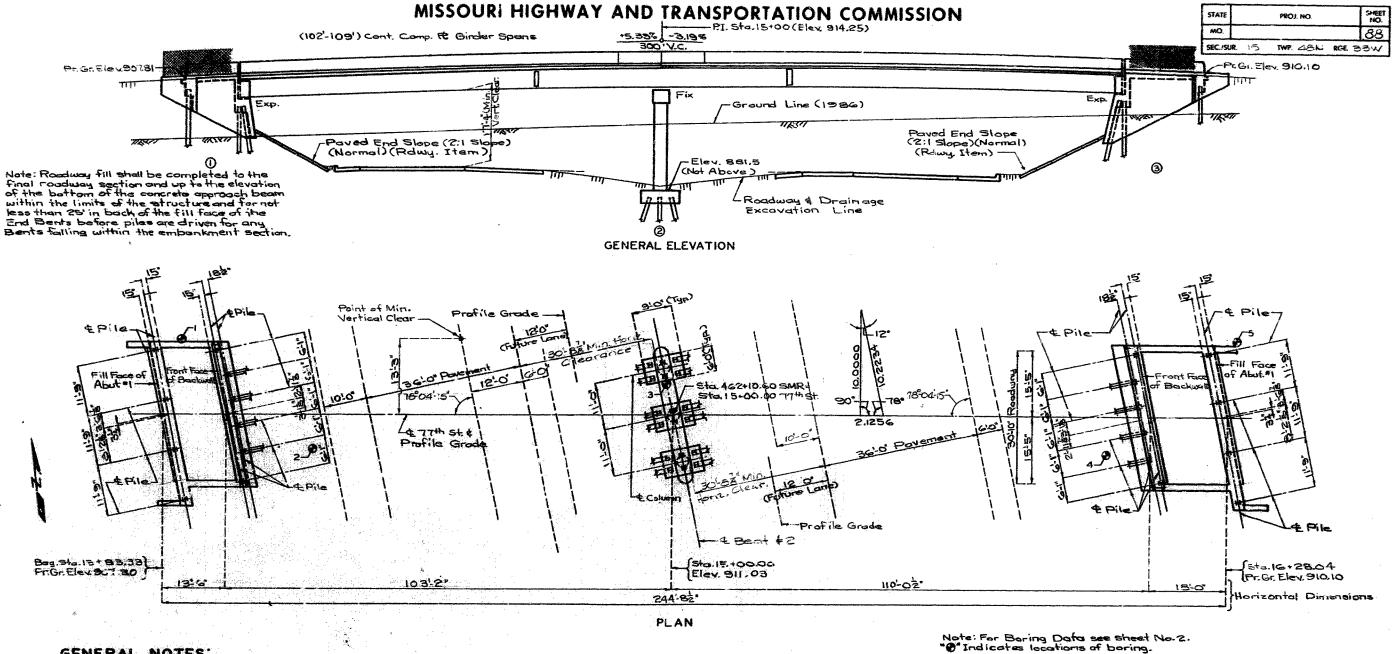
PROJ. NO

STATE



MISSOURI HIGHWAY AND TRANSPORTATION COMMISSION

GENERAL NOTES:

Design Specifications: A.A.S.H.T.O.-1983 and Interimentary 1984, 1985, 1986 \$ 1987 LOAD FACTOR DESIGN

Design Ladding:

HS20-44
35/30-11 Future Wearing Surface
Earth 120*/ou.ft, Equivalent Fluid Pressure 45*/ou.ft.
Fatigue Stress-Case II

Design Unit Stresses:

Reinforcing Steel (ASTM A572) Grade 50 fy:50,000 psi

Steel Pile fb: 3,000 psi

Class BI Concrete (Substructure) fb: 3,000 psi

Class BI Concrete (Superstructure except Safety

Barrier Curb) fb: 4,000 psi

Reinforcing Steel (Grade Go) fy:50,000 psi

Structural Carbon Steel fy:36:000 psi

Structural Steel (ASTM A572) Grade 50 fy:50,000 psi

Steel Pile fb:3000 psi

Fobricated Steel Connections:
Field connection, High Strength Bolts 34, holes 184, except as noted.

Note: Prebare for piles of approach beam Bff1, Bff2 \$ 845.00 866.3 and 887.00 respectively.

Joint Filler:
All joint filler shall meet the requirements of Std. Spec. 1057.24, except as noted.

Reinforcing Steel:

Minimum alearance to reinforcing steel shall be 12°, unless otherwise shown.

System C by contractor in accordance with Std. Spec. 71212. (Color of the final field-coat shall be green.)

*	PILE	DATA				
BENT NO.		T .		2	1 :	3
Location		App. Bm	Brg. Bm.	Footing	App Bm.	Bra.Bm.
Pile Type and Size				HP10×42		
Number		4	6	. 18	4	G
Approximate: Length	F÷.	18	15	- 11	14	15,
Design Bearing	Tons	26	50	50	30	54
Hammer Energy Required	Ft.Lbs.	7000	12400	11700	7000	13200

Minimum energy requirement of hammer is based on plan length and design bearing value of piles.
All piles shall be driven to practical refusal. SEE FINAL PLANS

B.M. No. 93 Flev. 892.94 "X" in top flange NE bolt of fire hydrant SE corner of 77th St. and Agnes

BRIDGE:77 ST. OVER SOUTH MIDTOWN ROADWAY

STATE ROAD FROM 75th St to 85th, STREET

IN KANSAS CITY

PROJECT NO.

STA. 462 +10.60

JOB NO. 4-U-71-2B ".CKSON

RTE. 71

STD.611.60 STD. 706.35 A-4738

DESIGNED FEBRUARY1988 DETAILED MARCH 1988 CHECKED July

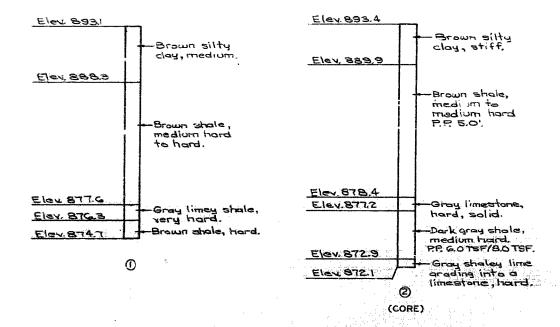
Note: This drawing is not to scale. Follow dimensions

Sheet No. 1 of 4

DATE 7/25/38

COUNTY

STATE	PROJ. NO.	SHEET NO.
MO.		89



ESTIMATE	D QUANT	TIES		
TEM 2		SUBSTR.	SUPERSTR	TOTAL
Class I Excavation	CUHL	110		110
(72 In.) Pedestrion Fence	. Un Ft.		255	255
Struct⊃ral/Steel Pile(IO')	Lin Pt.	512		512
Pre-Bore For Piling	Lin.Et.	363		363
Clase & Concrete	CUY4	156.3		1565
() Slab In Steel (See Spec Provi	510ms) Sc. 16L	78/9/W(2)	792	792
Safety Barrier Cort	Cin.Ft.		500	503
Slob On Semi-Deep Abutmen!	3q.\H.		117	117
Sidewalk	9q.F1.	46.75.75	1099	eeo!
Laminated Neoprene Bry Feds(Steel S	ructures)Ea.		12	12
Preformed Compression Fxx JA Scal			63	G3
Reinforcing Steel (C. ede Go)		15,780		15.78C
Reinforcing Steel (Epoxy Coated)	Lb.	2690		2690
Fabricated Structural Carbon Blee 1870	e Gdr.) Lo.	J 27 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	155600	155600
Fabricated Structural Low Allay Steel (Pla		Marian - sugar	35190	35190
Slati Crains	76.76		TI (a)	12
Painting (System C) Green	Ton		94,9	94.9
		200		
			54	

Note: All concrete and reinforcing steel below top of slob and above Const. Joint under slab in Sami-Reep Abutments are included in superstructure quantities for Slab on Semi-Deep Abutments.

All concrete and reinforcing steel in the sidewalk

are included in the superstructure quantities for sidewalks.

			Elev. 302.2	
Elev. 896.4 Elev. 895.4 Black top and base. Brown silty clay, medium to stiff.	Elev. 839.3 Elev. 828.3 Elev. 833.8	—Black top and base, —Brown silty clay, stiff.	Elev. 892.2	Brown silty clay, stiff,
Erown shale, medium to hard, P.P. 4.0/9.0 TSF		Brown shale, medium hard to hard 9+.		Brown shale, medium hard to hard,
Elev. 873.3 Dark gray limey shale, very hord. Elev. 873.7 Elev. 876.4 Gray limestone, hord. Flev. 876.1 3	Elev. 877.0	Dark gray limey shale, hard. Gray limestone, hard, medium bedded with sparadic thin weathered	Elev. 881.7 Elev. 877.6	Dark gray shale, hard. Gray limey shale, hard.
(CORE)	Elsv8723	seans.	(9
	@			
	(COR	E)		

BORING DATA

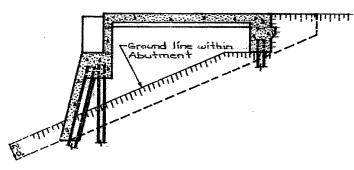
Note: For location of borings see sheet No.1.

ESTIMATED QUANTITIES FOR	ALTER	NATE S	LAB
	SL	AB ON S	TEEL
TYPE OF SLAR	REIN	IF.(LBS.)	
	PLAIN	120XY	(CU, YD.)
Cast-In-Place Conventional Forms	C	6,40	195.6
Precest Panel Forms		43,07C	
Stay In-Flace Forms	0	61410##	185.6

Note: The table of Estimated Quantities for Alternate Slabs represents the quantities used by the State in preparing the cost estimate for concrete slabs. Variations may be encountered in these astimated quantities but these variations cannot be used for an adjustment in the Contract Unit Price per square years of Alternate slab used.

See Special Provisions for alternate slatinoss forming slabs.

bar Supports.
Precast panal quantities are based on skewed end panels.



GROUND LINE AND PILING IN ABUTMENTS

Note: In no case shall the earth within Abutments No. 1 and 3 be above the Ground Line shown. Forms supporting the Abutment slab may be left in place.

The maximum variation of the head of the pile and the battered face of the pile from the

position shown on the plans shall be not more than Rinches for pile under Abutments No. 1 and 3. Exposed steel piles within the Abutment shall te coated with a heavy coating of an opproved bituminous point.

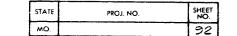
DETAILED DECEMBER 1987 CHECKED JULY 1988 FOR INFORMATION ONLY A47382. Sht. 3 SHEET 90 38-5-U5 ElZ'ets. (Placed Farallel To & Road way) 8'-34' −**t** Roadway © Fill Face 5'-6" 17=5-519 9"cts. Elev 908.05--Elev. 907.80 -7-45H49 4.38-HZ @ Fill Face -const. st. -Const. Jt.-Key-2'x10" -#4.SZI -Pile Cut-off. Elev. 905,04 L2-8-H3 L4-8-42 LEIEV 903.54 LElev. 899.76 3 Fb-VIER ds (Each Foice) **ELEVATION C-C** 35-4-UI (Spaced as shown in "Plan At Upper Const. Jt.) SECTION NEAR APPROACH BEAM SECTION E-E Note: For locations of Sections E-E and F-F, see sheet No. 4 For additional notes, see 35.4-UI (Spaced as shown) 10 Spaces @12" sheet No.4.
For location of Elev. C-C
see Sht. \$4. Const.Jt. Const. Jt. Key-10"x2" Const. St. - Key-2x4" -Fill Force of L 2.35-H15--2.#5-H14 -2-4-HI2 -234-HIZ & Roadway -2-4-H25 32-5-VB @ 12cts (Each Face Const. Jt. -Elev. 893.49 - 2-#4-V14 L2-44-T3 118' | G-4-V52 18'cts(Ea. Face) 15 18 18" 22 4-U3 (Speced os ' xun) SECTION F-F ټ٠ ر° 3-02 3-02 DETAILS OF ABUTMENT NO. 1 PLAN AT UPPER CONSTRUCTION JOINT **DETAILED JANUARY 1988** A-4738 CHECKED July 1983 Note: This drawing is not to scale. Follow dimensions. JACKSON COUNTY Sheet No. 3 of 24

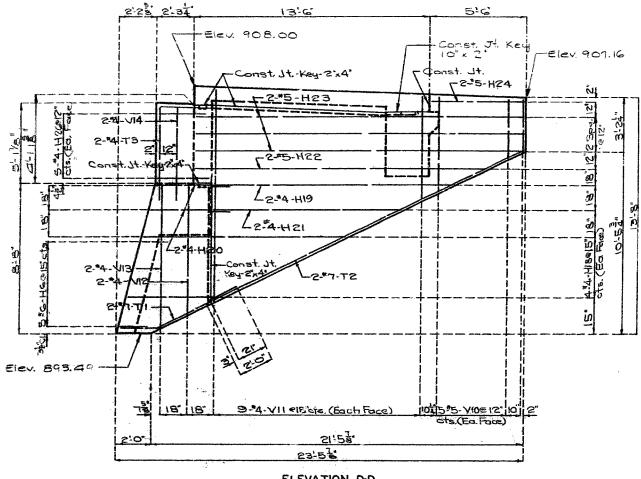
FOR INFORMATION ONLY A47382. Sht. 4 32-15- V3 912"chs (Each Face) 91 32.14.U3 (Spaced as shown in "Plan At Upper Const. Jt) 10-4-UA (Spaced as shown) 13:50. 9- 4.517@18 cts (TOA) 17:5-518@9" cts. (Bottom) F2-4-H9 11 Elev. 902.39--Elev. 902.16 r Elev. 908138 4.517 Const. Jt .- Ke + 2'x10'et Roodway Timber Header 15-518 -Pile Cut off Elev. 900.66 - Elev. 903.54 Const. St. Key 24-*G-H8 23- 5- V2@18 cts. (Each face) #-8-H3 S.-C. **ELEVATION** Elec 908,05 4:10 enily to gotte -Elev. 907.16 Elev. 893.49 etes of Wing 15 182 4:8-H7 FIII Face of Abutment SECTION A-A Note: Top of Abutment slab and expansion device for Abutment No. Land 3 Shall conform to crown of roadway elab. Abutment slab above the upper construction joint shall not be poured until the superstructure slab has been poured in the 44-5/3(Tap) 16-5/4(Butt) 17 aniforcent span.

