMUM CLEARANCE		ING STEEL SHALL	BE
1 1/2", UNLESS OTHERWISE SHOWN.			
BARS BONDED IN JLD CONCRETE NO ND EMBEDDED INTO NEW CONTRITE WHE DLD BARS SHALL FXTEND INTO NEW CON DEFORMED BARS, UNLESS OTHERWISE NO	RE POSSIBL	E. IF LENGTH IS EAST 30 DIAMETER	AVAILABLE, RS FOR
RUCTION CLEARANCE: A MINIMUM VER (ISTING LANES AND A MINIMUM LATER) (ING LANES SHALL BE MAINTAINED DUP		ANCE OF 15'-0"	FROM CROWN
TING LANES SHALL BE MAINTAINED DU			
CONSTRUCTION. (SEE ROADWAY PLANS) TING STRUCTURE: OUTLINE OF OLD WOR			
HEAVY LINES INDICATE NEW WORK. CONTRACTOR SHALL VERIFY ALL DIMENS			
PIALS.			
ANCHOR SYSTEMS: THE CONTRACTOR S D IN THE SPECIAL PROVISIONS, THES DIG TO THE MANUFACTURER'S SPECIF PECIAL PROVISIONS AND THAT A PLAI SHALL BE SUBSTITUTED FOR THE THRE 60, REINFORCING BAR 23" LONG SHA TUD IN THE RAISED MEDIAN BARRIER.	E ANCHOR SY ICATIONS EX	STEMS SHALL BE	INSTALLED D BY THE
SHALL BE SUBSTITUTED FOR THE THRE 60, REINFORCING BAR 23" LONG SHA	ADED ROD S	TUD. AN EPOXY CO	ATED #4, THREADED
IUD IN THE RAISED MEDIAN BARRIER.			
JACKS	SON CO		-1643R1