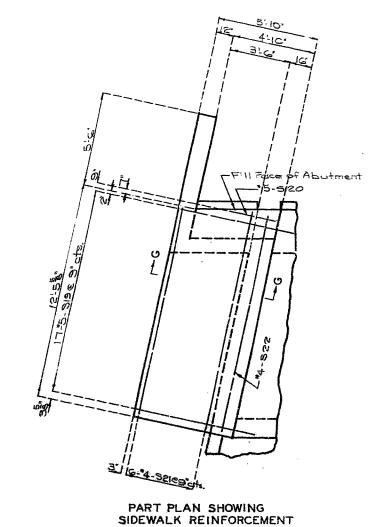
All reinfe cing bars in the tops of substructure beams or caps shall be spaced to clear anchor bolts for bearings by at least 2.

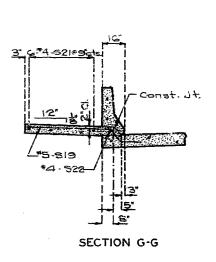
Field bending shall be required for *G-H3 and *4-H3 (Top) (B) BICTYP) -*4-S15@18"cts 57-78-816 e7 cts. (Bott) SECTION B-B Neoprene Brg Fod-18 x12 in backwall at wings.
For Elevations C-C&DD and Sections E-E&F-F, sec sheets No. 3 & 5. For details and reinforcement of barrier curb see sheet No. 20. Butt Splice (if required)
Top of lower section For location and details of Anchor Folt Wells see Street No. 10. to be cut square. See sheet No. 20 for details of Timber Keader. For details and reinforcement of Sidewalk not shown see sheet No. 19. ZFront Face of 2 DI DETAIL OF STEEL PILE SPLICE a;0§. 3413" PLAN DETAILS OF ABUTMENT NO. I DETAILED JANUARY 1988 A-4738 CHECKED July 1986 COUNTY **JACKSON** Note: This drawing is not to scale. Follow dimensions Sheet No. 4 of 24

A47382, Sht. 5









ELEVATION D-D

Note: This drawing is not to scale. Follow dimensions.

Note: For Incation of Elevation D.D see sheet No.4
For location of sidewalk on Abutment No.1 see sheet No.4.
For additional notes see sheet No.4.

DETAIL OF ABUTMENT NO. 1

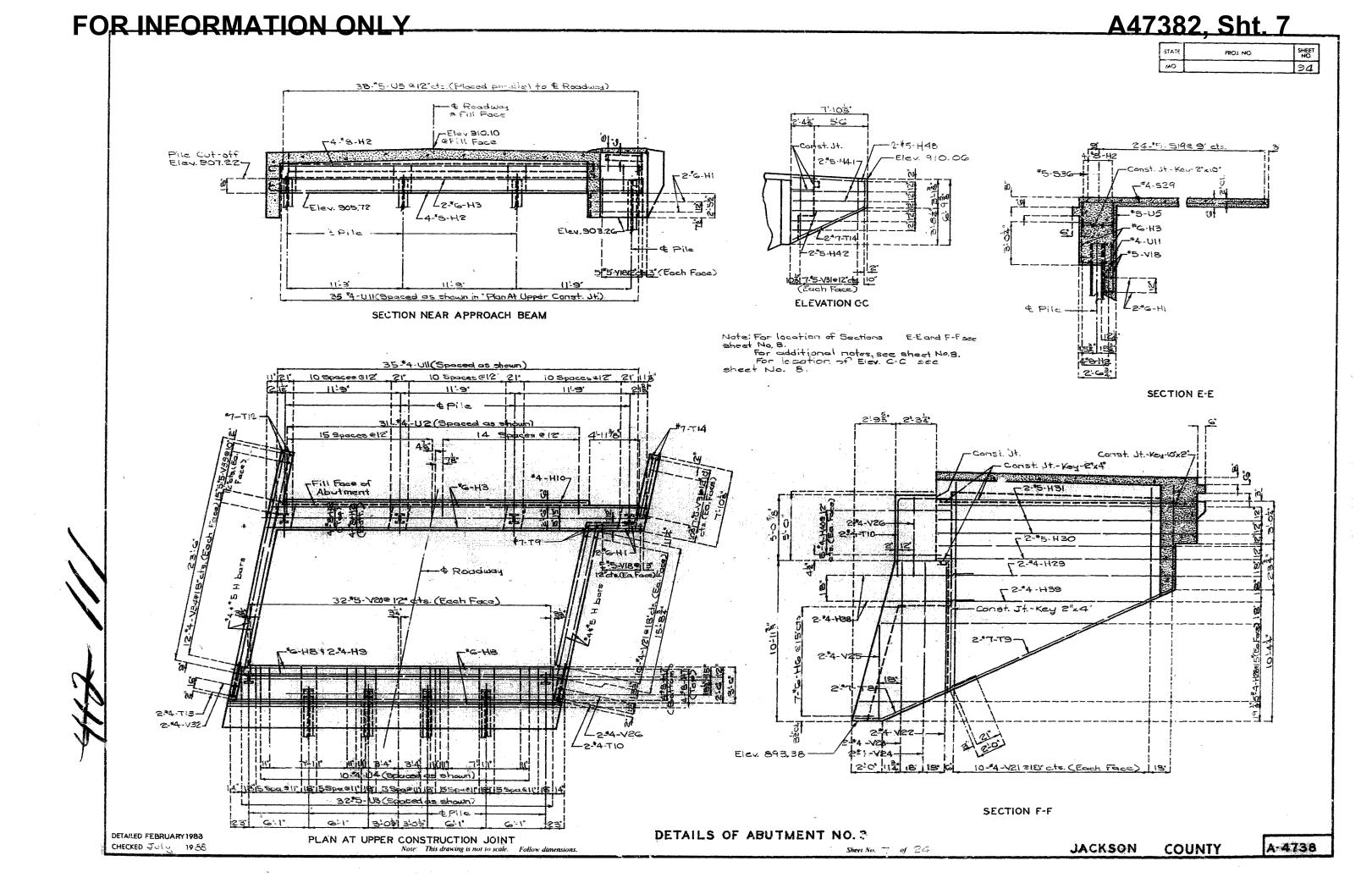
DETAILED FEBRUARY 1988 CHECKED July 1988

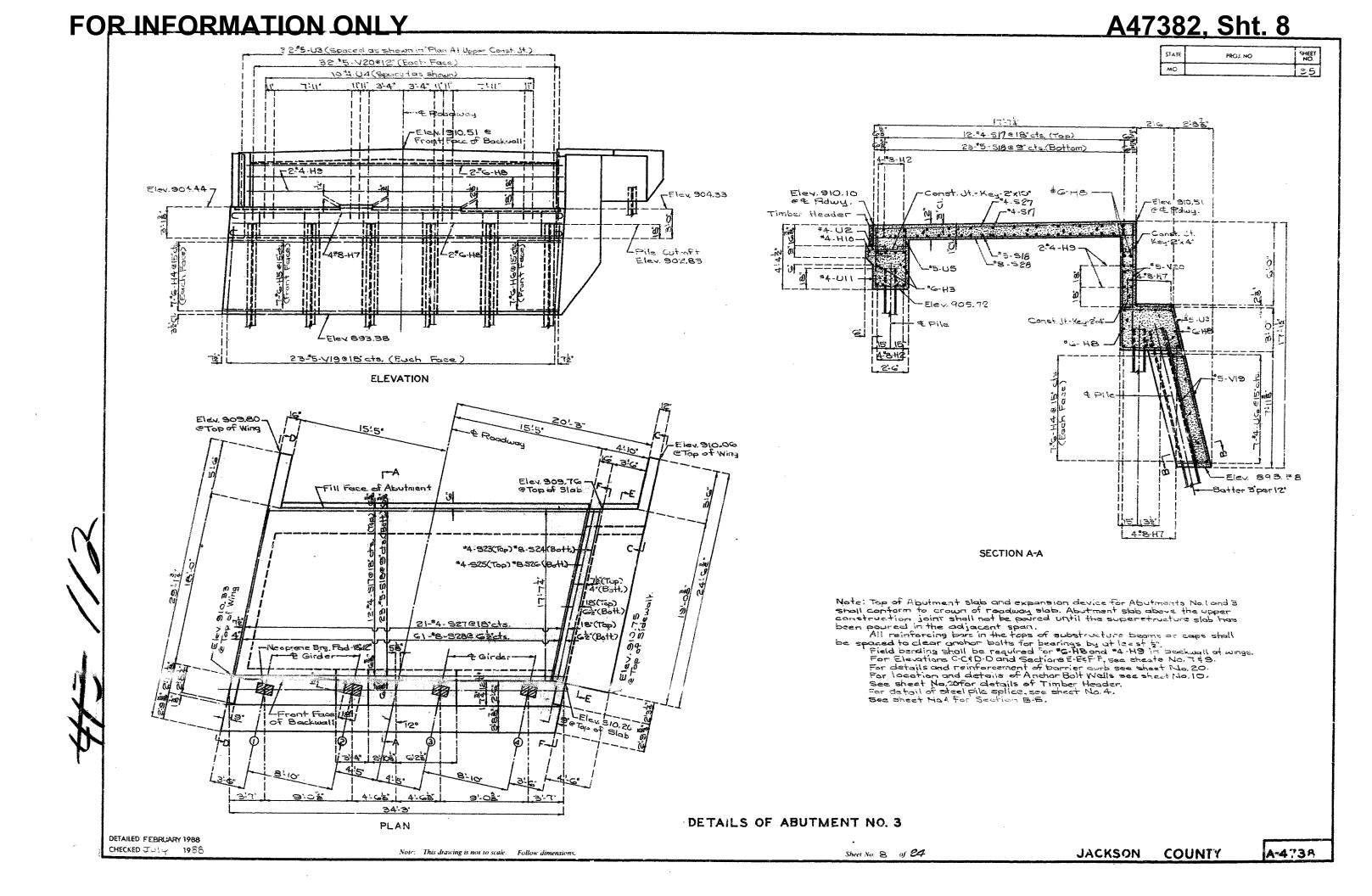
Sheet No. 5 of 24

COUNTY

A-4738

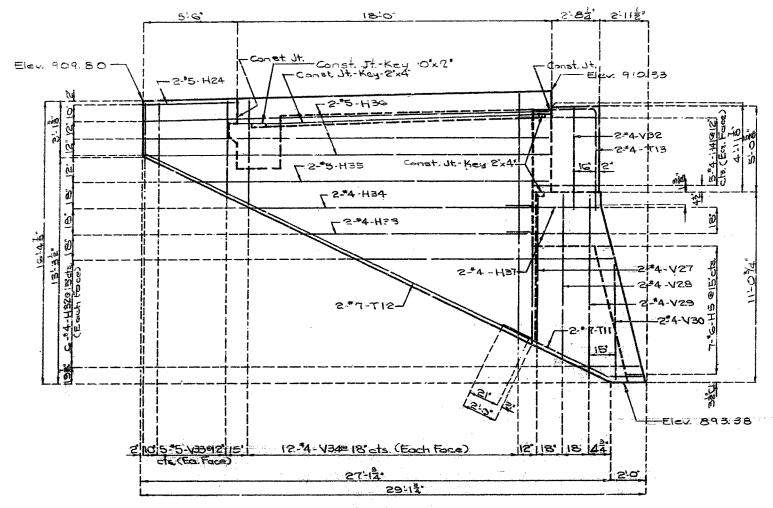
FOR INFORMATION ONLY A47382, Sht. 6 UB 11 12 16-6-UT (Spa. as shown 15'-11" 65pa.69 6 85pa.66 5-9-H45 -Symmabt. & Bent except as shown #G-U7 5-9-H45 18.40 e.v. 2-6-010-11 5-6-09 *9-H4G--*5-H46 5-19 H45 + B-1 *9-H46 -6-H44 B-LElev, 901.91 Const. Jt. "Ly-10'x10'x2" ·tL.tenco-& Column **€** Calumr #Column 5 6-9-H43 4:11 11,0, 11'-0' 4-11 SECTION B-B *8-VI7 9*8-VII 9.8-VI 2,0, -Const.ult Key-10x10x2* Cortst. Jt. Key-10:10:2 File Cut - 37 F & '5-D3 1 5.2 DI A (Not Above) Elev. 878.50 -Batter 2'per12' (Not Above) 6'-0" 3.2-C **ELEVATION** Note: All reinforcing bars in tops of 3'0" 3'0' substructure beams or cape shall be spaced to clear anchor bolts for 310, bearings by at least 2.
For location of Ancher Bolt SECTION A-A 4:11* 11'0' 11:0" Walls see sheet No.10. 910° 12468 + Readway # Structure Meoprane / Neoprane 2-6-U10--2-4-010 2-6-49-246-09 4-17-447 delt & Bent L=G-DI 8:10. 9:08 aro§. 9:0 X PLAN OF FOOTING DETAILED MARCH 1988 PLAN DETAIL OF INT. BENT NO. 2 CHECKED July 1988 A-4738 JACKSON COUNTY Note: This drawing is not to scale. Follow dimensions.