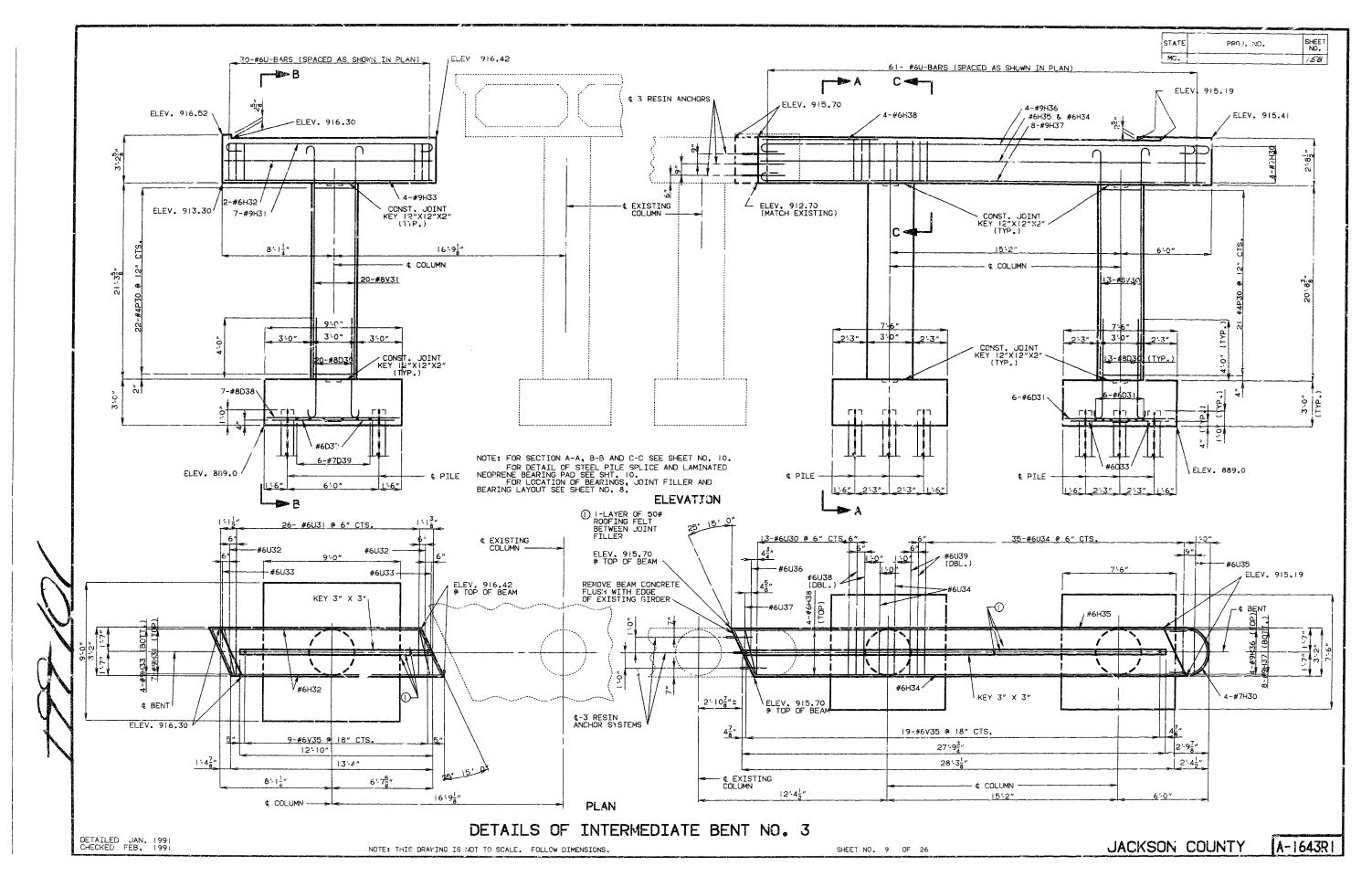


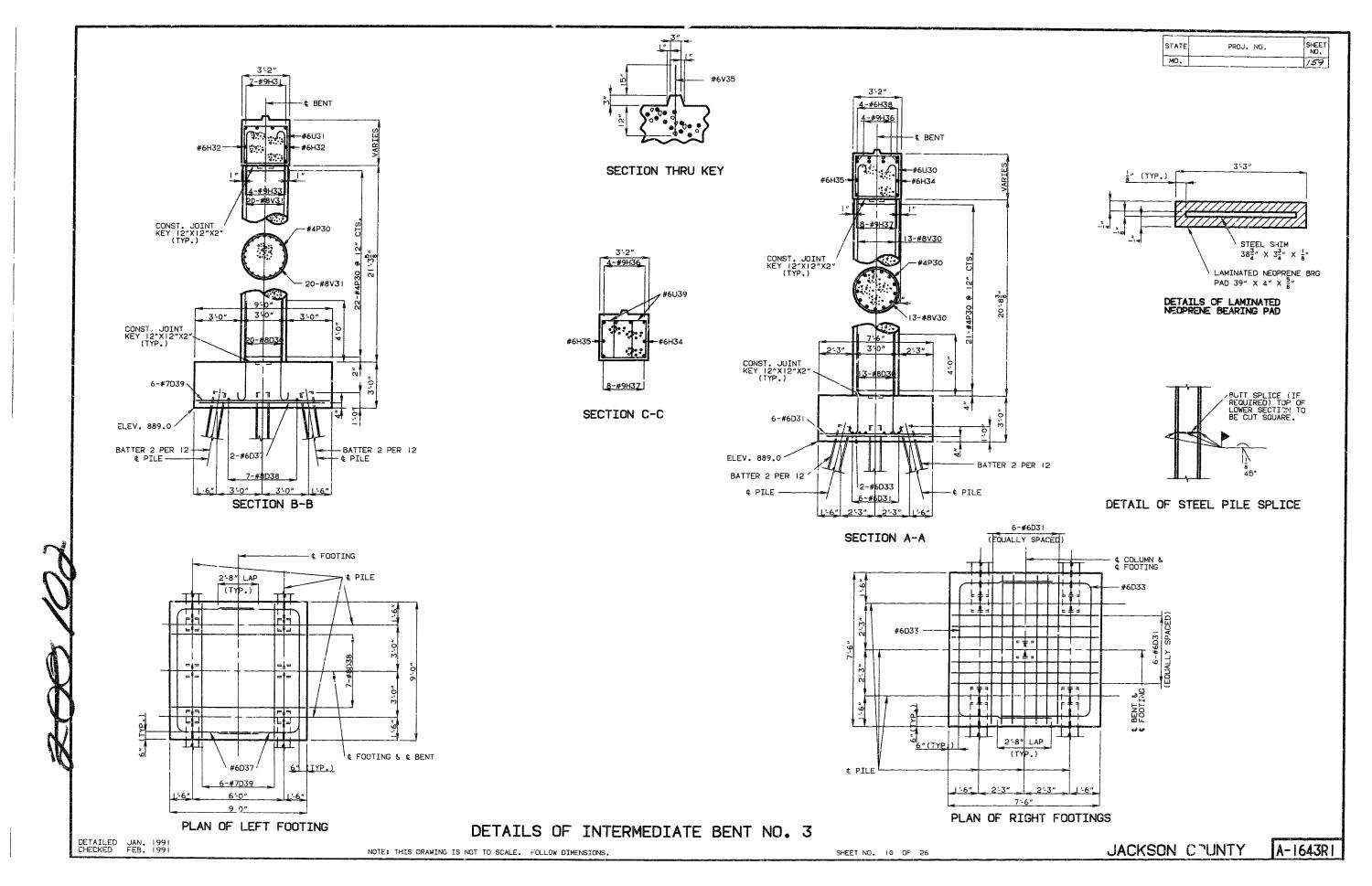


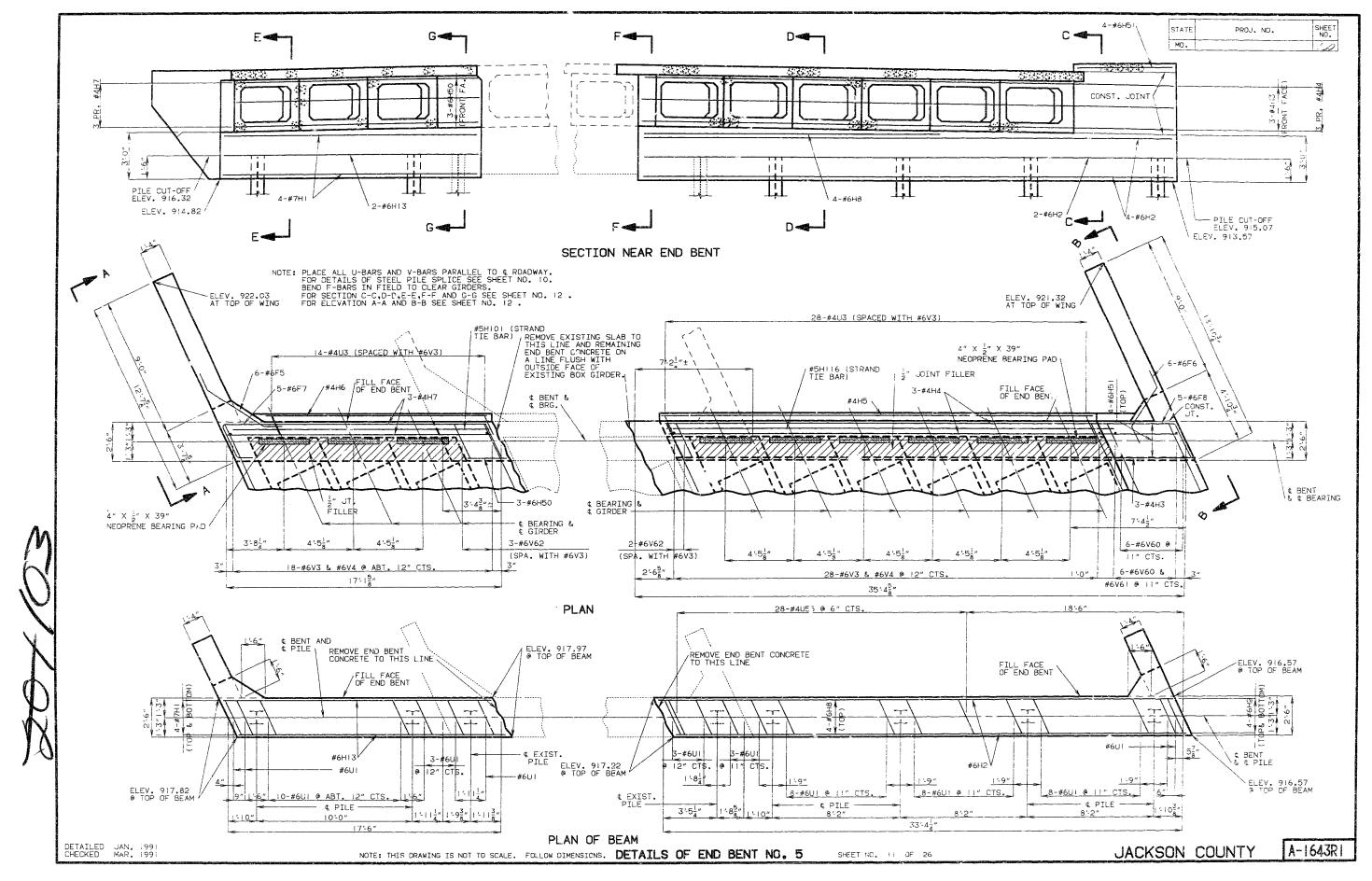


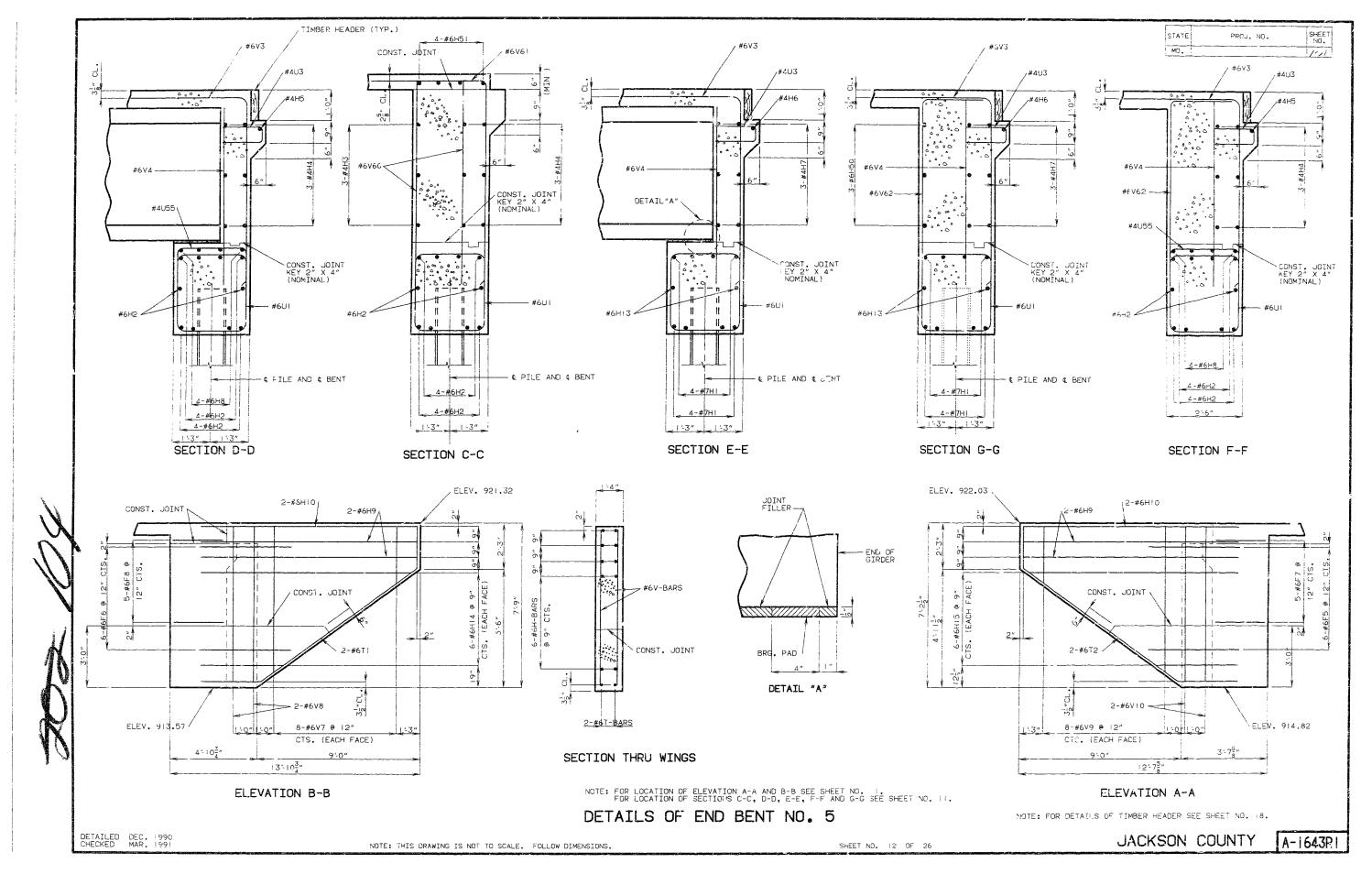


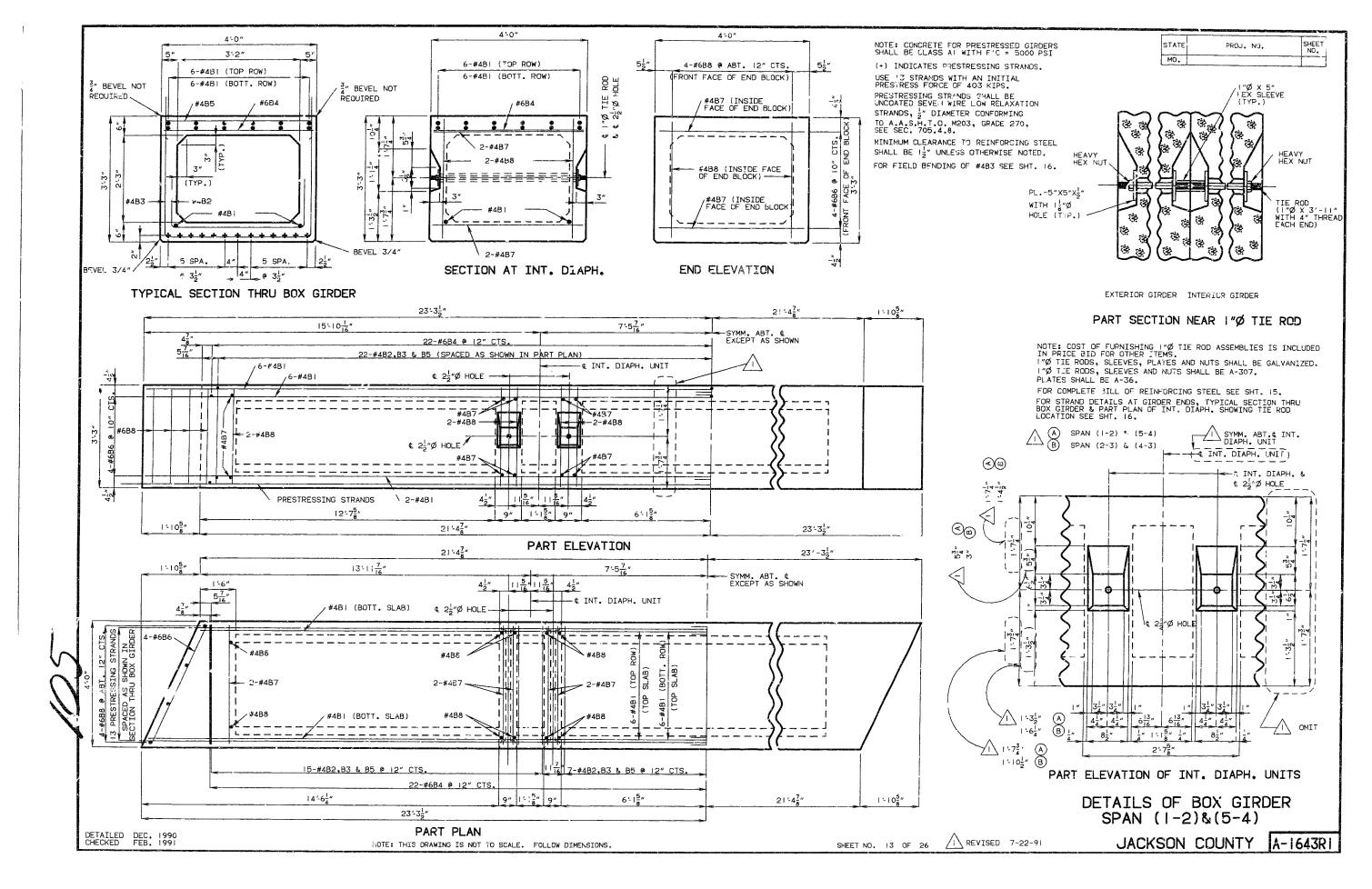


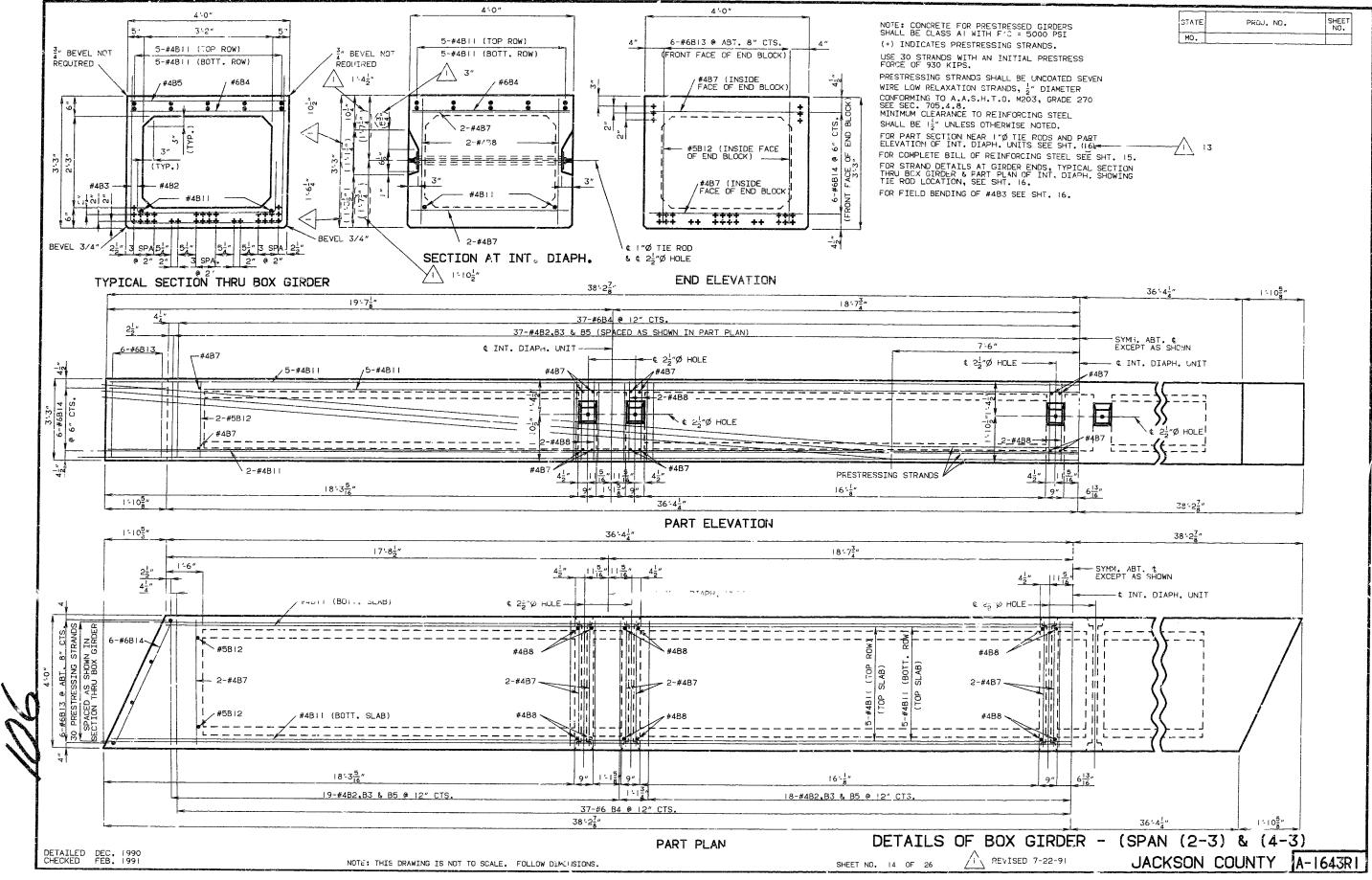










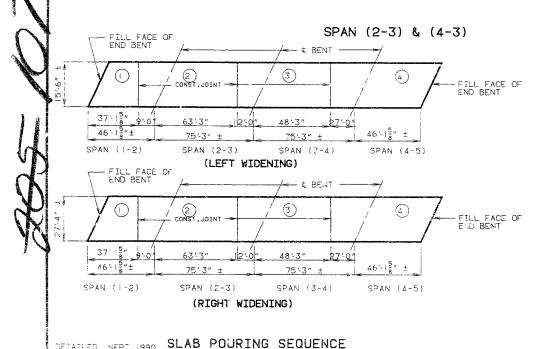


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13	482	BOX GIRDER	ÌΗ	10	S	$\square$	1	1		22.000	3	6.000			1						1 7	3	7	1	203
.3	483	BOX GIRDER	н	10	S	11	1	i		4 2.000	3	9,000			1						+2	ł	11		343
13	684	BOX GIRDER	ĺн	20	T	TT		3	9.000												3	9	3	ġj	242
13	485	BOX GIRDER	Н	10	S		1	,		20.000	3	9,000			<b> </b>						17	1	6	11	199
	6B6	BOX GIRDER	İН	20	1	ΓT	1	4	2.000												4	2	4	5	50
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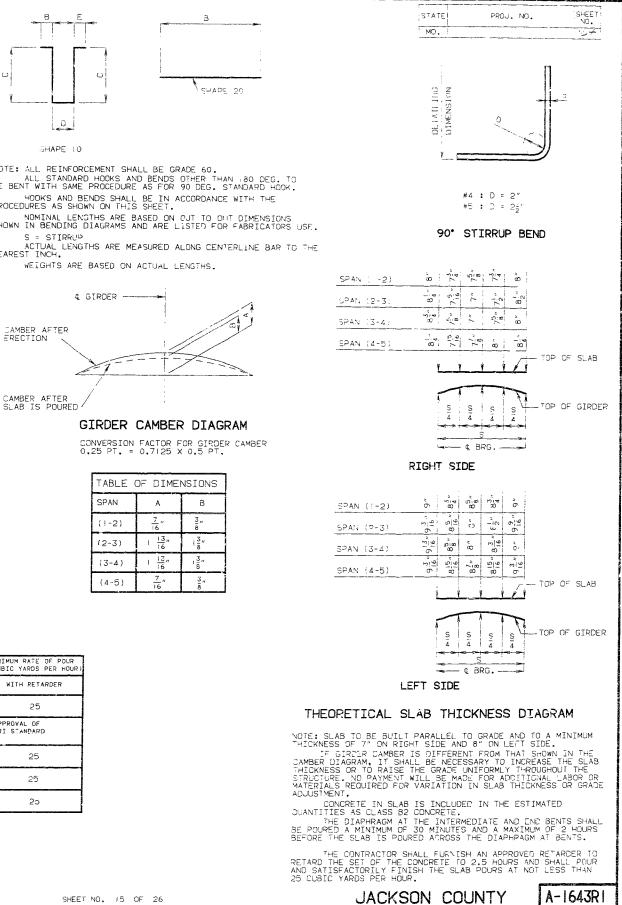
#### SPAN (1-2) & (5-4)

1		COMP	L.E	TE	5 6	3IL	L	OF	REI	NF	ORCIN	٩G	STE	EL -	EA	СН	BOX	GI	RDE	२						
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CN.	SIZE MARK		GRADE	SHAP	STIRRUP	SUBSTR. VARTES	NO.	FT.	IN.	FT.	. IN.	FT	. IN.	FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN	FT.	IN	LBS.
73	452	BOX GIRDER	ΤH		Isl	1	1	1		1	22.500	3	6.000					1				<u> </u>	3	<u> </u>	7 1	345
73	4B3	BOX GIRDER	Н	10	s					4	2.000	3	9.000			[						12	1	1		581
73	684	BOX GIRDER	H	20	Π			3	9.000			1										3	9	3	5 9	411
73	4B5	BOX GIRDER	Н	10	S	T					20.000	3	9.000			[						7	I	6	5 11	337
28	487	BOX GIRDER	Н	20	$\left  \right $			5	9.000					<u> </u>			. <u> </u>					3	9	3	9	70
24	4B8	BOX GIRDER		20				3	0.000	+												3	0	3		48
L			-		Li	1				L				L								L				
24	4B11	BOX GIRDER		20	11			38	0.000					L								38	0	3	8 0	609
4	5B12	B' X GIRDER	Н	20			<u> </u>	3	0.000													3	0	3	0	13
12	6313	BOX GIRDER	H	20	:			3	0.000													3	0	3	0	54
12	6B14	BOX GIRDER	H	20				4	2.000													4	2	4	2	75



DETAILED SEPT :990 CHECKED FEB. :991

		SEQUENC	E OF P	OUR	S	MINIMUM RATE OF POUR (CUBIC YARDS PER HOUR
		DIREG	CTION			WITH RETARDER
BASIC		2	3		1,	0.5
SEQUENCE	END TO 2	I TO 3	2 TO	4	3 TO END	25
	ER IN ACCORD					HE APPROVAL OF SSOURI STANDARD
ALTERNATE	1 + 2		3		4	25
"A" POUPS	END TO	3 2	TO 4		3 TO END	20
ALTERNATE	1 +	5	T	3 +	4	05
"E" POURS	END	TO 3	2	ΤO	END	25
ALTERNATE		1 +	2 + 3	+ 4		
"C" POURS		END	TO EN	D		20