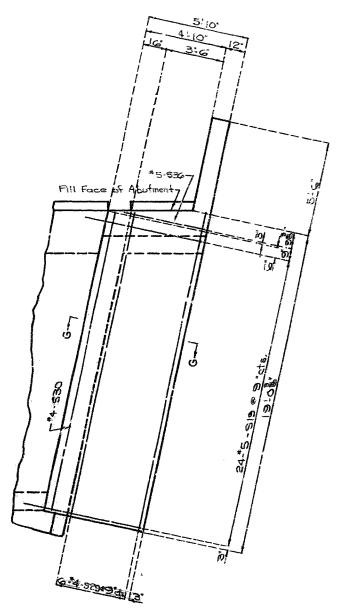
A47382 Sht. 9

STATE	PROJ. NC.	SHEET NO.	
WO		96	l



ELEVATION D-D

Note: For location of Elevation D-D see sheet No. 8.
For location of sidewalk on Abutment No. 3 see sheet No. 8.
For additional notes see sheet No. 8.
See sheet No. 5.



PART PLAN SHOWING SIDEWALK REINFORCEMENT

DETAILS OF ABUTMENT NO. 3

AA

DETAILED FEBRUARY 1988 CHECKED July 1988

Sheel No. 9 of 24

KSON CO

COUNTY

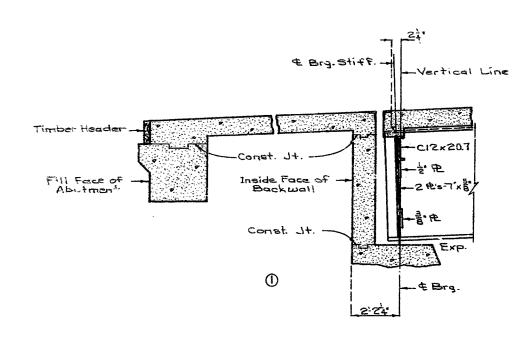
A-4738

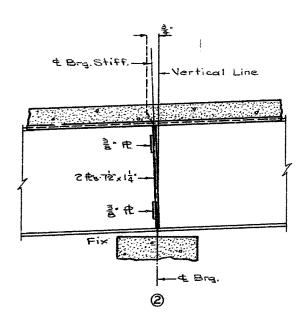
Note: This drawing is not to scale. Follow dimensions.

JACKSON

A47382. Sht. 10

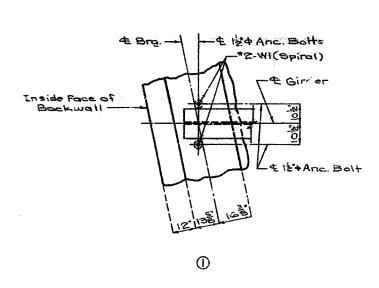
STATE	PROJ. NO.	SHEET NO.	
MO		97	

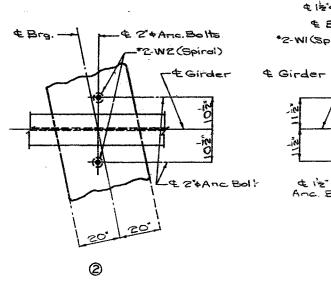


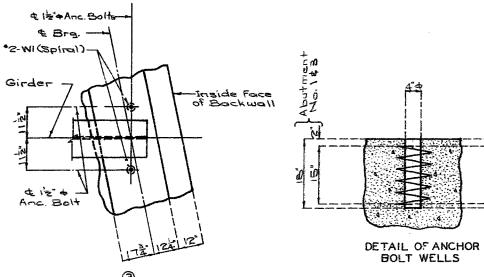


Vertical Line-Timber Header C12×20.7-章 电 -Fill Face of Abutment Backwall Const. Jt.

PART LONGITUDINAL SECTION







PART ANCHOR BOLT PLAN

DETAILED JANUARY 1988 CHECKED July 1988

Note: This drawing is not to scale. Follow dimensions.

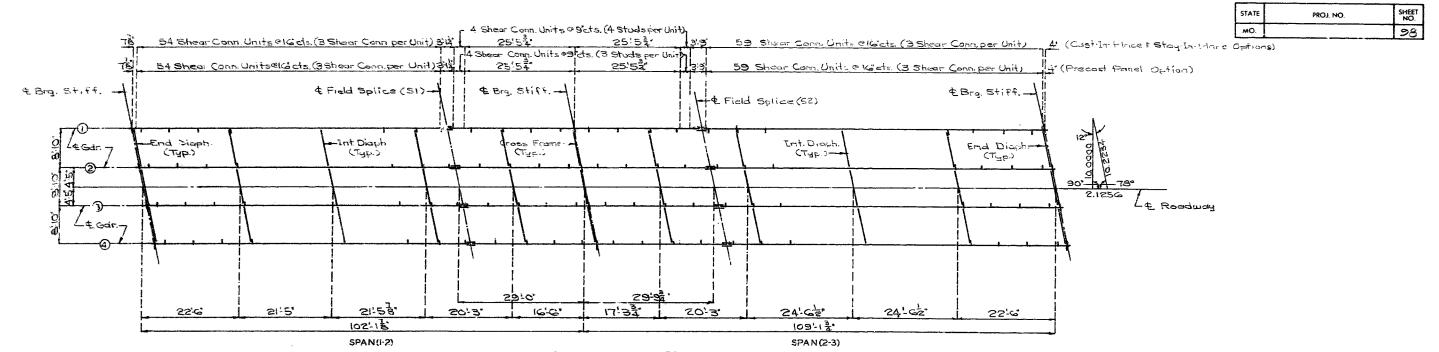
Shra No. 10 of 24

JACKSON

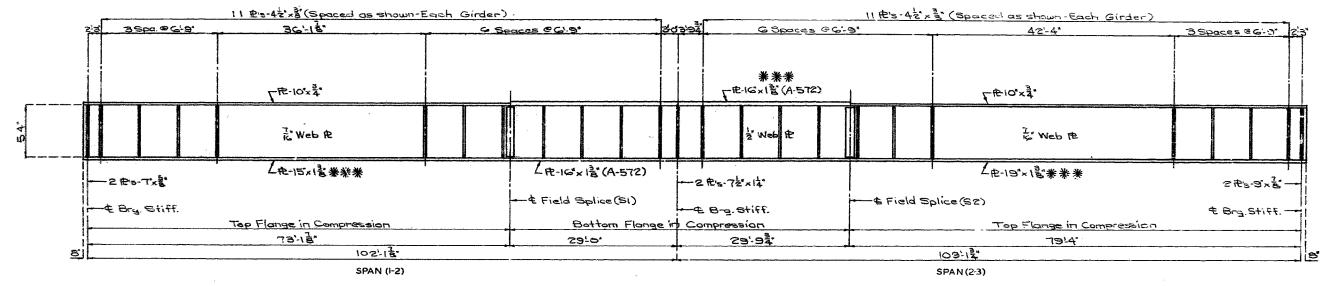
A-4738

COUNTY

A47382, Sht. 11







ELEVATION OF GIRDER

Note: Plate girders shall be fabricated to conform to the Camber Diagram shown on sheet No. 15.

Transverse web stiffeners shall be located as shown in Plan of Structural Steel.

Intermediate web stiffener plate and diaphragm spacings may vary from plan dimensions by a maximum of 3° for diaphragm to connect to the intermediate web stiffener plate.

*** Indicates Flange Plates subject to notch toughness requirements.

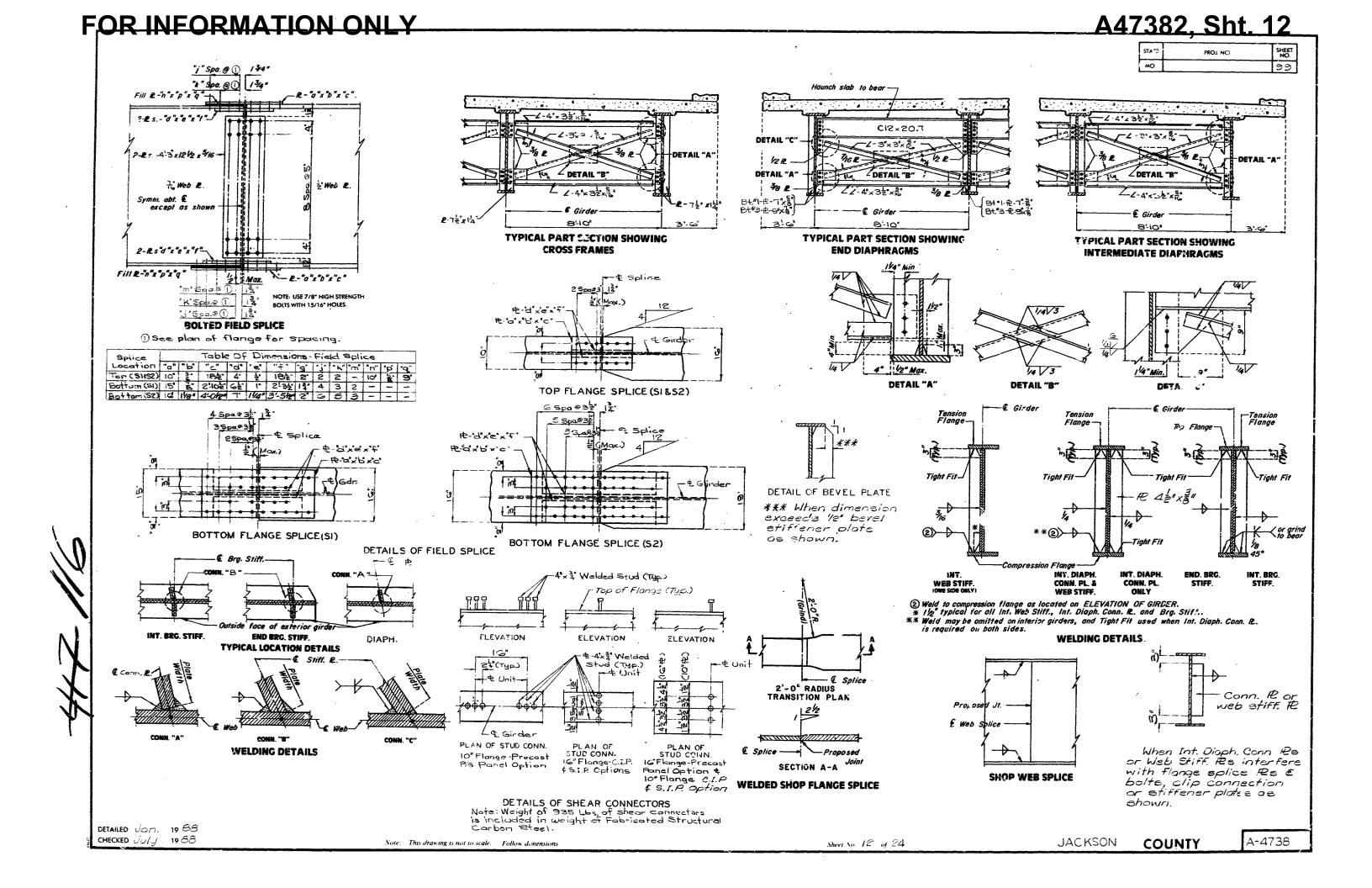
Fabricated structural steel shall be A36 except as noted.

Longitudinal dimensions are along top of web. See Part-Longitudinal Sections on sheet No. 10.

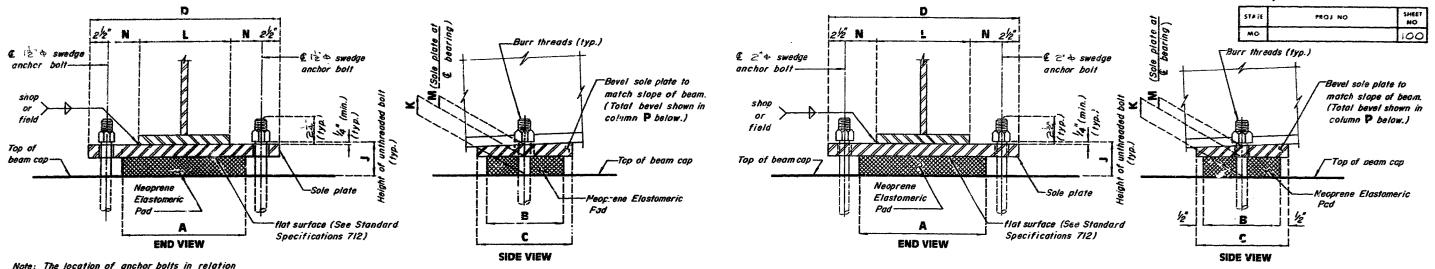
All web plates shall be subject to notch toughness requirements.