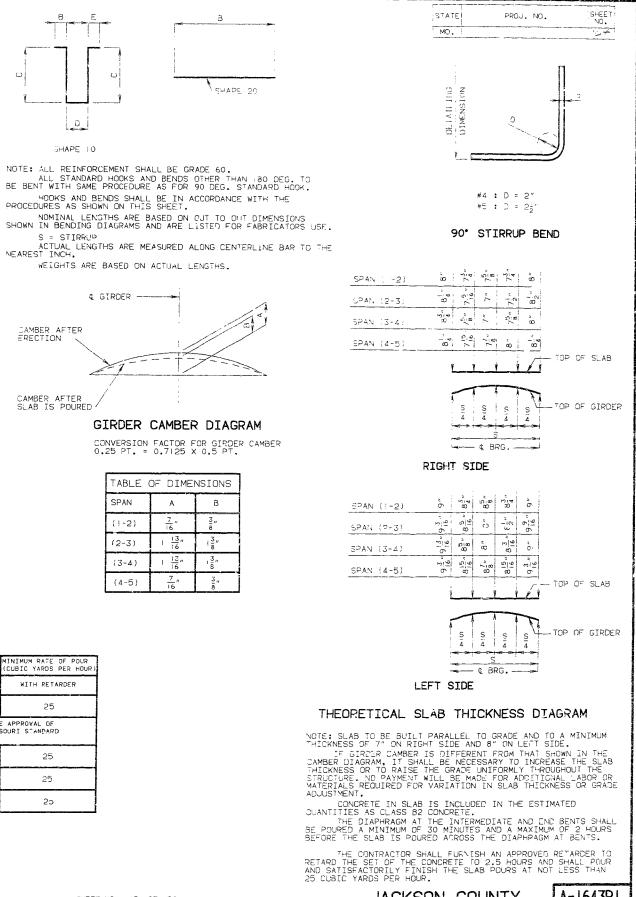
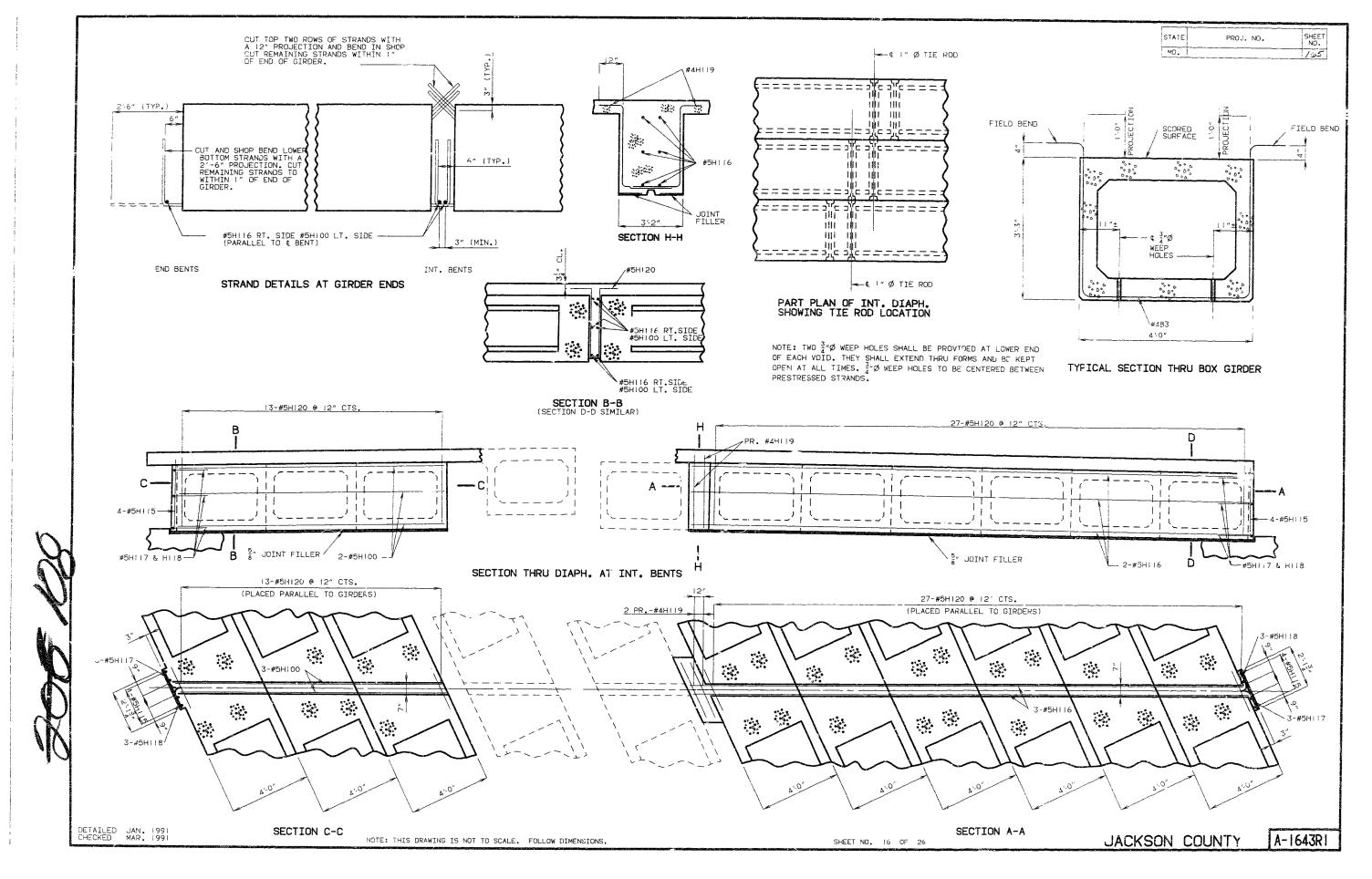
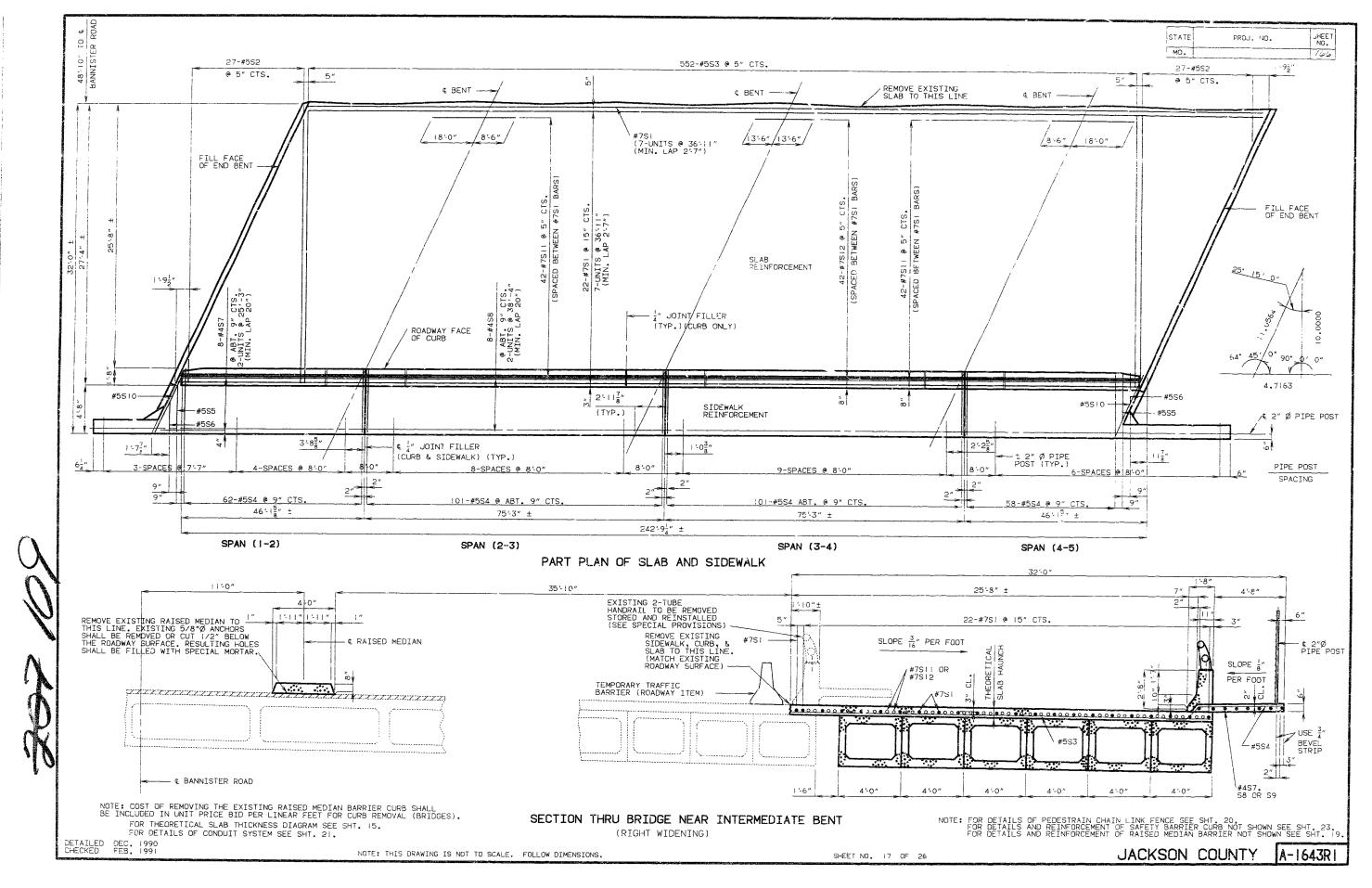
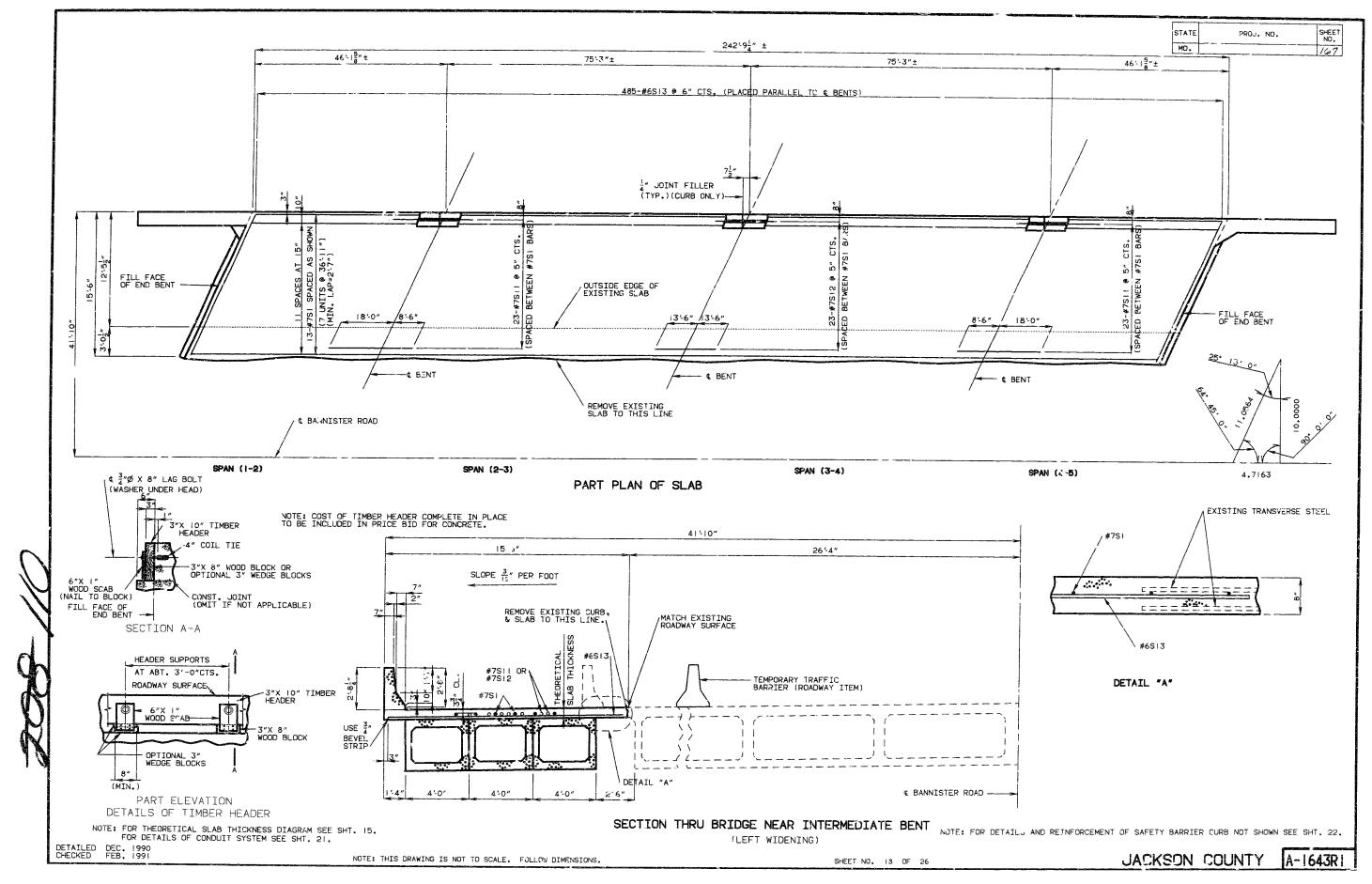
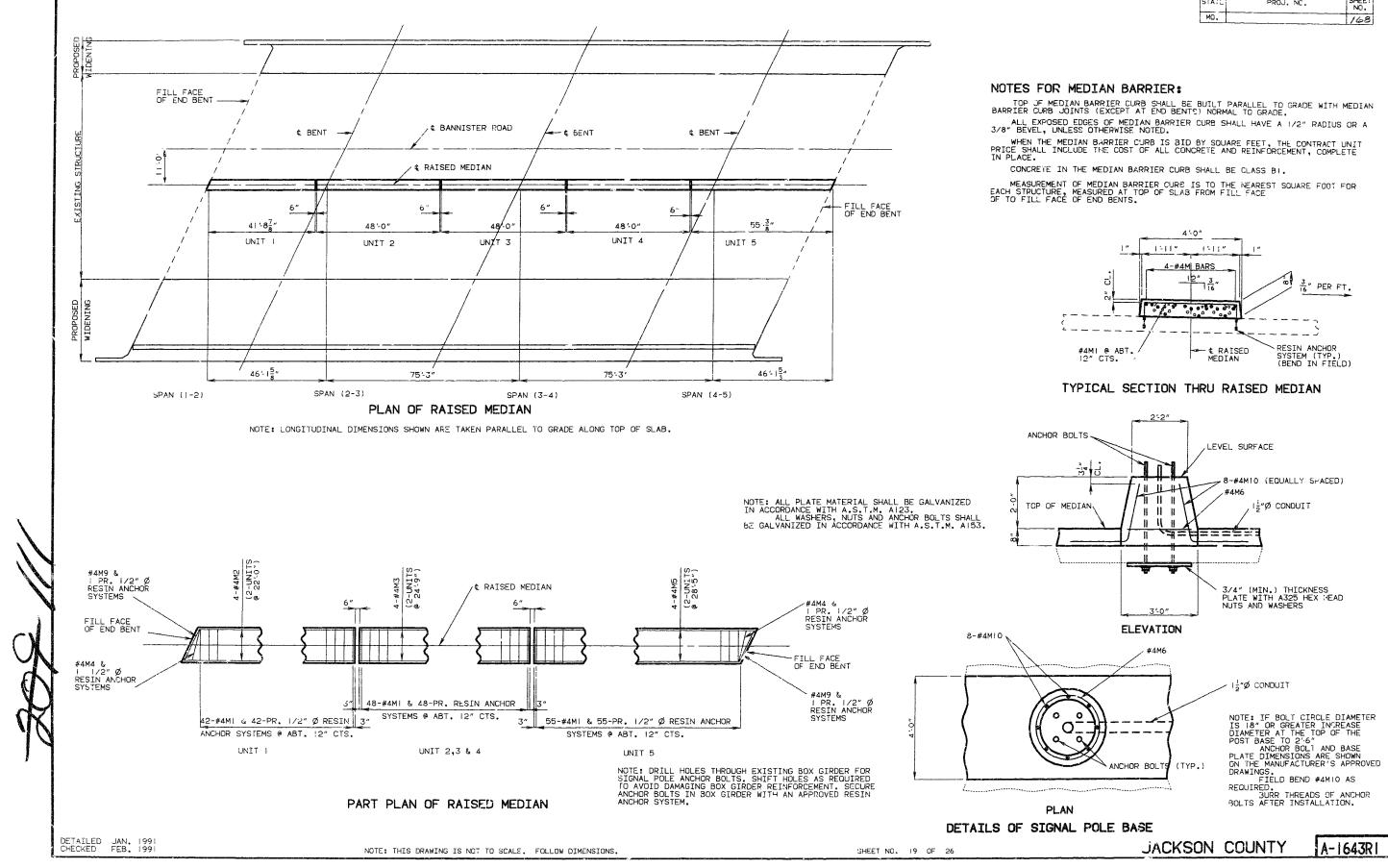


TABLE	OF DIME	NSIONS
SPAN	A	B
(1-2)	<del>7</del> "	3 <i>"</i>
(2-3)	1 13"	13"
(3-4)	$1 \frac{13}{16}''$	1 <del>3</del> "
(4-5)	<del>7</del> 16	$\frac{3}{8}$ "



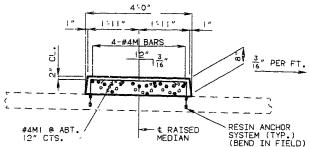


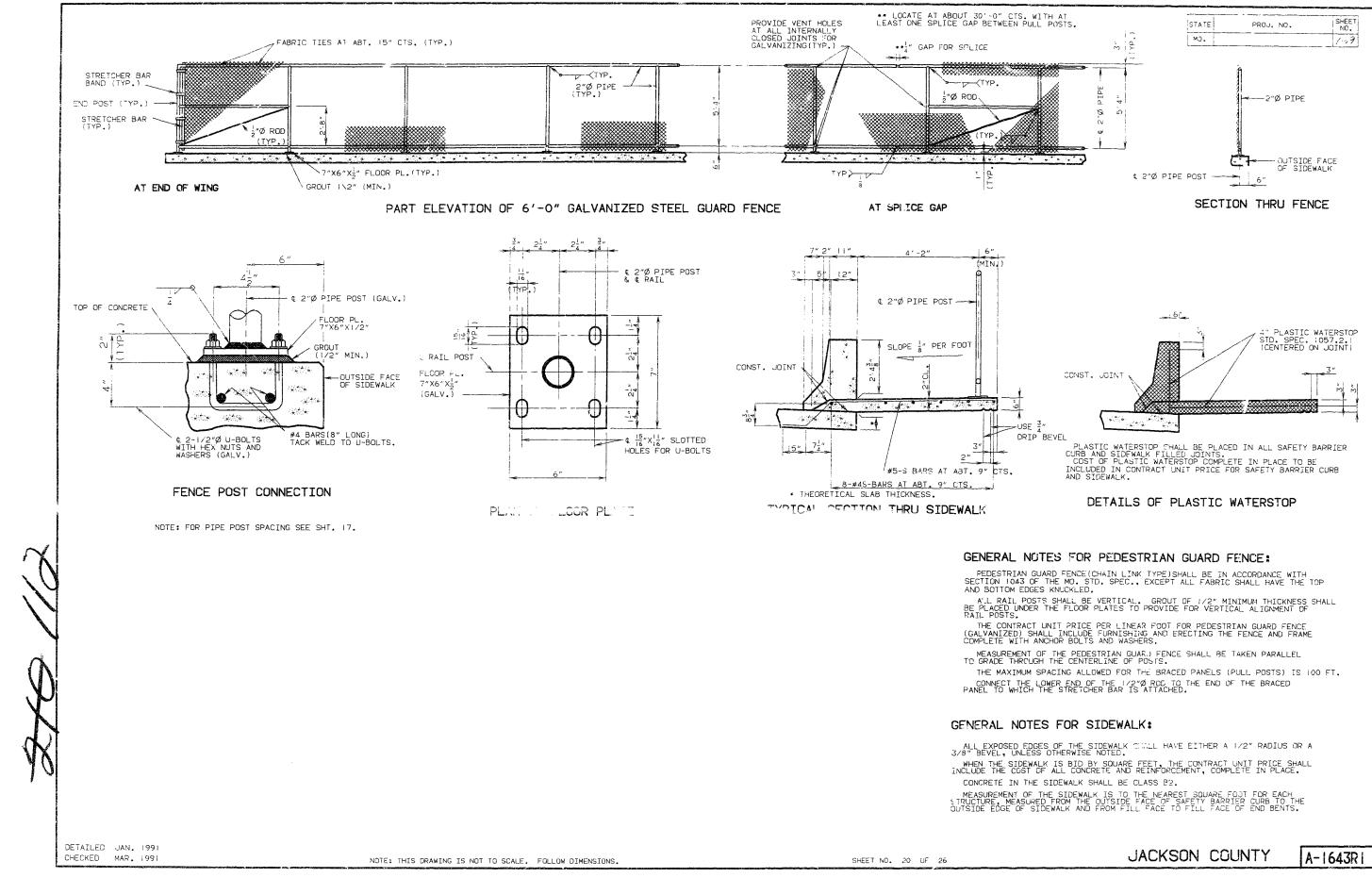


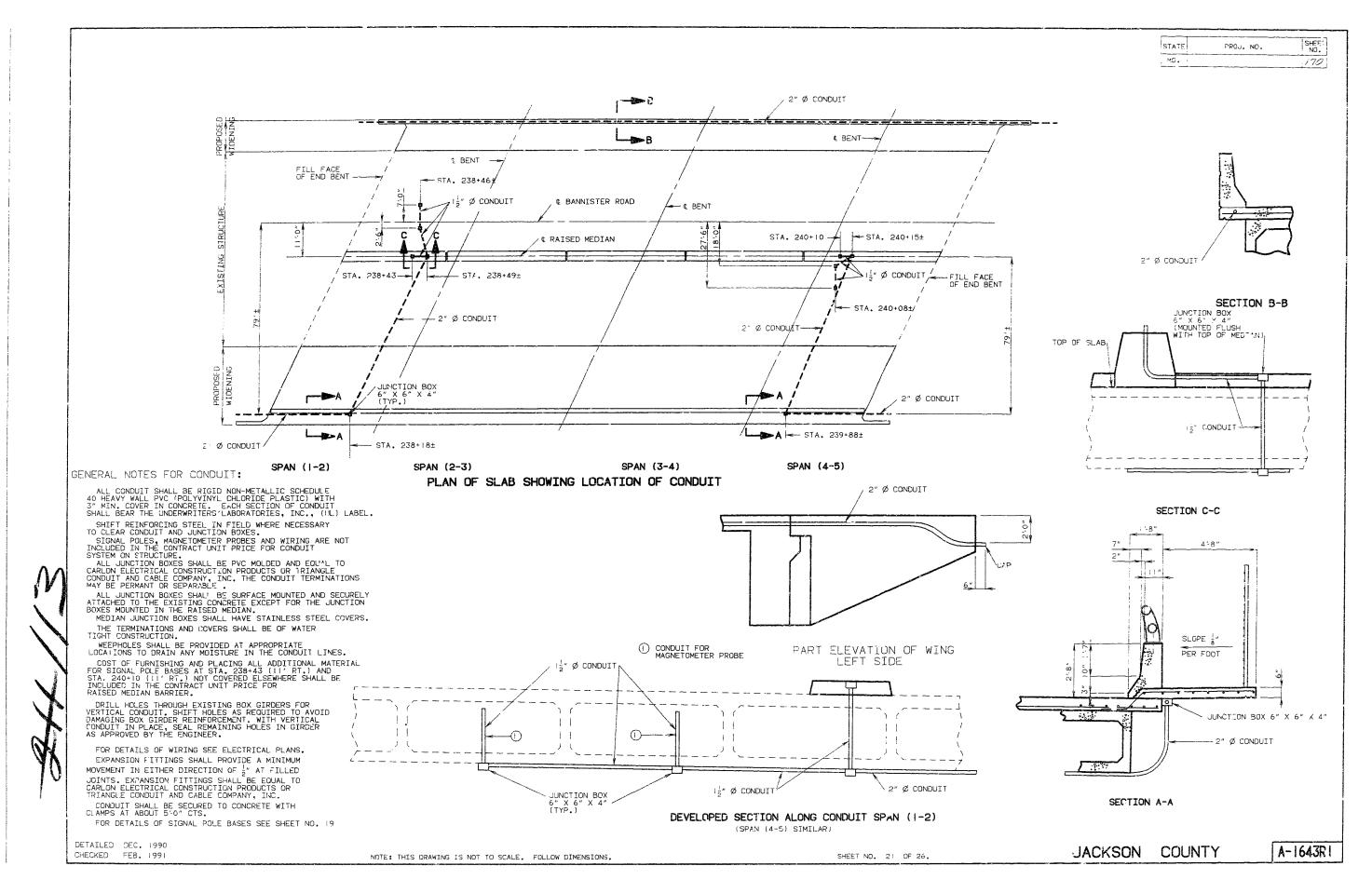


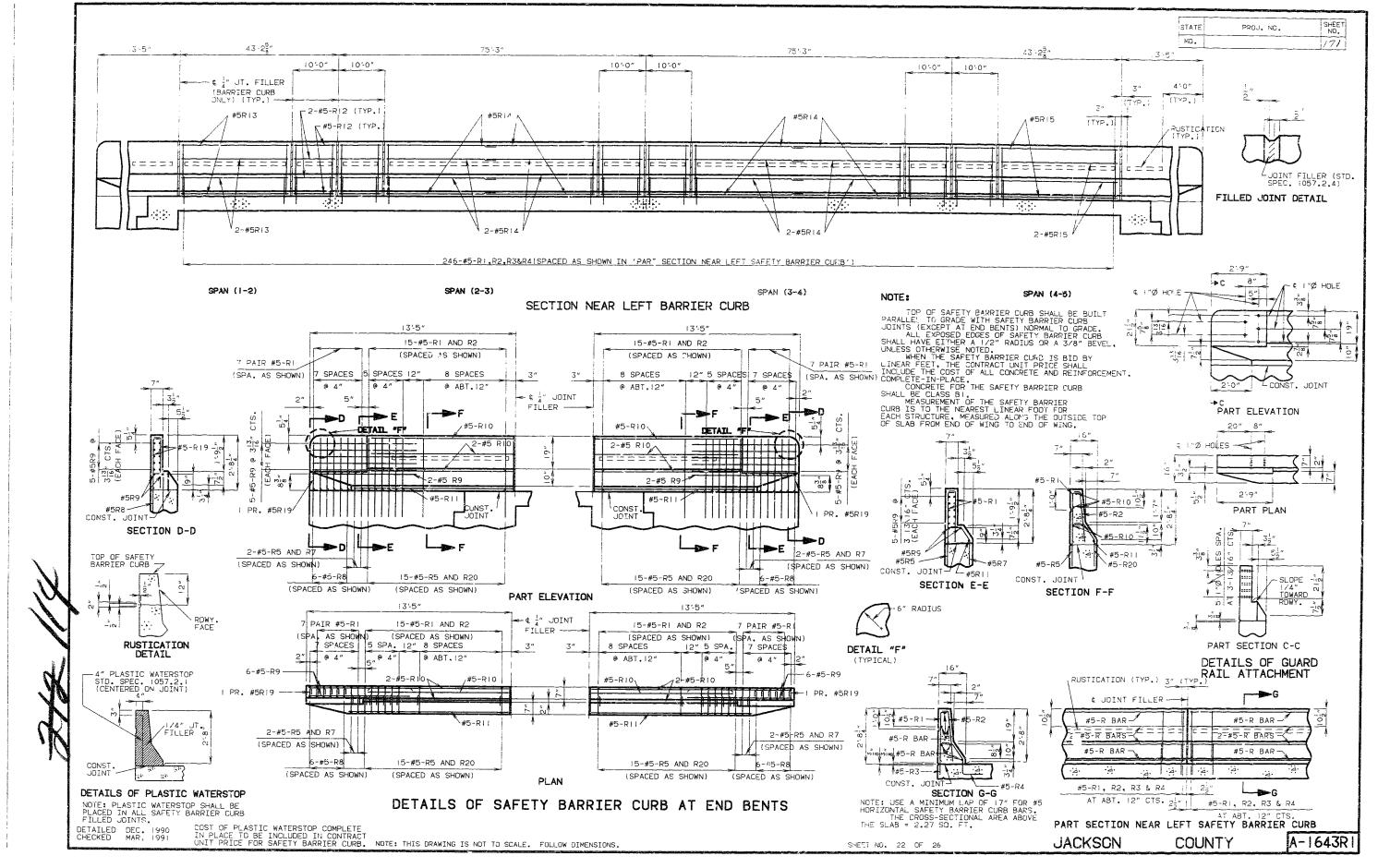
## A16433, Sht. 55

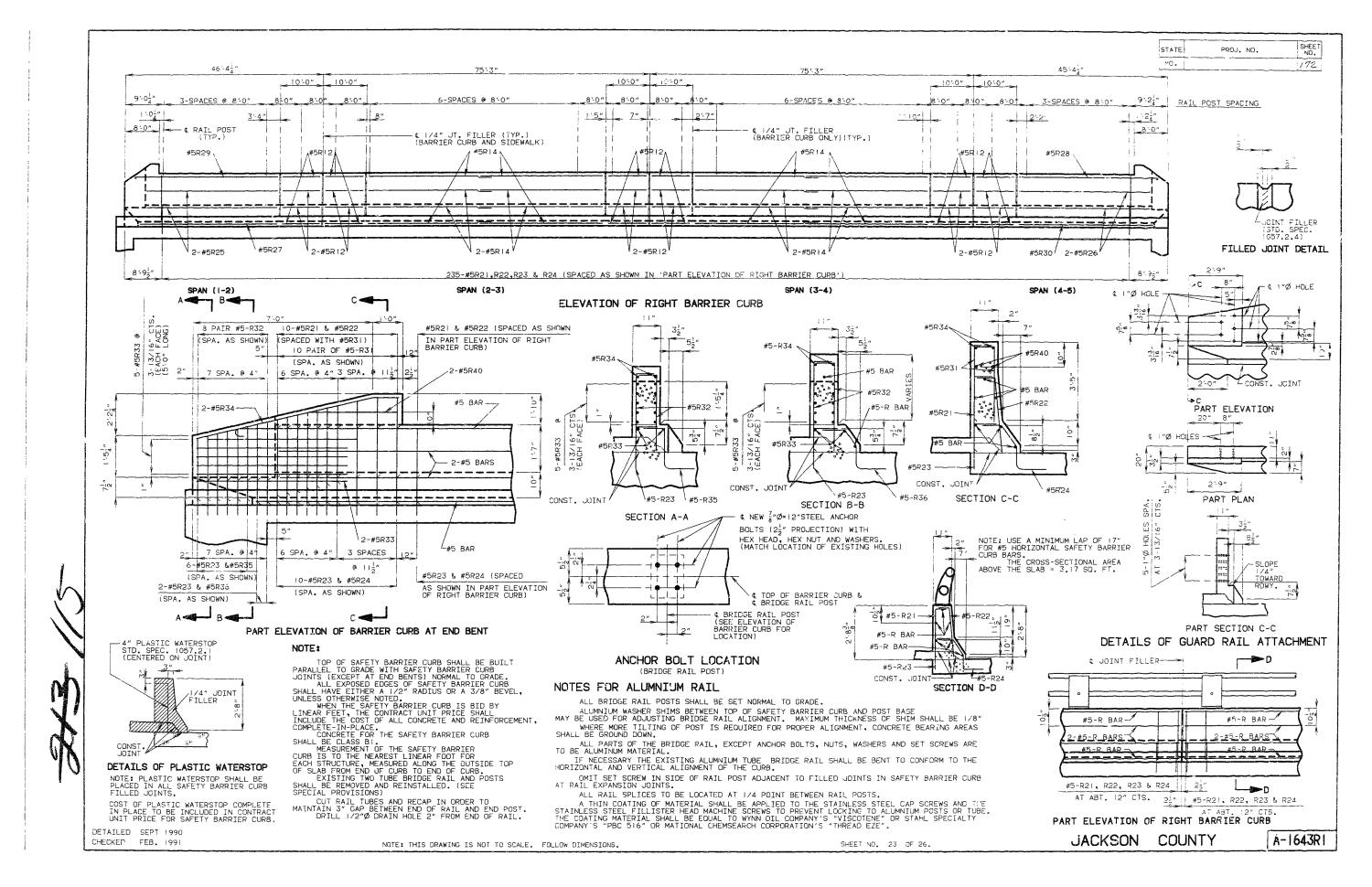
#### SHEET STAT PROJ. NC.