11-94

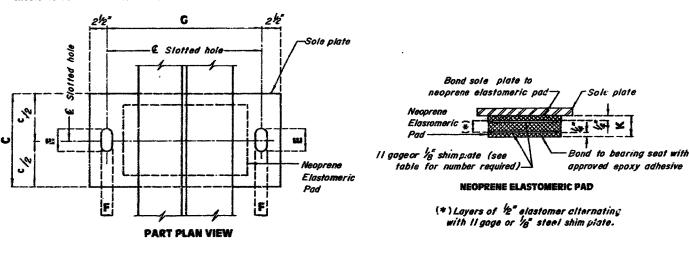
DETAILED JANUARY 1988 CHECKED July 1988

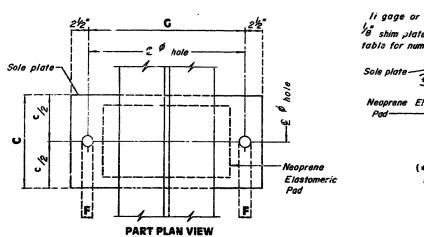


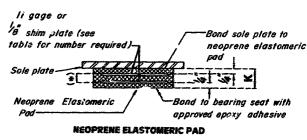
A47382. Sht. 13



Note: The location of anchor bolts in relation to the slotted holes in the sole plate shall correspond with the temperature at the time of erection. At 60°F, the slotted holes should center on the anchor bolts.







(*) Layers of ½" elastomer alternating with II gage of ½" steel shim plate.

EXPANSION BEARINGS

NUMBER REQUIRED = 4@ Abotment No.1 4@ abotment No.3

FIXED BEARINGS

NUMBER REQUIRED = 4 @ Int. Bt. Na 2

	EXPANSION BEARINGS													
BENT NO.	A	B	C	D	E	F	G	J	K	L	M	N	P	NUMBER OF SHAM PLATES (*)
ţ	15,	15,	13"	SC.	5	15	51,	5%	34	15	14"	3	2	Ço
3	187.	15.	19,	58.	5	nie)	23	5§.	3 [‡]	19"	15.	5.	4	G ₂

(*) 16.5 REQUIRED SHIM PLATE SHALL BE PLACED BETWEEN LAYERS OF ELASTOMER AND MOLDED TOGETHER TO PORM AN INTEGRAL UNIT.

** ... (Abutment Na.133) 18(Int.3t.No.2) ***12" (Abutment No.153) 2"中(Int.No.2)

GENERAL NOTES:

ANCHOR BOLTS SHALL BE業業業AS88 STEEL SWEDGED BOLTS AND SHALL EXTEND業業NTO THE CONCRETE WITH A 194 - 2, 2H Or A563 - C, C3, D, DH, DH3 HEAVY HEAGON NUTS. ACTUAL MANUFACTURER'S CERTIFIED MILL TEST REPORTS (CHENICAL AND MECHANICAL [INCLUDING PROOF GOADS) SHALL BE 2/ROVIDED. (SWEDGING SHALL BE 1"LESS THAN EXTENSION INTO THE CONCRETE.)

ALL STRUCTUPAL STEEL FOR THE SOLE PLATE, ANCHOR BOLTS AND HEAVY HEXAGON NUTS SHALL BE PAINTED WITH 2 COATS (5 MILS MIN) OF INORGANIC ZINC, WELD AREAS TO BE TOUCHED UP AFTER ASSEMBLY.

WEIGHT OF ANCHOR BOLTS AND HEAVY HEXAGON NUTT FOR BEARINGS SHALL BE INCLUDED IN WEIGHT OF THE FABRICATED STRUCTURAL STETS.

The sole plate shall be furnished with the bearing and held or shop welded to the stringers or garders.

STRUCTURAL STEEL FOR SOLE PLATE SHALL BE A-36.

NEOPRENE ELASTOMERIC PADS SHALL BE 60 DUROMETER.

PAYMENT FOR THE SOLE PLATE WILL BE INCLUDED IN THE COST OF THE BEARING ASSEMBLY. SEE SPECIAL PROVISIONS.

THE ACCEPTED QUANTITY OF ELASTOMERIC BLARING ASSEMBLIES, COMPLETE IN-PLACE, WILL BE PAID FOR AT THE CONTRACT UNIT PRICE FOR LAMINATED NEOPPRENE BEARING PADS (STEEL STRUCTURES), EACH.

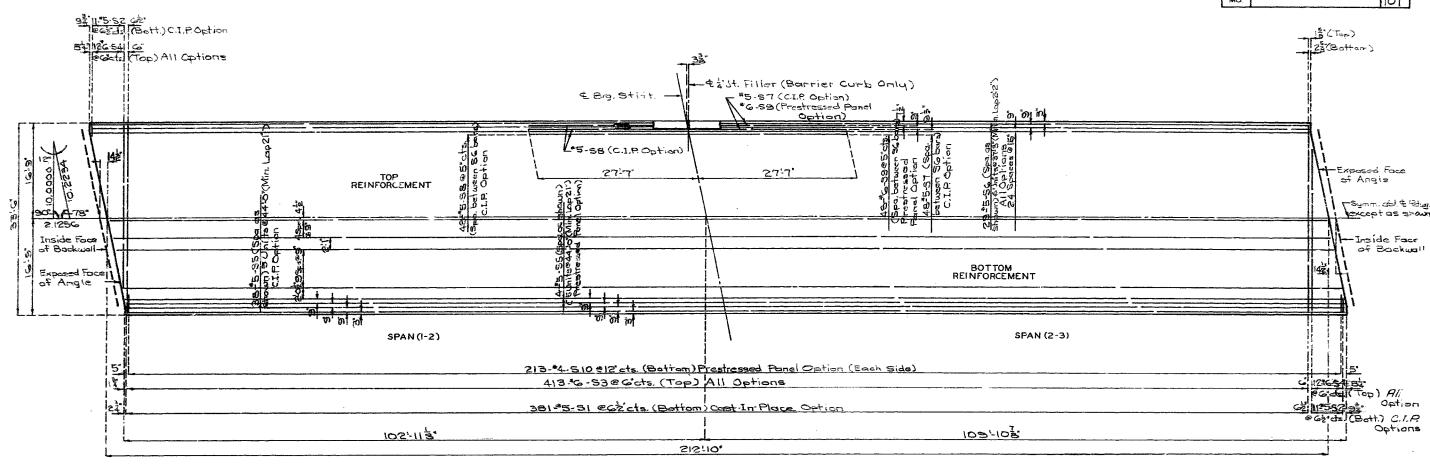
	FIXED BEARINGS													
	BENT NO.	A	В	C	D	F	G	J	K	L	M	N	P	NEJAMBER OF SHIM PLATES (*)
Gdc1,244	5	18"	30"	311	50,	25	51.	Ste	38.	16	116	5 5°	g) tg	5
Gdr.3	2	18"	30"	31.	26	28	51.	5 2	38.	16	15	2 <u>5</u> ,	∄.	5
														<u> </u>

(*) THE REQUIRED SHIM PLATE SHALL BE PLACED BETWEEN LAYERS OF ELASTOMER AND MOLDED TOGETHER TO FORM AN INTEGRAL UNIT.

DETAILS OF LAMINATED NEOPRENE BEARINGS (STEEL STRUCTURES)

CHECKED JAN. 1988 CHECKED July 1988

A47382, Sht. 14 SHEET NO. 101



PLAN OF SLAB SHOWING REINFORCEMENT

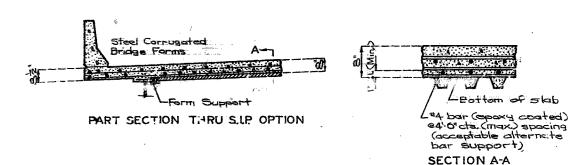
Note: Longitudinal dimensions are parallel to grade along top of Flab.

Transverse steel in Glab near exponsion device shall be shifted in field to clear & A.

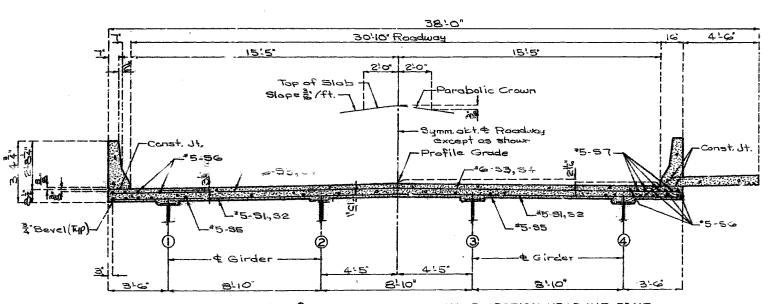
clear & M.
Longitudinal reinforcing steel shall be placed so that the ends shall not be more than I't from vertical leg of angle for Expansion Device.

For details of Alternate Slab Forming methods see sheet No.16.

See sheet No.15 for "Slab Souring Sequence" and "Theoretical Slab Hounch:



The corrugation valleys of forms shall match the bottom transverse reinforcing steel spacing.
Slab reinforcement for S.I.P. Option to be the same as for C.I.P. Option.



HALF SECTION NEAR INT. BENT HALF SECTION NEAR & SPAN Note: For details and reinforcement of sidewalk not

shown, see sheet No. 19.

For details and reinforcement of safety barrier curb not shown, see sheets No. 20 \$ 21.

Cast-in-place option shown. For details of precast

panel option see sheet No. 16.

DETAILED JANUARY 1988 CHECKED July 1988

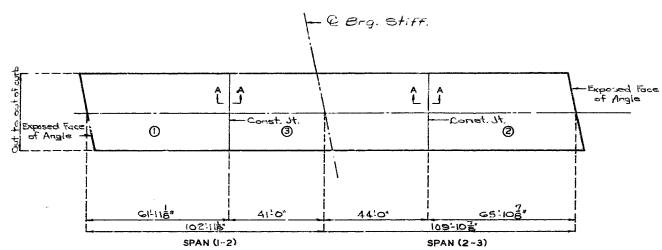
Note: This drawing is not to scale. Follow dimensions.

Sheet No. 14 of E4

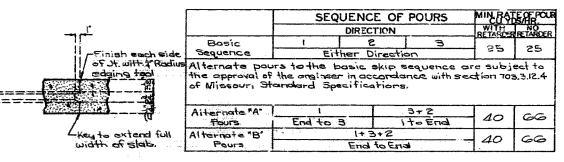
JACKSON COUNTY A-4738

A47382, Sht. 15

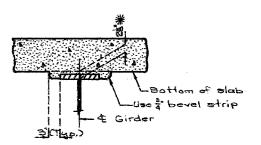
SHEET



SLAB POURING SEQUENCE



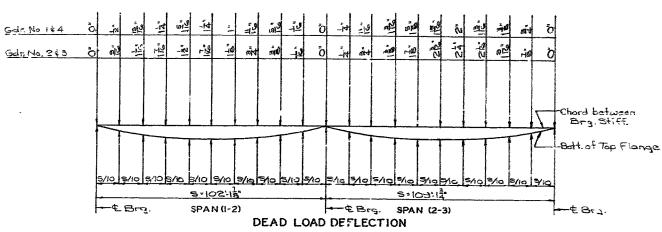
Note: The contractor shall pour and satisfactorily finish the slab pours at the rate given. Retarder, if used, shall be an approved type and retard the set of concrete to 2.5 hours.



THEORETICAL SLAB HAUNCH (C.I.P.)

*Dimension may vary if girder comber after erection differs from plan camber by more than the % of D.L. deflection due to weight of Structural Sizel No payment will be made for additional forming or concrete required for variation in haunching.

To determine the hounch for Stay in Place Alternate, add 16 to the hounch for the Cast in Place Alternate.



Note: 12% of dead load deflection due to weight of Structural Steel.

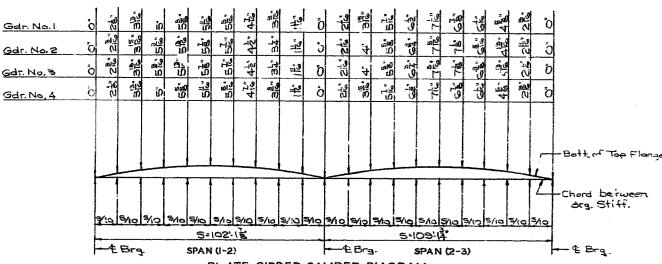


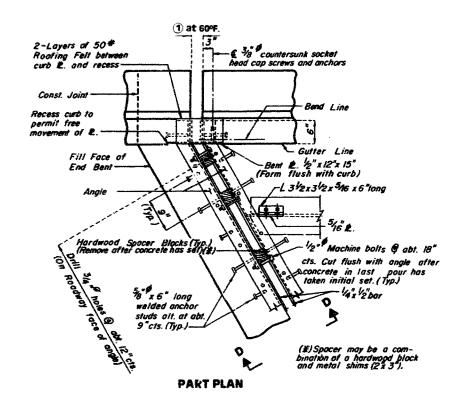
PLATE GEDER CAMBER DIAGRAM

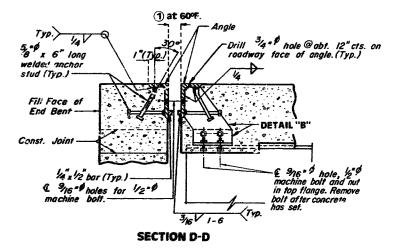
Note: Comber includes allowance for vertical curve and for dead load deflection due to concrete slab, curb and structural steel.