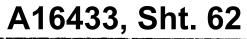


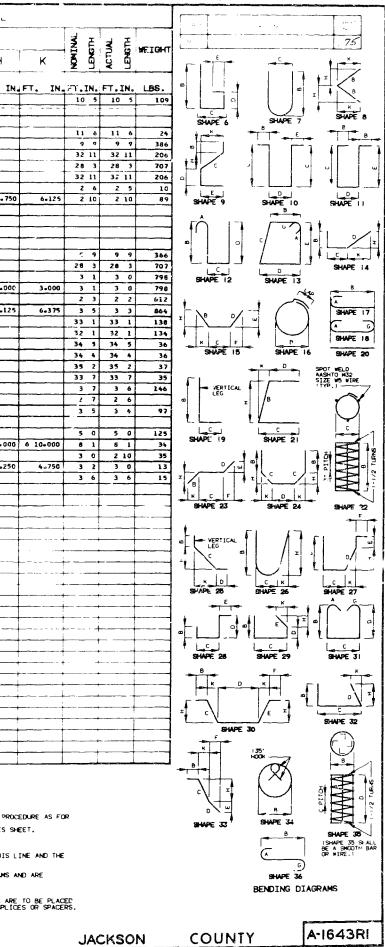


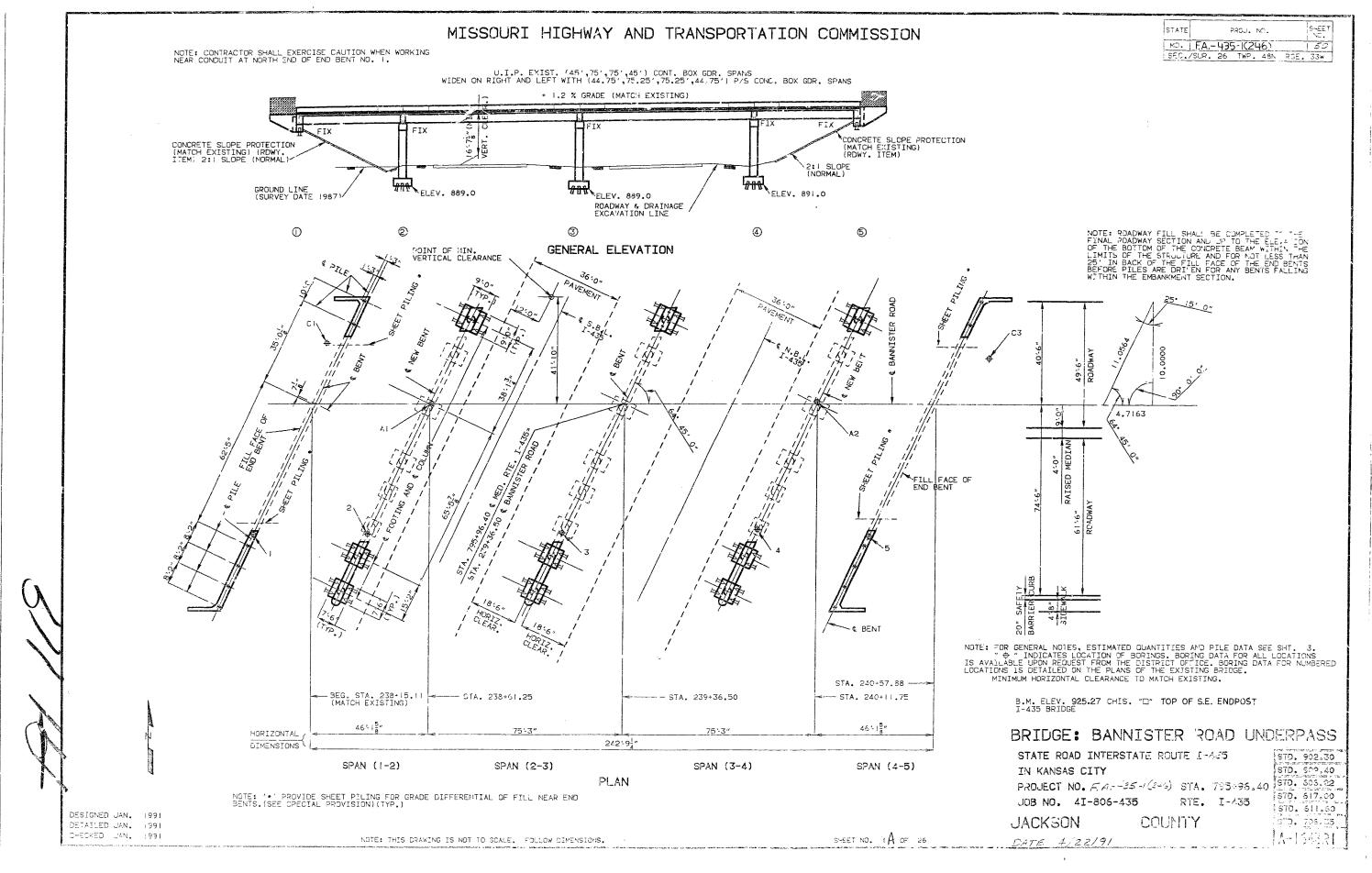
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NOTE LURLESS OTHERWISE HOTED DIAMETER 0 STIRRUP 135° STI		 90° °	STIND ID	1	— ⊨ 37•••		10	D* I	S THE SA	AME FOR A	NLL BENDS /	NO HOOKS	ं चै	د 180*	'L		¥	#9 9	1/2-	15- 11-3	/4" 19"		WEIGHT	SARE B	ASED ON A	CERS AF	RE REQUI	RED FOR	EACH COLU	MN SPIRAL	SPACERS	ARE TO BE PL	CED				DIAL		
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CUMPLETE BILL OF REINFORCIN	NG STEEL	COMPLETE BILL OF REINFORCING STEEL
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4         6H6         BFAM         20         14         0.000           8         6H9         WING         20         12         0.000		
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66 GRADES 40 - 50 - 60 KSI		IN, I A OR G J A OR G E EPOXY COATED REINFORCEMENT.
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|              | 6460<br>6461<br>6472<br>5H100<br>5H115<br>5H116<br>5H116<br>5H116<br>5H118<br>5H17<br>5H17<br>5H17<br>5H18<br>4H119<br>5H120<br>555<br>555<br>555<br>555<br>555<br>555<br>555<br>555<br>555<br>5 | 64950         BEAM           6491         BEAM           6491         BEAM           6491         BEAM           6492         DIAPH           1         INTERMEDIATE           DIAPHRAGHS         S           5H100         STRAND BAR           5H110         STRAND BAR           5H115         INT- DIAPH           5H115         INT- DIAPH           5H116         STRAND BAR           5H117         INT- DIAPH           5H118         INT- DIAPH           5H110         INT- DIAPH           5H120         INT- DIAPH           5H117         INT- DIAPH           5H118         INT- DIAPH           5H110         INT- DIAPH           5H120         INT- DIAPH           5H120         INT- DIAPH           5S5         SLAB           5S5         SLAB           5S5         SLAB           5S5         SLAB           5S10         SLAB           5S10         SLAB           5S10         SLAB           5S11         SLAB           5S12         SLAB           SS13         SLAB <td>64960         BEAM         E         20           6491         BFAM         E         19           6491         BFAM         E         19           6491         BFAM         E         19           6492         DTAPH-         E         19           0TAPHAGMS         1         1           5H100         STRAND BAR         20           5H110         STRAND BAR         20           5H115         INT- DTAPH-         E           5H116         STRAND BAR         20           5H117         INT- DTAPH-         E           5H118         INT- DTAPH         21           64119         TAT- DTAPH         E           5H117         INT- DTAPH         E           553         SLAB         E         20           553         SLAB         E         20           554         SLAB         E         20           555         SLAB         E         20           555         SLAB         E         20           5510         SLAB         E         20           5511         SLAB         E         20           <td< td=""><td>6460         BEAM         E         20           6401         BFAN         E         19         S           6401         BFAN         E         19         S           6472         DTAPH-         E         19         S           INTERREDIATE         INTERREDIATE         INTERREDIATE         INTERREDIATE           DTAPHRAGMS         I         INT         INT         INT           SH100         STRAND BAR         20         S           SH115         INT-         DTAPHA         E         20           SH115         INT-         DTAPH         E         20           SH116         STRAND BAR         20         S         S           SH117         INT-         DTAPH         23         S           SH118         ITT-         DTAPH         24         S           SH120         INT-         DTAPH         E         20         I           SLAB         E         20         I         S           SLAB         E         20         I         S           SSLAB         E         20         I         S           SSLAB         E         20<!--</td--><td>6V6-0         DEAN         E         20        </td><td>64%0         DEAM         E         20         6         1.000           64%0         DEAM         E         19         5         6         1.000           64%1         DEAPH         E         19         5         5         5.000           64%2         DEAPH         E         19         5         5         5.000           01APHRAGMS         1         1         1         1         1         1           01APHRAGMS         1         1         1         1         1         1           5H100         STRAND BAR         20         15         6.000           5H115         INT- DEAPH         E         20         15         6.000           5H116         INT- DEAPH         E         28         12         12.125           5H118         INT- DEAPH         E         10         12.000         12.000           5S1         SLAB         20         1         12.000         12.000           5S2         SLAB         E         20         26         7.000           5S3         SLAB         E         20         27         1.000           5S4         SLAB</td><td>64950       DEAM       E       20       6       1-000         6491       BFAN       E       19       S       6       1-000         6492       DTAPH.       E       19       S       5       5.000       2       5.1         INTERMEDIATE       I</td></td></td<><td>6 V50         DEFAM         E         20         6         1.000           6 V61         BFAN         E         19         S         6         1.000           6 V62         DTAPH-         E         19         S         5         5.000         2           INTERMEDIATE         I         S         S         5.000         2         5.000           SH100         STRAND BAR         20         I         5         6.000         5           SH100         STRAND BAR         20         I         5         6.000         5           SH115         TAT. DTAPH         E         20         I         2.0000         5           SH117         IAT. DTAPH         E         20         I         2.000         5           SH117         IAT. DTAPH         E         20         I         1.2.000         3         5.000           SH118         IAT. DTAPH         E         20         I         1.2.000         3         5.000           SSLAB         E         20         I         2.6         7.000         5           SSLAB         E         20         I         I         2.000</td><td>4460       DEAM       E       20       1       6       1.000      </td><td>4960       DEAM       C       20       1&lt;</td><td>6FAR       C       20       6       1.000       1.000       1.000         6W4       DIAPH.       E       19       5       6       1.000       4.000       1.000         6W7.0       DIAPH.       E       19       5       6       1.000       4.000       1.000         1MTERRECIATE       19       5       5       5.000       1.000       1.000       1.000         1MTERRECIATE       10       15       6.000       1.000       1.000       1.000       1.000         SHIDO       STRAND BAR       20       15       6.000       1.000       1.000       1.000         SHIDO       STRAND BAR       20       15       6.000       1.000       1.000       1.000         SHIDI       INT. DIAPH       21       5       1.2.125       18-000       1.000       1.000         SHIDI       INT. DIAPH       21       5       1.2.125       18-000       1.000       1.000       1.000         SHID       INT. DIAPH       21       5       1.000       1.000       1.000       1.000       1.000       1.000       1.000       1.000       1.000       1.000       1.000       1.000       1.</td><td>GRAM         E         ZO         TO         <thto< th="">         TO         TO         TO&lt;</thto<></td><td>GRAM         C         ZO         A         Loop         <thloop< th=""> <thloop< th=""> <thloop< th=""></thloop<></thloop<></thloop<></td><td>GEAR         C         20         6         1.000         A</td><td>BAN         C         D         D         D         D         D         D         D         D         D         D         D         D         D         D         D         D        
D         <thd< th="">         D         <thd< th=""> <thd< th=""></thd<></thd<></thd<></td><td>AHA         C         D         A         Loop         <thloop< th=""> <thloop< th=""> <thloop< th=""></thloop<></thloop<></thloop<></td><td>9900       NAM       C 100       1</td><td>AMD         LIM         <thlim< th=""> <thlim< th=""> <thlim< th=""></thlim<></thlim<></thlim<></td><td>arry         c</td><td>arry         brane         c&lt;</td><td>arr.         bit Martin         c         1         &lt;</td><td>arr         Company         C        C        C         <thc< t<="" td=""><td>ency         print         p i p i p i p i p i p i p i p i p i p i</td><td>name         name         <th< td=""><td>arr.         Data         B         I        I         I         I<td>arr         brief         b</td><td>unv         unv         unv</td></td></th<><td>and       black       black</td></td></thc<></td></td> | 64960         BEAM         E         20           6491         BFAM         E         19           6491         BFAM         E         19           6491         BFAM         E         19           6492         DTAPH-         E         19           0TAPHAGMS         1         1           5H100         STRAND BAR         20           5H110         STRAND BAR         20           5H115         INT- DTAPH-         E           5H116         STRAND BAR         20           5H117         INT- DTAPH-         E           5H118         INT- DTAPH         21           64119         TAT- DTAPH         E           5H117         INT- DTAPH         E           553         SLAB         E         20           553         SLAB         E         20           554         SLAB         E         20           555         SLAB         E         20           555         SLAB         E         20           5510         SLAB         E         20           5511         SLAB         E         20 <td< td=""><td>6460         BEAM         E         20           6401         BFAN         E         19         S           6401         BFAN         E         19         S           6472         DTAPH-         E         19         S           INTERREDIATE         INTERREDIATE         INTERREDIATE         INTERREDIATE           DTAPHRAGMS         I         INT         INT         INT           SH100         STRAND BAR         20         S           SH115         INT-         DTAPHA         E         20           SH115         INT-         DTAPH         E         20           SH116         STRAND BAR         20         S         S           SH117         INT-         DTAPH         23         S           SH118         ITT-         DTAPH         24         S           SH120         INT-         DTAPH         E         20         I           SLAB         E         20         I         S           SLAB         E         20         I         S           SSLAB         E         20         I         S           SSLAB         E         20<!--</td--><td>6V6-0         DEAN         E         20        </td><td>64%0         DEAM         E         20         6         1.000           64%0         DEAM         E         19  
      5         6         1.000           64%1         DEAPH         E         19         5         5         5.000           64%2         DEAPH         E         19         5         5         5.000           01APHRAGMS         1         1         1         1         1         1           01APHRAGMS         1         1         1         1         1         1           5H100         STRAND BAR         20         15         6.000           5H115         INT- DEAPH         E         20         15         6.000           5H116         INT- DEAPH         E         28         12         12.125           5H118         INT- DEAPH         E         10         12.000         12.000           5S1         SLAB         20         1         12.000         12.000           5S2         SLAB         E         20         26         7.000           5S3         SLAB         E         20         27         1.000           5S4         SLAB</td><td>64950       DEAM       E       20       6       1-000         6491       BFAN       E       19       S       6       1-000         6492       DTAPH.       E       19       S       5       5.000       2       5.1         INTERMEDIATE       I</td></td></td<> <td>6 V50         DEFAM         E         20         6         1.000           6 V61         BFAN         E         19         S         6         1.000           6 V62         DTAPH-         E         19         S         5         5.000         2           INTERMEDIATE         I         S         S         5.000         2         5.000           SH100         STRAND BAR         20         I         5         6.000         5           SH100         STRAND BAR         20         I         5         6.000         5           SH115         TAT. DTAPH         E         20         I         2.0000         5           SH117         IAT. DTAPH         E         20         I         2.000         5           SH117         IAT. DTAPH         E         20         I         1.2.000         3         5.000           SH118         IAT. DTAPH         E         20         I         1.2.000         3         5.000           SSLAB         E         20         I         2.6         7.000         5           SSLAB         E         20         I         I         2.000</td> <td>4460       DEAM       E       20       1       6       1.000      </td> <td>4960       DEAM       C       20       1&lt;</td> <td>6FAR       C       20       6       1.000       1.000       1.000         6W4       DIAPH.       E       19       5       6       1.000       4.000       1.000         6W7.0       DIAPH.       E       19       5       6       1.000       4.000       1.000         1MTERRECIATE       19       5       5       5.000       1.000       1.000       1.000         1MTERRECIATE       10       15       6.000       1.000       1.000       1.000       1.000         SHIDO       STRAND BAR       20       15       6.000       1.000       1.000       1.000         SHIDO       STRAND BAR       20       15       6.000       1.000       1.000       1.000         SHIDI       INT. DIAPH       21       5       1.2.125       18-000       1.000       1.000         SHIDI       INT. DIAPH       21       5       1.2.125       18-000       1.000       1.000       1.000         SHID       INT. DIAPH       21       5       1.000       1.000       1.000       1.000       1.000       1.000       1.000       1.000       1.000       1.000       1.000       1.000       1.</td> <td>GRAM         E         ZO         TO         <thto< th="">         TO         TO         TO&lt;</thto<></td> <td>GRAM         C         ZO         A         Loop         <thloop< th=""> <thloop< th=""> <thloop< th=""></thloop<></thloop<></thloop<></td> <td>GEAR         C         20         6         1.000         A</td> <td>BAN         C         D         <thd< th="">         D         <thd< th=""> <thd< th=""></thd<></thd<></thd<></td> <td>AHA         C         D         A         Loop         <thloop< th=""> <thloop< th=""> <thloop< th=""></thloop<></thloop<></thloop<></td> <td>9900       NAM       C 100       1</td> <td>AMD         LIM         <thlim< th=""> <thlim< th=""> <thlim< th=""></thlim<></thlim<></thlim<></td> <td>arry         c</td> <td>arry         brane         c         c         c         c         c         c         c         c         c         c         c         c         c         c         c        
c         c&lt;</td> <td>arr.         bit Martin         c         1         &lt;</td> <td>arr         Company         C        C        C         <thc< t<="" td=""><td>ency         print         p i p i p i p i p i p i p i p i p i p i</td><td>name         name         <th< td=""><td>arr.         Data         B         I        I         I         I<td>arr         brief         b</td><td>unv         unv         unv</td></td></th<><td>and       black       black</td></td></thc<></td> | 6460         BEAM         E         20           6401         BFAN         E         19         S           6401         BFAN         E         19         S           6472         DTAPH-         E         19         S           INTERREDIATE         INTERREDIATE         INTERREDIATE         INTERREDIATE           DTAPHRAGMS         I         INT         INT         INT           SH100         STRAND BAR         20         S           SH115         INT-         DTAPHA         E         20           SH115         INT-         DTAPH         E         20           SH116         STRAND BAR         20         S         S           SH117         INT-         DTAPH         23         S           SH118         ITT-         DTAPH         24         S           SH120         INT-         DTAPH         E         20         I           SLAB         E         20         I         S           SLAB         E         20         I         S           SSLAB         E         20         I         S           SSLAB         E         20 </td <td>6V6-0         DEAN         E         20        </td> <td>64%0         DEAM         E         20         6         1.000           64%0         DEAM         E         19         5         6         1.000           64%1         DEAPH         E         19         5         5         5.000           64%2         DEAPH         E         19         5         5         5.000           01APHRAGMS         1         1         1         1         1         1           01APHRAGMS         1         1         1         1         1         1           5H100         STRAND BAR         20         15         6.000           5H115         INT- DEAPH         E         20         15         6.000           5H116         INT- DEAPH         E         28         12         12.125           5H118         INT- DEAPH         E         10         12.000         12.000           5S1         SLAB         20         1         12.000         12.000           5S2         SLAB         E         20         26         7.000           5S3         SLAB         E         20         27         1.000           5S4         SLAB</td> <td>64950       DEAM       E       20       6       1-000         6491       BFAN       E       19       S       6       1-000         6492       DTAPH.       E       19       S       5       5.000       2       5.1         INTERMEDIATE       I</td> | 6V6-0         DEAN         E         20 | 64%0         DEAM         E         20         6         1.000           64%0         DEAM         E         19         5         6         1.000           64%1         DEAPH         E         19         5         5         5.000           64%2         DEAPH         E         19         5         5         5.000           01APHRAGMS         1         1         1         1         1         1           01APHRAGMS         1         1         1         1         1         1           5H100         STRAND BAR         20         15         6.000           5H115         INT- DEAPH         E         20         15         6.000           5H116         INT- DEAPH         E         28         12         12.125           5H118         INT- DEAPH         E         10         12.000         12.000           5S1         SLAB         20         1         12.000         12.000           5S2         SLAB         E         20         26         7.000           5S3         SLAB         E         20         27         1.000           5S4         SLAB | 64950       DEAM       E       20       6       1-000         6491       BFAN       E       19       S       6       1-000         6492       DTAPH.       E       19       S       5       5.000       2       5.1         INTERMEDIATE       I | 6 V50         DEFAM         E         20         6         1.000           6 V61         BFAN         E         19         S         6         1.000           6 V62         DTAPH-         E         19         S         5         5.000         2           INTERMEDIATE         I         S         S         5.000         2         5.000           SH100         STRAND BAR         20         I         5         6.000         5           SH100
        STRAND BAR         20         I         5         6.000         5           SH115         TAT. DTAPH         E         20         I         2.0000         5           SH117         IAT. DTAPH         E         20         I         2.000         5           SH117         IAT. DTAPH         E         20         I         1.2.000         3         5.000           SH118         IAT. DTAPH         E         20         I         1.2.000         3         5.000           SSLAB         E         20         I         2.6         7.000         5           SSLAB         E         20         I         I         2.000 | 4460       DEAM       E       20       1       6       1.000 | 4960       DEAM       C       20       1< | 6FAR       C       20       6       1.000       1.000       1.000         6W4       DIAPH.       E       19       5       6       1.000       4.000       1.000         6W7.0       DIAPH.       E       19       5       6       1.000       4.000       1.000         1MTERRECIATE       19       5       5       5.000       1.000       1.000       1.000         1MTERRECIATE       10       15       6.000       1.000       1.000       1.000       1.000         SHIDO       STRAND BAR       20       15       6.000       1.000       1.000       1.000         SHIDO       STRAND BAR       20       15       6.000       1.000       1.000       1.000         SHIDI       INT. DIAPH       21       5       1.2.125       18-000       1.000       1.000         SHIDI       INT. DIAPH       21       5       1.2.125       18-000       1.000       1.000       1.000         SHID       INT. DIAPH       21       5       1.000       1.000       1.000       1.000       1.000       1.000       1.000       1.000       1.000       1.000       1.000       1.000       1. | GRAM         E         ZO         TO         TO <thto< th="">         TO         TO         TO&lt;</thto<> | GRAM         C         ZO         A         Loop         Loop <thloop< th=""> <thloop< th=""> <thloop< th=""></thloop<></thloop<></thloop<> | GEAR         C         20         6         1.000         A | BAN         C         D <thd< th="">         D         <thd< th=""> <thd< th=""></thd<></thd<></thd<> | AHA         C         D         A         Loop         Loop <thloop< th=""> <thloop< th=""> <thloop< th=""></thloop<></thloop<></thloop<> | 9900       NAM       C 100       1 | AMD         LIM         LIM <thlim< th=""> <thlim< th=""> <thlim< th=""></thlim<></thlim<></thlim<> | arry         c | arry         brane         c< | arr.         bit Martin         c         1         < | arr         Company         C
        C        C        C <thc< t<="" td=""><td>ency         print         p i p i p i p i p i p i p i p i p i p i</td><td>name         name         <th< td=""><td>arr.         Data         B         I        I         I         I<td>arr         brief         b</td><td>unv         unv         unv</td></td></th<><td>and       black       black</td></td></thc<> | ency         print         p i p i p i p i p i p i p i p i p i p i | name         name <th< td=""><td>arr.         Data         B         I        I         I         I<td>arr         brief         b</td><td>unv         unv         unv</td></td></th<> <td>and       black       black</td> | arr.         Data         B         I        I         I         I <td>arr         brief         b</td> <td>unv         unv         unv</td> | arr         brief         b | unv         unv | and       black       black |







ITEM		SUBSTR.	SUPERSTR	TOTAL
REMOVAL & STORAGE OF EXIST. BRIDGE RAIL	LIN. FT.	1	225	226
CURB REMOVAL (BRIDGES)	LIN. FT.		747	747
PARTIAL REMOVAL OF SUBSTR. CONC.	LUMP SUM	1		1
PARTIAL REMOVAL OF EXIST. BRIDGE DECK	SQ. FT.		1184	1184
CLASS   EXCAVATION	CU. YD.	294.5	1	294.5
SHEET PILING	LUMP SUM			1
(72") PEDESTRIAN FENCE (STRUCTURES)	LIN. FT.		263	263
STRUCTURAL STEEL PILES (10")	LIN. FT.	432		432
STRUCTURAL STEEL PILES (12")	LIN. FT.	738		1738
CLASS B CONCRETE (SUBSTR.)	CU. YD.	190.1	1	190.1
SUBSTR, REPAIR (UNFORMED)	SO. FT.	0	1	0
SUPSTR. REPAIR (UNFORMED)	SQ. FT.		0	NO 1
CLASS B2 CONCRETE (SUPSTR) CONC ON BOX	GDR CU. YD.		295.6	295.6
J6" SAFETY BARRIER CURB	LIN. FT.		264	264
20" SAFETY BARRIER CURB	LIN. FT.		242	242
RAISED MEDIAN BARRIER	SO. FT,		971	971
SIDEWALK (BRIDGES)	SQ. FT.		1133	1133 .
PLAIN NEOPRENE BEARING PADS	EACH		18	- 18 -
AMINATED NEOPRENE BEARING PADS	EACH		54	54
P/S CONC. BOX GDR. (45' SPAN)	EACH		18	18
P/S CONC, BOX GDR. (75' SPAN)	EACH		18.	18 /
REINFORCING STEEL (BRIDGES)	FOUND	25740	8260	×34000 ·
CONDUIT SYSTEM ON STRUCTURE	LUMP SUM			1997 F
REINFORCING STEEL (EPOXY COATED)	POUND		65690	55690
BRIDGE RAIL RELOCATED	LUMP SUM			250 L V
		·		

NOTE: ALL CONCRETE ABOVE LOWER CONSTRUCTION JOINT IN END BENTS IS INCLUDED WITH SUPERSTRUCTURE QUANTITIES.

WITH SUPERSTRUCTURE QUANTITIES. ALL REINFORCEMENT IN THE END BENTS IS INCLUDED WITH SUPERSTRUCTURE QUANTITIES. THE COST OF FURNISHING, FABRICATING AND INSTALLING NEOPRENE BEARING PADS, COMPLETE-IN-PLACE, WILL BE PAID FOR AT THE CONTRACT UNIT PRICE FOR PLAIN AND LAMINATED NEOPRENE BEARING PADS PER EACH. ALL CONCRETE AND REINFORCING STEEL IN THE SIDEWALK ARE INCLUDED IN THE SUPERSTRUCTURE QUANTITIES FOR SIDEWALKS. ALL CONCRETE AND REINFORCING STEEL IN THE RAISED MEDIAN BARRIER ARE INCLUDED IN THE SUPERSTRUCTURE QUANTITIES FOR RAISED MEDIAN BARRIER. COST OF FURNISHING AND INSTALLING RESIN ANCHOR SYSTEMS, COMPLETE IN PLACE, SHALL BE INCLUDED IN THE UNIT PRICE BID FOR CONCRETE.