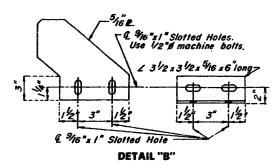
Note: This drawing is not to scale. Follow dimensions.

A47382. Sht. 16

STATE	PROJ NO	SHEET
MO		104





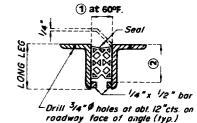


GENERAL NOTES:

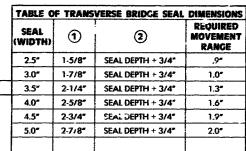
- STRUCTURAL STEEL FOR EXPANSION DEVICE SHALL BE FABRICATED IN ONE SECTION, EXCEPT THAT WHEN THE LENGTH IS OVER 50', SPLICING IS PERMISSIBLE.
- THE EXPANSION DEVICE SHALL BE BENT TO CONFORM TO CROWN AND GRADE OF ROADWAY.
- STRUCTURAL STEEL FOR THE ARMORED JOINT SHALL BE GRADE A36.
- ANCHORS FOR COMPRESSION SEAL ARMOR SHALL BE APPROVED STUD WELDED ANCHORS (C1010 THRU
- PLAN DIMENSIONS ARE BASED ON INSTALLATION AT 60° F.
- DIMENSIONS (1) SHALL BE INCREASED $\frac{1}{6}$ " FOR EACH 10° FALL IN TEMPERATURE AND DECREASED $\frac{1}{6}$ " FOR FACH 10° RISE IN TEMPERATURE AT INSTALLATION.
- SEE SPECIAL PROVISIONS FOR THE REQUIREMENTS OF COMPRESSION JOINT SEAL

NEOPRENE EXTRUSIONS SHALL MEET A.S.T.M. D3542-83.

Furnishing, pointing and installing the structural steel ormored joint, skid resistant plate on sidewolk and curb plates shall be included in the contract unit price for Preformed Exponeion Joint Seal.



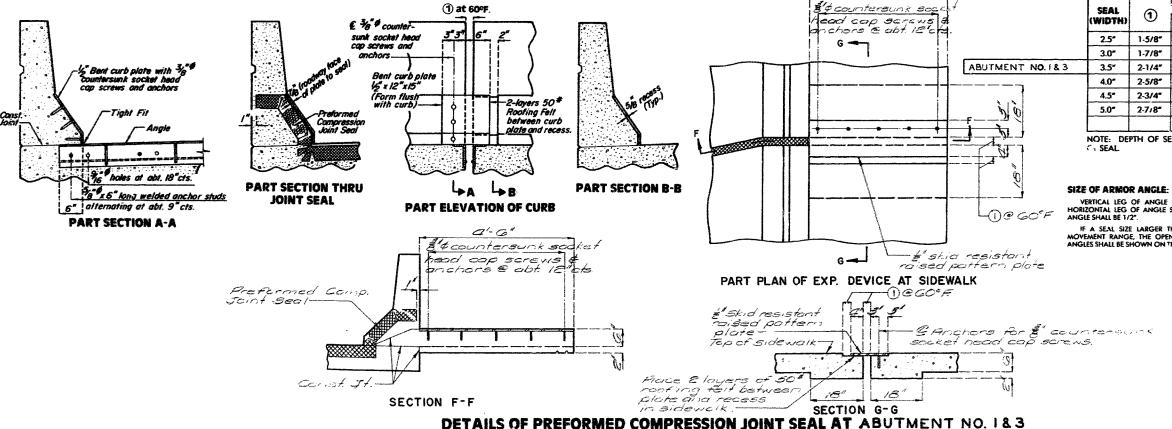
PART CROSS SECTION THRU EXPANSION JOINT



NOTE: DEPTH OF SEAL SHALL NOT BE LESS THAN WIDTH

VERTICAL LEG OF ANGLE SHALL BE A MINIMUM OF DEPTH OF SEAL \pm 1-1/2". HORIZONTAL LEG OF ANGLE SHALL BE A MINIMUM OF 3". MINIMUM THICKNESS OF ANGLE SHALL BE 1/2".

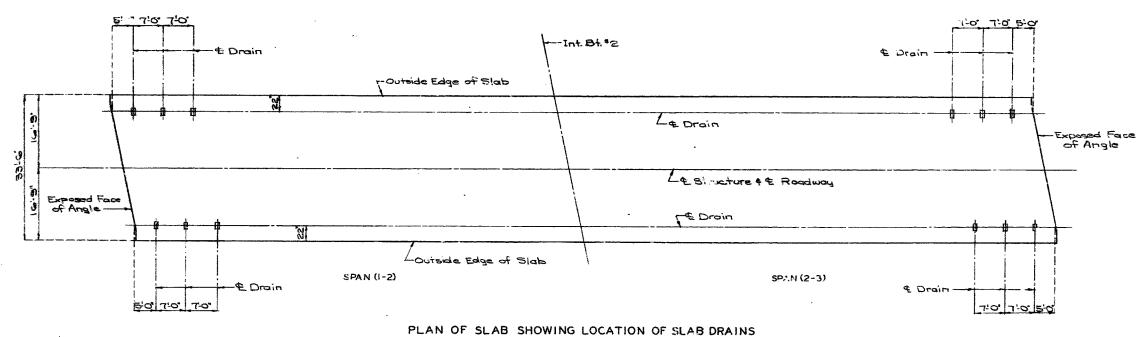
IF a seal size larger than that indicated on the plans is used, the movement range, the opening at 60° and all dimensions for the armor angles shall be shown on the shop drawings.



PA PB

DETAILED JAN. 1988 CHECKED July 1985 0-6"

FOR INFORMATION ONLY A47382, Sht. 17 £ 1 2 x 716 slotted holes, £ 76 bolt with 2 € 1 2 x 76 stotted holes, € 76 bolt with 2 -Bar 1/4"x 1" (A.S.T.M. A36) PROJ NO hardened washers, hardened washers, Flange Plate lock washer and nut-Plate € 38 " bolt with € 3/8 " bolt with & Drain lock washer and lock washer 13/2x3/2x4nut (Typ.) and nut (Typ.)-GENERAL NOTES: 132 x 3/2 x slab drains may be fabricated of either $\%^\circ$ welded sheets of a.s.t.m. a36 steel or from $\%^\circ$ stre. Fural steel tubing a.s.t.m. a500 or a501. OUTSIDE DIMENSIONS OF DRAINS ARE 8" x 4". LOCATE DRAINS IN THE SLAP BY UMENSIONS SHOWN IN THE PART ELEVATION. E Bolt with SHIFT REINFORCING IN FIELD WHERE NECESSARY TO CLEAR DRAINS. THE DRAINS AND BRACKET ASSEMBLY SHALL BE GALVANIZED IN ACCORDANCE WITH A.S.T.M. A.123. Bent strip-10 lock washer and with lock Bent strip -10 washer and ALL BOLTS, HARDENED WASHERS, LOCK WASHERS AND NUTS SHALL BE GALVANIZED IN ACCORDANCE WITH A.S.T. W. AT 53. gage (min.) x 2 THE BOLT HOLE FOR THE BRACKET ASSEMBLY ATTACHMENT SHALL BE LOCATED ON THE PLATE GIRDER SHOP DRAWINGS. SHOP DRAWINGS WILL NOT BE REQUIRED FOR SLAB DRAINS AND BRACKET ASSEMBLY. PART SECTION SHOWING BRACKET ASSEMBLY PART SECTION ST. DWING BRACKET ASSEMBLY **ELEVATION OF DRAIN** Rod 12"0 x 3" (A.S.T.M. A36) or Shear Connector 12" x 3"+ (Typ.) PART PLAN OF SLAB AT DRAIN PART PLAN OF SLAB AT DRAIN **DETAILS OF DRAINS TRANSVERSE TO ROADWAY DETAILS OF DRAINS PARALLEL TO ROADWAY** PART ELEVATION OF SLAB AT DRAIN (*) If dimension is less than I", drains shall be placed parallel to roadway, otherwise place drains transverse to roadway. PLAN OF DRAIN **SLAB DRAIN DETAILS** <u>" 7'5" 7'5" "</u> 1-0 7:0 5-0



SHEET NO.

105

STEEL

DETAILED Jan. 1988

CHECKED July 1988

FOR INFORMATION ONLY A47382, Sht. 18 * 1/4" Gap for splice PROJ NO -Fabric Ties at abl 15" cts. (Typ.) 1 8 Gap at 60 F. __ 2" 0.D. x 10" long tube (Typ.) Stretcher Bar Band (Typ.)-2" Pipe (Typ.) this panel for Provide vent holes expansion. -2" # Pipe at all internally closed joints for 16" Rod (Typ.) galvanizing (Typ.) Stretcher Bor Outside face of sidewalk Typ. \\ \frac{1}{1/8} -7"x 6" x 1/2" Floor 12. (Typ.) AT END OF WING SECTION THRU FENCE AT EXPANSION GAP AT SPLICE GAP Note: Connect the lower and of gip roll to PART ELEVATION OF 6'-0" (GALVANIZED STEEL) GUARD FENCE the end of braced panel to which the stretcher bor is attached. * At about 30'-0" centers, with at Note: Maximum spacing allowed for braced least one splice gap between pull posts. panels (pull posts) is 100 ft. 4'-0" "P Pipe Posi Pipe Post € 2" Pipe Post Top of concrete #4-5 bar Slope 'A" per foot Const. Joint -4" plastic waterstop Floor R. Std. Spec. 1057.2.1 Const. Joint (centered on joint)-Outside Face of sidewalk -S bars at abl. ∌"cts-\ Plastic waterstop shall be placed in all safety barrier 15/6 × 1/6 slotted bars (8"lona) (توپي ۲) curb and sidewalk filled joints. with hex nuts and Tack weld to U-bolts holes for U - bolts #4-S bars at abt. 9"cts. Cost of plastic waterstop complete in place to be included in contract unit price for concrete. **FENCE POST CONNECTION** PLAN OF FLOOR PLATE TYPICAL SECTION THRU SIDEWALK **DETAILS OF PLASTIC WATERSTOP** - & Int. Bt. No. 2 NOTES: (20° ac 11/2-12/2014 - 14/16 16 8 - 14/16 8 - Pedestrian guard fence (chain link type) shall be in accordance with Section 1043 of the Std. Spec., except all fabric shall have top Fill Face of Abut ment No.1 and bottom edges knuckied. Fill Face of Abutment No.3 All fence posts shall be vertical. Grout of 2" minimum thickness shall be placed under floor plates to provide for vertical alignment of fence posts. & Fence Post The contract unit price per linear foot for pedestrian guard fence (galvanized) shall include furnishing and erecting the fence complete with U - anchor bolts, hex nuts and washers. Measurement of pedestrian guard fence shall be taken parallel to -et J. Filler grade through the centerline of posts. & Fence Post(Typ.) -A Callosson till et en gløting kollo am en ngenad lid en a gløtisk ill and nøljani et ennad 284-15-519@9" 12/4 TO THE CONTROL OF THE 6 5 7 5 7 5 3 7 7-12 15 Spaces @7:10 14 Sp. 15 @7-10" SALECTE CO. E. 21600 @ 60'F. 17'-11量" 102-118 109-108 Note For details of sidework Note: For details of sidewalk reinforcement on Abutment No. 3 see sheet No. 9. reinforce will on Aput ment No. I ere sheet No.5. PLAN OF SIDEWALK SHOWING REINFORCEMENT AND LOCATION OF FENCE POSTS DETAILED MARCH 1988 CHECKED I 1955 JACKSON COUNTY A-4738 Note: This drawing is not to scale. Follow dimensions. Sheet No. 19 of 22.

FOR INFORMATION ONLY A47382, Sht. 19 SHEET 107 102-118" 109'-105" 19-0" 23'-6" 10'-0" 10'-0" 221000 24 GOD @ \$0°F @ a" St. Filler (Borrier Curb Only) @ 60° F #5-R22 End ruetication -#5-121 #5-R23 (Typ.)-----2-#5-R22 -C-#5-RC3 217-#5-R1,R2,R3 & R4 (Spo. as shown in Port Section Near Left Borrier Curb) 5PAN (1-2) SPAN (2-3) SECTION NEAR LEFT BARRIER CURB Note: Longitudinal dimensions shown parallel to grade along outside to edge of slob. Rustication not shown for clarity. C/L 3/4"@x8" Lag Bolt 3" at obt. 3'-0" cts. Top of Safety 3"x10" Timber He Barrier Curb-3*x10" Timber 3"x8" Wood Block or Optional 3" Wedge Blocks #5-83 PART ELEVATION SECTION A-A 991 RUSTICATION DETAIL DETAILS OF TIMBER HEADER AT END BENTS TOP OF SAFETY BARRIER CURB SHALL BE BUILT PARALLEL TO GRADE WITH SAFETY BARRIER CURB JOINTS (EXCEPT AT END BENTS) NORMAL TO GRADE. ALL EXPOSED EDGES OF SAFETY BARRIER CURB SHALL HAVE EITHER A 1/2" 19'-0" SECTION F-F RADIUS OR A 3/8" BEVEL, UNLESS OTHERWISE NOTED. -RI BRE"* 14. *5-RI ,RE ,R3 BR4 18-5-R1 ,R2,R3 BR4 WHEN THE SAFETY BARRIER CURB IS BID BY LINEAR FEET, THE CONTRACT LINIT PRICE SHALL INCLUDE THE COST OF ALL CONCRETE AND REINFORCEMENT, COMPLETE-IN-PLACE. 11 4-45-RI BR2 (Spo. os shown) 13 Spo. at Abt. 12 (Spaced as shown) 17-spaces at 12" € CONCRETE FOR THE SAFETY BARRIER CURB SHALL BE CLASS 81. PART ELEVATION DETAIL "F" MEASUREMENT OF SAFETY BARRIER CURB IS TO THE NEAREST LINEAR FOOT FOR EACH STRUCTURE, MEASURED ALONG THE OUTSIDE TOP OF SLAB FROM END OF WING TO END OF WING. (TYPICAL) \$5-R15 TE-R2 \$5-R1 *5-R13 5-R/3 OFTAIL "F DETAIL "F" -*5-R20- Const. **ELEVATION C-C** SECTION E-E SECTION B-B **DETAILS OF GUARD RAIL ATTACHMENT** ** 5-#5-RI, RZ, RI3 ELEVATION 3'(Typ.) "**" 3 Spa. at 12" *PIN 'Spa as shown) 19'-0" 23-6 727 -Endrustication #5-RI BR2## |4-#5-RI RC R3 BR4
Spaced as | (Spaced as shown) 4" Joint 18-#5-R1 ,R2 ,R3 8R4 "X" 4-#5-R1 8R2 −€ Joint Filler 13 500. of Abt. 12" 5-R bar-17- spaces at 12" #5-R bar --2-5-Roors 2-75-R bars #5-Rbar 7 4" plastic watersiop Std. Spec. 1057.2.! (Centered on joint) P 5-Rbar Joint Filler(Std. NOTE: PLASTIC WATERSTOP SHALL BE PLACED IN ALL SAFETY BARRIER CURB FILLED JOINTS. EXCEPT Spec. (057.2.4) FILLED JOINT DETAIL STRUCTURES WITH SUPERSLEVATION, USE ON ALL LOW, ER SAFETY BARRIER CURB JOINTS ONLY). 5-RI ,R2 ,R3 AR4 #5-R17 Z#5-R20 L#5-R19 SECTION D-Q 212 45-R1 ,R2 ,R3 8R4 at abt. 12 cts Note: Use a minimum lap of 17" for barrier curb bars. COST OF PLASTIC WATERSTOP COMPLETE IN PLACE TO BE INCLUDED IN CONTRACT UNIT PRICE FOR SAFETY BARRIER CURB. PLAN ABUT. NO. 1 ABUT. NO. 3 PART SECTION NEAR LEFT SAFETY BAFRIER CURB DETAILS OF PLASTIC WATERSTOP DETAILED JUNE 1988 **DETAILS OF SAFETY BARRIER CURB AT END ABUTMENTS** 4-4738 CHECKED JU/4 1988 JACKSON COUNTY Mere Vo. 20 of 24. Note: This drawing is not to scale. Follow dimensions.

FOR INFORMATION ONLY A47382, Sht. 20 MO 13'-25" 102-115 109'-10a" 18'0" 10-0" 10-0" EZ" GOP ZA" GOP BBO°F € å" Jt. Filler (Barrier Curb Only) 9 90° F End rustication #5-122 -#5-**@**2/ - \$5-R23 (Typ.) -2-#5-R21 2-#5-B22 2-#5-R23 217 - #5 - RI, R2, R3 & RA (Spa. 08 shown in Port Elev. Near Right Barrier Curb) 'AN (1-2) SPAN (2-3) ELEVATION NEAR RIGHT BARRIER CURB Notes Longitudinal dimensions shown are parallel to grade along outside top edge of slab.
Rustication not shown for clarity. Sidewalk not shown for clarity. at abt. 3'-0" cts. Top of Safety 3"x10" Timber Heade Barrier Curbo ₹5-R/ or Optional 3* PART ELEVATION SECTION A-A 799/ NOTE: DETAILS OF TIMBER HEADER AT END BENTS RUSTICATION DETAIL TOP OF SAFETY BARRIER CURB SHALL BE BUILT PARALLEL TO GRADE WITH SAFETY BARRIER CURB JOINTS (EXCEPT AT END BENTS) NORMAL TO GRADE. ALL EXPOSED EDGES OF SAFETY BARRIER CURB SHALL HAVE EITHER A 1/2" RADIUS OR A 3/8" BEVEL UNLESS OTHERWISE NOTED. 13-28" 12'-0" SECTION F - F WHEN THE SAFETY BARRIER CURB IS BID BY LINEAR FEET, THE CONTRACT UNIT PRICE SHALL INCLUDE THE COST OF ALL CONCRETE AND REINFORCEMENT, COMPLETE. IN-PLACE. 18-#5-R1 ,R2 ,R3 8R4 13-5-RI ,RE ,R3 BR4 4-45-R / BR 2 & 8 (Spaced as shown) (G-spaces at ASt. 12" (Spaced as shown) CONCRETE FOR THE SAFETY BARRIES CURB SHALL RE CLASS B1. PART ELEVATION DETAIL "F" MEASUREMENT OF SAFETY BARRIER CURB IS TO THE NEAREST LINEAR FOOT FOR EACH STRUCTURE, MEASURED ALONG THE OUTSIDE TOP OF SLAB FROM FILL FACE TO FILL FACE. (TYPICAL) #5-RIO-DETAIL "F DETAIL "F" 2-45-R8 --2-45-RII **ELEVATION C-C** SECTION E-E SECTION B-B **DETAILS OF GUARD RAIL ATTACHMENT** 3 (Typ.) SECTION /3'-2₹ 18-0 > End rustication 4" Joint 18-45-H1 RE R3 BR4 13-#5-R1 R@ R3 BR4 Joint Filler (Spaced as shown) 18 18 RI 16- spaces at Abt. 12" - spaces at Abt. 12 #5-Rbar-#5-Ribar 2-45-Rbars 2 5 R. Bars #5-Rbor-4" plastic waterstop Std. Spec. 1057.2.1 (Centered on joint) NOTE: PLASTIC WATERSTOP SHALL BE PLACED IN ALL SAFETY BARRIER CURB FILLED JOINTS. (EXCEPT STRUCTURES WITH SUPPRESENSATION, USE ON ALL LOWER SAFETY BARRIER CURB JOHNTS ONLY). Spec. (057.2.4) FILLED JOINT DETAIL 5-RI .RZ .R3 BR4 22 L#5-RI2 SECTION D-D 2"\|8"|8" Note: Use a minimum log of 17" for 5 horizontal sat barrier curb bars. The cross-sectional area above the slab = 2.27 sq. ft. 2/2" "5-R1 ,R2 ,R38R4 at abt. 12" cis COST OF MASTIC WATERSTOP COMPLETE IN MACE TO SE INCLUDED IN CONTRACT UNIT PRICE FOR SAFETY SARRIER CURB. Fill Face **PLAN** of Abut. ABUT. NO. 3 ABUT, NO. I PART ELEV. NEAR RIGHT SAFETY BARRIER CURB DETAILS OF PLASTIC WATERSTOP Fill face DETAILED JUNE 1988 DETAILS OF SAFETY BARRIER CURB AT END ABUTMENTS of Abut. A-4738 JACKSON COUNTY CHECKED JULY 1958 Note: This drawing is not to scale. Follow dimensions.