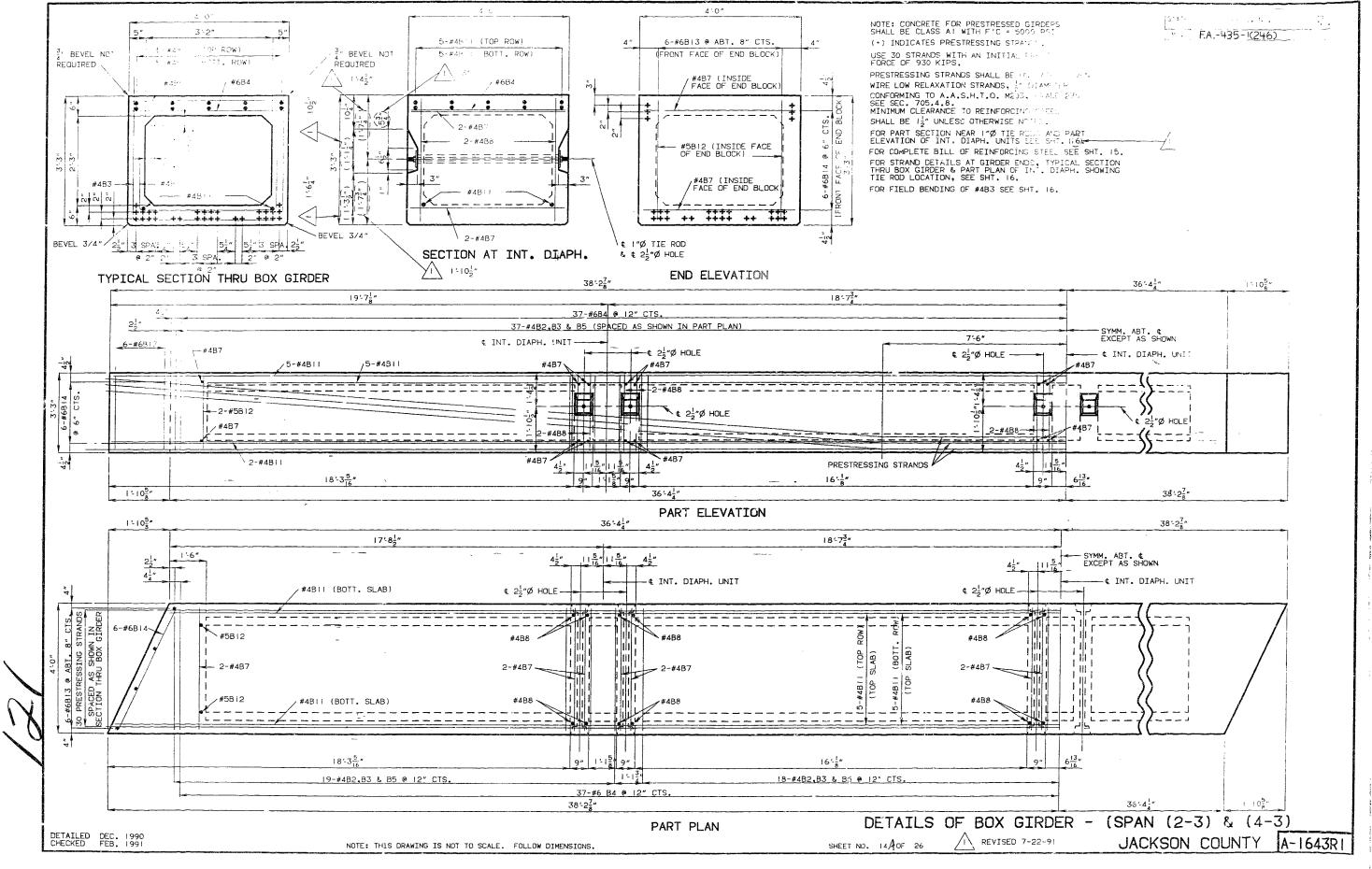
					and a subscription of the second second second second second second second second second second second second s	No. of Concession, Name of Street, or other				
BENT NUMBER	I LT.	I RT.	2 LT.	2 RT.	3 LT.	3 RT.	4 LT.	4 RT.	5 LT.	5 RT.
PILE TYPE AND SIZE	HPI0X42	HP10X42	HP12X53	HP12X53	HP12X53	HP12X53	HP12X53	HP12X53	HPI0X42	HPI0X42
NUMBER	2	4	5	8	6	10	5	8	2	4
APPROXIMATE LENGTH (FT.)	31	34	17	17	17	17	19	.19	36	42
DESIGN BEARING (TONS)	52	52	60	69	61	64	60	69	52	52
HAMMER ENERGY RED'D. (FT/LBS)	11500	11500	14100	16200	14400	16400	14100	16200	11500	11500

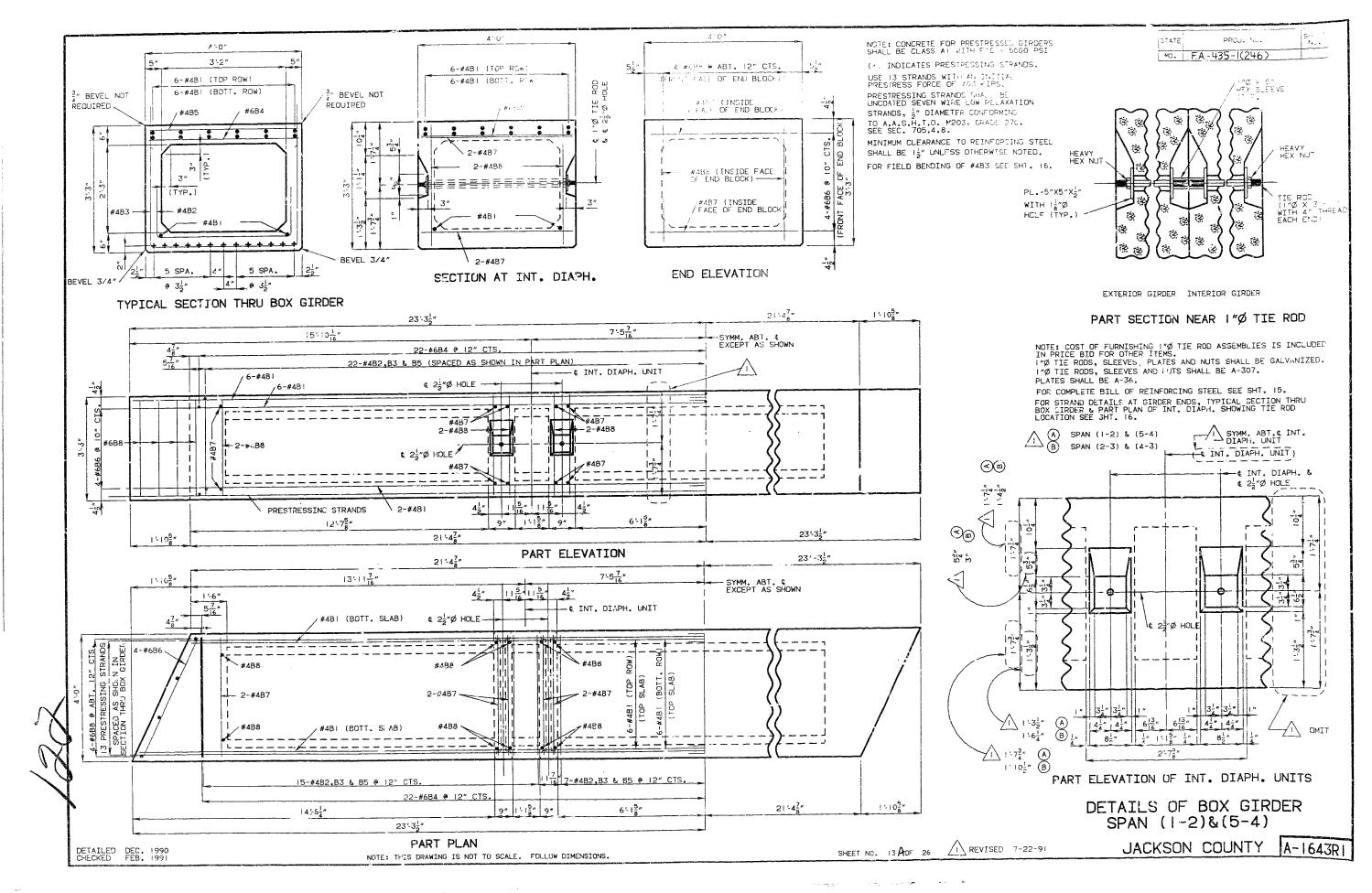
MINIMUM ENERGY REQUIREMENT OF HAMMER BASED ON PLAN LENGTH AND DESIGN BEARING VALUE OF PILES. ALL PILE SHALL BE DRIVEN TO PRACTICAL REFUSAL.

## A16433, Sht. 64

STATE PROJ. NO. SHEET NO. F.A435-1(246) 1752								
GENERAL NOTES:								
DESIGN SPECIFICATIONS: A.A.S.H.T.D1989 LOAD FACTOR DESIGN. A.A.S.H.T.D1983 GUIDE SPECIFICATIONS FOR SEISMIC DESIGN PERFORMANCE CATEGORY A DESIGN LOADING:								
HS20-44 35# FUTURE WEARING SURFACE MODIFIED 24,000# TANDEM AXLE EARTH 120#/CU. FT., EDUIVALENT FLUID PRESSURE 45#/CU. FT.								
SUPERSTRUCTURE: SIMPLY-SUPPORTED, NON-COMPOSITE FOR DEAD LOAD. Continuous composite for live load.								
DESIGN UNIT STRESSES:								
CLASS B CONCRETE (SUBSTRUCTURE) F'C=3,000 PSI								
CLASS B2 CONCRETE (SUPERSTRUCTURE, EXCEPT PRESTRESSED GDRS., RAISED MEDIAN BARRIER AND SAFETY BARRIER CURB) F'C=4,000 PSI								
CLASS BI CONCRETE (SAFETY BARRIER CURB & RAISED MEDIAN BARRIER)								
REINFORCING STEEL (GRADE 60) FY=60,000 PSI								
STEEL PILE FB=9,000 PSI								
FOR PRESTRESSED GIRDER STRESSES, SEE SHT. 13 & 14.								
NEOPRENE BEARING PADS: BEARINGS SHALL BE 60 DUROMETER NEOPRENE PADS.								
JOINT FILLER: ALL JOINT FILLER SHALL MEET THE REQUIREMENTS OF STD. SPEC. 1057.2.4, EXCEPT AS NOTED.								
REINFORCING STEEL: MINIMUM CLEARANCE TO REINFORCING STEEL SHALL BE 1 1/2", UNLESS OTHERWISE SHOWN.								
BARS BONDED IN OLD CONCRETE NOT REMOVED SHILL BE CLEANLY STRIPPED AND EMBEDDED INTO NEW CONCRETE WHERE POSSIBLE. IF LENGTH IS AVAILABLE, OLD BARS SHALL EXIEND INTO NEW CONCRETE AT LEAST 30 DIAMETERS FOR DEFORMED BARS, UNLESS OTHERWISE NOTED.								
CONSTRUCTION CLEARANCE: A MINIMUM VERTIC/ CLEARANCE OF 151-0" FROM CROWN OF EXISTING LANES AND A MINIMUM LATERAL CL_ARANCE OF 401-0" CENTERED ON EXISTING LANES SHALL BE MAINTAINED DURING CONSTRUCTION.								
TRAFFIC MAINTAINED: TRAFFIC OVER STRUCTURE TO BE MAINTAINED DURING CONSTRUCTION. (SEE ROADWAY PLANS)								
EXISTING STRUCTURF: OUTLINE OF OLD WORK IS INDICATED BY LIGHT DASHED LINES. HEAVY LINES INDICATE NEW WORK.								
CONTRACTOR SHALL VERIFY ALL DIMENSIONS IN FIELD BEFORE ORDERING NEW MATERIALS.								
RESIN ANCHOR SYSTEMS: THE CONTRACTOR SHALL USE ONE OF THE ANCHOR SYSTEMS LISTED IN THE SPECIAL PROVISIONS. THESE ANCHOR SYSTEMS SHALL BE INSTALLED ACCORDING TO THE MANUFACTURER'S SPECIFICATIONS EXCEPT AS MODIFIED BY THE JOB SPECIAL PROVISIONS AND THAT A PLAIN, #6, GRADE 60 REINFORCING BAR 2'-6" LONG SHALL BE SUBSTITUTED FOR THE THREADED ROD STUD. AN EPOXY COATED #4, GRADE 60, REINFORCING BAR 23" LONG SHALL BE SUBSTITUTED FOR THE THREADED ROD STUD IN THE RAISED MEDIAN BARRIER.								

JACKSON COUNTY A-15-18





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