FOR INFORMATION ONLY A47382. Sht. 21 COMPLETE BILL OF REINFORCING STEEL COMPLETE BILL OF REINFORCING STEEL MARK NARK DIMENSIONS DIMENSIONS NOMINA LENGTH ACTUAL LENGTH ACTUAL LENGTH WEIGHT WEIGHT NO. NO. 109 D 14 B C E LOCATION В C D F K LOCATION SHAPE SUBSTR Q IN FT IN. FT. IN FE IN FT IN FT IN LBS. IN FT IN FT IN FI IN. FT. IN. FT. IN. LBS E 20 X V 2 3 4-000 5 5-500 10 5V10 WING SUBSTRUCTURE 3 4 3 4 IRCR = 6-500 IN 5 6 5 8 C ئىخا E V11 MING E 20 X V 2 5 11.000 | INCR = 9.625 IN | 12 4.000 ABUTHENT NO. 1 18 4V11 WING 5 11 5 11 SHAPE 6 SHAPE 7 SHAPE 8 SHAPE 9 8 4H1 APPROACH SEAR 20 X 5 8.000 5 8 5 8 12 4 12 4 110 20 X 7 4-000 MIZ APPROACH SEAR 10 38 5-000 60 3 40 3 S 4AIS MINE 7 4 7 4 2 6H3 APPROACH SEAM 20 X 38 5-000 10 6H4 APROM 20 X 34 0-000 115 2 4VL3 MING 38 5 38 5 20 X 8 1-000 8 1 8 1 511 20 X 6 7-000 20 X 5 4-000 IO 6HA APRON 2 4V14 PUDWALL 6 7 6 7 4-500 3 6 3 3 21 2 X 21-000 5 6H6 APRON 2 4V15 WING 20-500 4-500 3 6 3 5 21-630 D В D 8 SH7 BEAR 18 ACKHALL 20 X 34 0-000 4 SH9 BEAR & BACKHALL 20 X 34 0-000 1 SH10 APPROACH MAURICH 20 X 30 6-000 1 SH10 APPROACH MAURICH 20 X 30 6-000 35 10 35 10 8 2W1 BEAN 15.000 9.125 23 0 23 0 30 SHAPE 10 SHAPE 11 SHAPE 12 SHAPE 13 34 0 34 0 204 34 0 34 0 30 6 30 6 4 4HIL KENE 20 X V 2 4 9-00 4 9 4 9 INT. ST. NG.2 INCR = 29.000 IN 9 7-000 WING 20 X 10 4-000 C K 9 7 9 7 6 601 F00119G 20 X 8 9-000 3 6.000 8 0.000 19 0 14 8 132 16 4 10 4 4 4HLZ WING 21 802 FGGTING SHAPE 14 SHAPE 15 8 9 8 9 491 5 8 5 8 4 4HL3 MING 20 X 5 8-000 18 503 FGOTING 20 X 5 9.000 5 9 5 9 108 20 X 11 5-000 20 X 12 11-000 20 X 7 2 2 0-000 21 4 12 4 20 X 6 8.000 S 2KT4 MIME 27 804 FOOTING & COLUMN 6 8 6 8 481 12 11 12 11 6 SHIS NING 2020 S SHIS WING SHAPE 17 3 8 9 8 0 20 X 28 6-000 20 X 28 6-000 THER = 24-000 8 0-000 6 9143 BEAN 28 6 24 6 54 6 5H17 WING 20 X 8 0-000 8080 2 6H44 BEAN 28 6 28 6 SHAPE 18 3 3 9 3 9 O ANIE WING 18 X 30 10-000 20 X V 2 3 8-500 5 9H45 BEAR 33 4 33 4 567 В 20 X 12 4-000 10 4 10 4 IMCR = 26.375 IN 18 X 28 6-000 2 91146 BEAN 31 6 31 6 211 SHAPE On -SHAPE 16 SHAPE 19 2 4H19 MING 12 4 12 4 7 X 4 6.500 2 11.500 8 7447 BEAN END 3 8 9 8 16 X 2 3,000 20 X 5 2.000 20 X 9 5.030 20 X 14 4.000 4 4420 MING 5 2 5 2 2 4H21 WING 14 6 34 6 2 SHR2 NIME 63 4P1 COLUMN 7 11 7 11 333 20 X 16 9-000 E 20 X 5 3-000 20 X 12 6-000 15 9 18 9 6 SH23 MING 13| 5 X | 3 1.000 2 9.000 3 1.000 2 9.000 5 ,3 5 3 35 607 BEAR 13 0 12 6 558 2 SH24 MENS 2 4H25 WIRE 12 6 12 6 24 6U8 BEAM 13 5 X 2 0.625 2 9.000 2 0.625 2 9.000 10 11 10 5 375 C 41624 #136 20 X 3 2-000 3 2 3 2 4 6U9 BEAM 22-250 2 9-080 22-250 2 9-000 10 6 10 0 SHAPE 27 SHAPE 22 SHAPE 23 10 4427 SING 20 X 3 9-000 3 9 3 9 4 6010 BEAM 16-000 2 9-000 16-000 2 9-000 9 6 9 0 25 17 X 22 11.000 E 20 X 8 0.000 - VERTICAL LEG 2 5H49 WING 8 0 8 0 13 X 5 6.000 2 G.000 E 15 X 19 6.375 2 11.000 19 3 X 6 6.500 3 2-000 2 6.000 5 1.000 7 8 7 8 2 771 LING 27 8V17 COLUMN 23 10 23 10 1714 K D K K D 2 772 WING 17 8.500 8 7.500 22 7 22 6 CK SHAPE 25 2 4T3 MID MALL SHAPE 24 SHAPE 26 8 ZWZ BEAM 9 9 9 8 9-125 18-000 15 X 2 0.125 6 2.000 E 15 X 11 0.000 7 7.000 2 -754 WEST 10.000 22.000 6 2 8 2 2 715 WELG 10 0-000 4 7-000 18 7 16 6 E 19 S X 4 8-000 3 10-000 2 474 MING 10 4 10 5 ABUTMENT SIG. 3 E 15 X 8 10-507 4 0-800 7 11-750 3 10-500 12 11 12 10 4 GHE APPROACH BEAM 20 X 5 8-960 18 X 38 5-000 2 717 MING 5 8 5 8 8 MIZ APPROACH BEAM 84 CK С _____ 13 5 X 2 3-750 2 5-625 2 3-750 2 5-625 10 8 X 17-500 6-000 20 X 30 5-000 20 X 34 0-000 21 S X 21-040 113 2 6H3 APPROACH BEAR 36 5 38 5 SHAPE 28 SHAPE 27 SHAPE 29 35 AUL APPROACH BEAM 14 4H4 APROM 34 0 34 0 715 1-500 3 6 3 3 31 AUZ APPROACH HAURCH 3 5 3 3 7 6H5 APRON 20.500 21-000 13 S X 3 3-000 32 4U3 SEAM 2 8-250 + 0-000 2 7-00 13 3 13 0 7 6H6 APROM 15 S X 21.000 21-000 20-500 4-500 3 6 3 5 TO MIL BEAR 4.006 3 3.000 4 3 4 1 8 867 BEAR 35 10 35 10 745 18 X 34 0+00 E 50 2 X E H 38 SUS APPROACH BEAM 2 0.000 2 3.750 4 6H8 BEAM & BACKHALL 20 X 34 0-660 34 C 34 G 204 6 4 6 1 20 496 APRON 22.000 C نے ا 15-000 4 11 4 9 34 0 34 0 4 4H9 BACKHALL 20 X 34 0-000 E 20 X 5 3-000 20 X 30 4-000 Z SH24 WING 5 3 5 3 SHAPE 30 SHAPE 31 SHAPE 32 30 6 30 6 1 4H10 APPROACH HAURCH 20 X 3 2-090 20 X 3 2-000 E 20 X 7 1-000 10 SVI APPROACH BEAR 4 1 4 1 DRIN SENA OI 20 X V 2 4 1-00 46 SYZ APRON 7 2 7 2 12 5 12 5 INCR - 25.000 IN 12 5.00 SA SVS BACKWALL 274 2 4HZ9 WING 13 9 13 9 7 1 7 1 20 X 13 9-00 0 MING 20 X 16 5-000
11 WING 20 X 16 0-000
2 WING 20 X 2 4 5-000
1 NCR = 26-750 IN 15 7-000
3 WING 20 X 14 4-000 14 5V4 N7MC M7MG E 20 X V 2 4 5-500 TMCR = 5-875 IN 7 4-500 4 SHISO WING DE E 4 6 4 6 16 5 16 5 7 5 7 3 18 0 18 0 113 6 SH31 WING SHAPE 33 MING E 20 X V 2 8 3.500 INCR = 8.750 IN 12 0.600 12 .473 8 6 8 7 12 4H32 WING 4 5 4 5 12 012 0 15 7 15 7 **BENDING DIAGRAMS** ANA DINE 20 X 6 8-600 14 4 14 4 2 4H33 HING 20 X 7 5-000 26 X 8 1-006 20 X 4 6-000 2 497 WING 7 5 7 5 Note: Two (E) addition all #4.511, #5-E10, #6-93 # #6-91E are included in bar bill for testing. 2 478 48.56 0 1 8 1 NOTES-90º HOOKS 2 479 ME MALL 180° HOOKS ALL STANDARD HOOKS AND BENDS OTHER THAN 180 DEG. TO BE BENT WITH SAME PROCEDURE AS FOR 90 DEG. STD. HOOKS. 4 0 4 0 BAR ALL GRADES ALL GRADES SIZE 6d FOR #3 THRU #5 (IN.) 66 OR 216" MIN. A OR G A OR G HOOKS AND BENDS SHALL BE IN ACCORDANCE WITH THE PROCEDURES AS 12d FOR 95 OR 2%" MIN. STIRRUP HOOK DIMENSIONS SHOWN ON THIS SHEET #3 24* 6" 3" A OR C 9. BEAM -- EPOXY COATED REINFORCEMENT. 1- 9 BEAM #4 3" 8" GRADES 40-50-60 KSI 6" 4" STIRRUP
 BAR IS INCLUDED IN SUBSTRUCTURE QUANTITIE #5 3%* 7* 5* 10" Q (IN) 4d OR 2½" MIN V - BAR DIMENSIONS VARY IN EQUAL INCREMENTS BETWEEN DIMENSIONS SHOWN ON THIS LINE AND THE FOLLOWING LINE. SIZE D - 5 4%* 8" 6" 12" #6 DETAILING A OR G AORG 1809 900 R3 1%" #7 54" 10-14" 7* SHOWN ON THIS LINE AND THE POLICIATION DITE.

NO, EA. A NUMBER OF BARS OF EACH LENGTH.

NOMINAL LENGTHS - ARE BASED ON OUT TO OUT DIMENSIONS SHOWN IN
BENDING DIAGRAMS AND ARE LISTED FOR FABRICATORS USE. (NEAREST INCH)

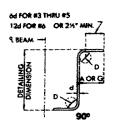
ACTUAL LENGTHS - ARE MEASURED ALONG CENTERLINE BAR TO THE NEAREST 4" 4" 214" ACKG 11* 16" R4 2" 4%* 41/2" 3* #8 6-8* SIZE of 90" HOOKS IALL GRADES SIZE OF 180" HOOKS GRADE 40 KSK #5 2½* #9 9%* 15-11%* 19" 51/4" 3%* AND 180° HOOKS (GRADE 60 KS) D SAFOR #3 THRU #11 13%" 7. 12" 74" Ofa 10%** 17" #6 415" 435" D 6d FOR #3 THRU #8 90 D 10d FOR #14 AND #18 PAYWEIGHTS ARE BASED ON ACTUAL LENGTHS 1350 2'-0" #11 12* 19* :4%* NOTE: UNIESS OTHERWISE NOTED DIAMETER "D" IS BEFORES, 410 AND #7 DETAILED = 19 THE SAME FOR ALL BENDS AND HOOKS ON A BAR. 10d FOR #14 AND #18 #14 18%° 2.3* 21%* 2*.7* A-4738 CHECKED 10 F ~ **JACKSON** COUNTY Note: This drawing is not to scale. Follow dimensions. there to BE of EN

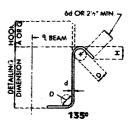
FOR INFORMATION ONLY A47382, Sht. 22 COMPLETE BILL OF REINFORCING STEEL COMPLETE BILL OF REINFORCING STEEL PROJ NO MARK MARK DIMENSIONS LENGTH WEIGHT DIMENSIONS NO. NO. ACTUAL MO 110 В C D E F Н LOCATION C D E F LOCATION B Н STIRRUP SUBSTR. VARIES NO EAC Q IN. FT. IN FT. IN. FT. IN. IN INIFT INIFT INIFT LBS. IN. FT. IN, FI IN IN. FT. IN. FT. IN. LBS 2 4H34 WING 20 X 17 4.000 127 5 17 4 21 4515 SLAB E 20 13 3.000 13 3 13 3 186 20 X 21 4-000 20 X 23 3-000 20 X 5 0-000 21 4 21 4 E 20 13 3.000 E 20 34 0.000 S 2K32 MIME 57 8SIE SLAB 13 3 13 3 2017 С K E ASET SLAB 6 SH36 BIKE 23 3 23 3 14 34 0 34 6 SHAPE 6 204 SHAPE 7 SHAPE 8 SHAPE 9 17 5518 SLAB 4 +N37 MING 3 0 5 0 E 20 34 0-000 34 0 34 0 20 X 5 0,000 20 X 5 2,000 20 X 20 11,000 20 X 3 3,000 20 X 7 7,000 20 X Y 2 3 1,250 603 4 4H34 WING 5 2 5 2 17 SSL9 SIDEWALK 11-875 4 11-000 15.000 8-750 7 2 7 1 7-250 126 E 9 S 1.500 E 20 V 1 12 2.006 2 4H39 WING 10 11 10 11 1 5520 SEDEWALK 1.500 11.875 16.750 7.250 8-750 2 6 2 5 10 WHO MUDIALL 3 3 3 3 6 4521 STOENALK 12 2 12 2 INCR = 1.750 IM 12 11-000 2 SIDEWALK E 20 12 11-000 SHAL MING 2 2 2 2 12 11 12 11 1 4S22 SIDEWALK 6 5H42 WING 3 1 3 1 12 11 12 11 D. 0 ENCR = 25,500 EN 7 3,500 WING E 20 X 7 7,000 تی D 7 4 7 4 SHAPE 11 SHAPE 12 SHAPE 13 2 5448 ABUTKENT NG.3 E 15 X 6 6.750 23.000 E 15 X 17 1.250 5 11.000 E 20 2 718 MING 3 0-000 5 10-000 8 6 8 6 12 4517 SLAB 34 0-000 34 0 34 0 273 2 779 MING 15 2-000 7 11-000 23 0 22 11 23 5518 SLAB 34 0.000 34 C 34 O 2 4TIO MUDWALL 19 X 6 3-900 3 1-750 9 5 9 4 24 5519 SIDEWALK E 9 S 15.000 11-875 4 11-000 7.250 7 2 7 1 6.750 177 15 X 6 8-506 E 20 4 0.000 E 20 18 1.000 1 5536 SIDEWALK 3 0.000 6 0.000 8 8 8 7 2 7711 WING 23-000 4 6 4 0 SHAPE 14 SHAPE 15 E 15 X 24 11-625 2 10-000 2 7712 MING 22 5-000 11 0-000 27 10 27 9 1 4523 SLAB 18 1 18 1 19 X 6 5.000 3 3.000 E 15 X 8 4.250 2 10.500 Z 4TL3 MUDWALL 9 8 9 7 1 8S24 SLAB E 20 18 1-000 18 1 18 1 E 20 E 20 7 6-500 3 7-000 11 3 11 2 2 7TL4 MING 1 4525 SLAB 17 10-600 17 10 17 10 SHAPE 17 Z 18 0-000 1 8576 5148 18 0 18 0 1 1 1 1 1 21 4527 SLAB E 20 17 9-000 17 9 17 9 249 13 S X 2 3-750 2 9-000 2 3-750 2 7-000 10 S X 17-500 6-000 35 4U11 APPROACH SEAR 61 8528 SLAB SHAPE 18 Z 10 11 10 8 249 17 9-000 17 9 17 9 2091 31 402 APPROACH HAUNCH 3 5 3 3 67 С В 13 S X 3 3-000 2 8-250 4 0-000 2 7-000 | SIDEWALK | E 20 | V 1 17 11.000 | INCR = 2.000 IN | 18 8.625 4529 SIDEWALK 32 SUJ BEAR 13 5 13 1 437 17 AL 27 LE SHAPE 20 SHAPE 16 SHAPE 19 10 WA BEAM 10 S X 6-000 3 3-000 26 18 9 18 9 36 SUS APPROACH BEAM E 10 S X 4530 SIDEHALK E 20 17 11-000 2 0.000 2 3.750 6 4 6 1 241 17 11 17 11 28 406 APRON 22-000 15-000 10 S X 4 11 4 9 89 CAST-IN-PLACE & STAY-IN-PLACE 20 X 3 9.000 LO SVIA PPROACH BEAR _ c 3 9 3 9 30 OPTION 46 SVIT APRON 20 X 10 11-000 10 11 10 11 524 381 551 SLAB E 20 33 3-000 33 3 13 3 1321 SHAPE 21 SHAPE 22 SHAPE 23 64 SV20 BACKHALL 22 552 SLAB 6 11-000 6 11 6 11 / 462 2 5 2.000 5 2 5 2 B VERTICAL LEG E 20 X V 2 6 9.000 INCR = 30-625 IN 30 8-000 20 4V21 NING 6 9 6 9 30 8 30 8 INCR = 9.750 IN | 14 1.000 415 653 SLAD 33 3 33 3 20726 14 1 14 1 139 2 4922 9396 X 8 5.000 8 5 6 5 11 24 654 SLAB 4 4 4 4 K D K K D C K X 10 1-000 INCR = 27-875 EN | 29 10-90 2 4923 WING 10 1 10 1 29 10 29 10 13 SHAPE 25 20 X 9 4-900 20 X 4 8-000 SHAPE 24 2 4924 NING 190 555 SLAS E 20 44 0.00 E 20 37 3.00 8719 9 4 5 4 12 44 0 44 0 2 4925 NING 4 8 4 0 174 556 SLAB 37 3 37 3 6766 2 4924 MUDWALL 20 2 5 8-000 52 557 SLAB € 20 40 0.000 5 8 5 8 2149 40 0 40 0 20 X \$ 11.000 20 X 9 7.000 E 20 17 4-00 2 4927 WING 52 558 SLAB 17 4 17 4 E/H 8 11 8 13 12 2 4458 NEWS 9 7 9 7 13 2 4V29 WING 20 X 9 3.000 9 3 9 /3 PRESTRESSED PANEL OPTION CK ئے ے 2 AVEC WINE 20 X 4 8-000 4 6 4 8 SHAPE 27 WINS E 20 X V 2 3 3.000 INCA = 5.875 IN 6 2.000 14 SY31 WINS 3 3 3 3 24 654 SLAB E 20 V Z 4 4-000 6 2 6 2 INCR = 27.875 IN 29 10.000 29 10 29 10 616 2 4V32 MUDWALL 20 X 5 11-000 415 653 SLAS E 20 33 3.000 5 11 5 11 33 3 33 3 20726 E 20 44 0.000 E 20 37 3.000 10 5933 MING 40 555 SLAB E 20 X V 2 3 3-000 3 3 3 3 44 0 44 0 1835 INCR = 6-250 IN | 5 4-000 5 4 5 4 3 174 556 SLAB 37 3 37 3 6760 C 24 4934 WING E 20 X Y 2 5 9.500 E 20 55 2.000 52 659 SLAB 5 10 5 10 55 2 55 2 430 INCR = 9-125 IN | 14 2-000 426 4510 SLAB SHAPE 30 SHAPE 31 SHAPE 32 E 20 3 5.00 972 14 2 14 2 7 160 3 5 3 5 22 X 15.000 9-125 STORMALK 23 0 23 0 284 5519 SIDENALK E 9 S 15-000 11-875 4 11-000 7.250 8-750 7 2 7 1 209 21 4531 SIDEWALK 35 4 35 4 SUPERSTRUCTURE 4532 STDEWALK E 20 37 8.000 529 21 37 8 37 8 SHAPE 33 ABUTHENT NO. 1 **BENDING DIAGRAMS** 3 4SII SLAD 12 11 12 11 111 3 8512 SLAS E 20 12 11-000 12 11 12 11 102 1 4513 SLAB E 20 , 13 1-000 E 20 13 1-000 NOTES: 13 1 13 1 180° HOOKS 90° HOOKS 1 E514 SLAB 23 1 13 1 ALL STANDARD HOOKS AND BENDS OTHER THAN 180 DEG. TO BE BENT WITH ALL GRADES ALL GRADES SAME PROCEDURE AS FOR 90 DEG. STD. HOOKS. SIZE 6d FOR #3 THRU #5 (IN.) AR OR 21/4" MIL HOOKS AND BENDS SHALL BE IN ACCORDANCE WITH THE PROCEDURES AS SHOWN ON THIS SHEET A OR G A OR G 12d FOR #6 OR 2%" MIN. STIRRUP HOOK DIMENSIONS 2%* #3 5* 3" 6" HOOK IA OR G - 9 RFAM GRADES 40-50-60 KSI #4 3, 6" 8" E - EPOXY COATED REINFORCEMENT 90° HOOK 135° HOOK BAR SIZE D (IN.) #5 3%" 7" 10* 3 - SIRAUF X - BAR B INCLUDED IN SUBSTRUCTURE QUANTITIES. V - BAR DIMENSIONS VARY IN EQUAL INCREMENTS BETWEEN DIMENSIONS 4%* 8' AORG AORG #6 6* 12' 180 SHOWN ON THIS LINE AND THE FOLLOWING LINE #3 135" 21/5" 4d OR 2%" MIN. #7 5%* 10" 74 14" NO. EA. - NUMBER OF BARS OF EACH LENGTH NO.EA. - NUMBER OF BARS OF EACH LENGTH.

NOMINAL LENGTHS - ARE BASED ON OUT TO OUT DIMENSIONS SHOWN IN BENDING DIAGRAMS AND ARE LISTED FOR FABRICATORS USE. [NEAREST INCH]

ACTUAL LENGTHS - ARE MEASURED ALONG CENTERLINE BAR TO THE NEAREST #4 2" 414" 4%* 3" #8 6" 16" SIZE of 90" HOOKS ALL GRADES SIZE OF 180° HOOKS GRADE 40 KSL #5 9%* 15" 1134" 21/5" 6. 5%* 3%* #9 19* AND 180° HOOKS GRADE 60 KS D 5d FOR #3 THRU #11 #6 44" 12* 74.* 4%" #10 10% 17" 1314 22" 900 D 6d FOR #3 THRU #8 PAYWEIGHTS ARE RASED ON ACTUAL LENGTHS 1350 C 10d FOR #14 AND #1R #11 12* 19* :4%* 2'-0" UNLESS OTHERWISE NOTED DIAMETER "D" IS DETAILED 19 56 THE SAME FOR ALL BENDS AND HOOKS ON A BAR. D 10d FOR #14 AND #18 2'-3" 21%* 2'-7" #14 18% CHECKED J. 10 - --738 Note: This drawing is not to scale. Follow dimensions. JACKSON COUNTY Sheet No. 23 of 24

A47382, Sht. 23





		GRADES 40 5	0-60 KS'	
BAR	D	190°HOOK	135° +	HOOK
SIZE	(IN.)	HOOK A OR G	HOOK A OR G	APPROX
#3	155"	4"	4"	2%*
#4	2*	415*	415*	3*
#5	2%*	6"	5%*	3%*
#6	41/2"	12"	7%"	4%*

-	DETAILING DIMENSION	HOOK IA OR G	
4			-
7	7 0-	S	סר
	46 OR 2%" ANN	180°	900
_		-	

SIZE OF 180° HOOKS (GRADE 40 KS): D - Sd FOR #3 THRU #11 D - 10d FOR #14 AHD #18

	DETAILING DIMENSION			
900	2 0 0 2			
SIZE of 90° HOOKS IALL GRADES				

SIZE :	of 90° HOOKS (ALL GRADES)
AND)	180° HOCKS (GRADE 60 KSI)

D = 6d FOR #3 THIRD #6
D - 8d FOR #9, #10 AND #11
D - 106 FOR #14 AND #18

	END HOOK DIMENSIONS								
BAR		180°H	OOKS	90º HOOKS					
SIZE	D	ALL GR	ADES	ALL GRADES					
31LL	(M)	A OR G	3	A OR G					
#3	2%*	5~	3"	6-					
#4	3″	6"	4"	8"					
#5	3%"	7-	5"	10"					
#6	415"	8-	6*	12-					
#7	E',."	10"	7*	14"					
#8	6*	11"	8-	16"					
119	Y%*	15*	11%*	19*					
#10	10%""	17"	13%"	22*					
#11	12*	19*	14%*	2.0					
#14	18%*	2'.3"	21%*	2'-7"					

ALL STANDARD HOOKS AND BENDS OTHER THAN 180 DEG. TO BE BENT WITH SAME PROCEDURE AS FOR 90 DEG. STD. HOOKS.

HOOKS AND BEHOS SHALL BE IN ACCORDANCE WITH THE PROCEDURES AS SHOWN ON THIS SHEET.

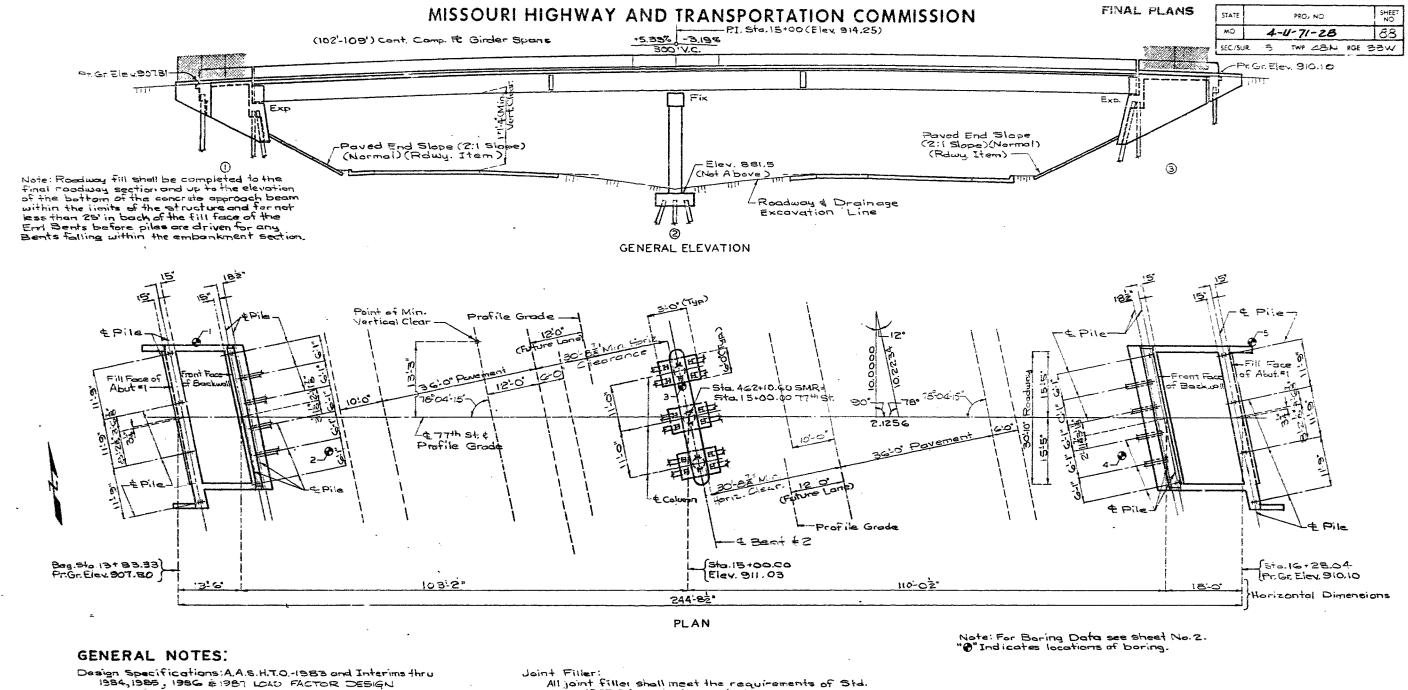
SHOWN ON 1915 SHEEL.

E - BPOXY COATED REINFORCEMENT.
S - STRRUP

X - BAR IS INCLUDED IN SUBS.INJUTURE QUANTITIES.
Y - BAR IS INCLUDED IN SUBS.INJUTURE QUANTITIES.
Y - BAR IS INCLUDED IN SUBS.INJUTURE QUANTITIES.
SHOWN ON THIS LINE AND THE FOLLOWING LINE.
NO. EA. - NUMBER OF BASE OF EACH LENGTH.
NO. BALL LENGTHS - ARE BLSD OF EACH LENGTH.
BENDING DIAGRAMS AND ARE ID...D FOR FABRICATORS USE. (NEAREST INCH)
BENDING DIAGRAMS AND ARE MEASURED ALONG CENTERLINE BAR TO THE NEAREST INCH.
PAYWEIGHTS ARE BASED ON ACTUAL LENGTHS.

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DETAILED July 19 86 CHECKED July 19 88



Design Specifications: A.A.S.H.T.O.-1983 and Interims thru 1984, 1985, 1986 \$ 1987 LOAD FACTOR DESIGN

Design Loading:

Sign Laboring.

1520-44

35789.Ft. Future Wearing Surface

Earth 1207/cu.ft., Equivalent Fluid Pressure 457/cu.ft.

Fatigue Stress-Case II

Design Unit Stresses:

Class B Concrete (Substructure) fiz=3,000psi
Class Bl Concrete (Safety Barrier Curb) fiz=4,000 psi

Reinforcing Steel (Grade GO) Fy GO, COOpsi Structural Carbon Steel fy 36,000 psi Structural Steel (ASTM ASTR) Grade SO fy 50,000 psi Steel Pile fb=9,000 psi

Fobricated Steel Connections:
Field connection, High Strength Bolts 34, holes 130, except as noted.

DESIGNED FEBRUARY 988 DETAILED MARCH 1988 CHECKED July

Note: Prebate for Oiles of approach beam 3: #1, 5-. #2 # 31. #3 to Elev. 893.00, 568.7 and 887.00 respectively.

Spec. 1057.2.4, except as noted.

Reinforcing Steel:

Minimum clearance to reinforcing steel shall be 12", unless otherwise shown.

System C by contractor in accordance with Std.
Spec.71212. (Color of the final field-cost shall be green.)

	PILE	DATA	A.			
BENT NO.			1	\2		3
Location		App. Em	Brg Sm.	Footing	Aca Em	ಶಿಗ್ರಾಶ್ರಿಗ
Pile Type and Size	~		<u></u>	Str coal		
Number		4	6	18	152.	156
Approximate Length	= +.	23	32	13	N25	85
Design Bearing	Tons	26	50	50	30	54
Hammer Energy Required	F+.Lbs.,	7000	12400	11700	7000	13200

Minimum energy requirement of hammer is based on plan length and design bearing unius of piles.
All piles shall be driven to practical refusal.

Sheet No. 1 A of =4

BAA. Elex 912.49 - top Lt. Wing 81#3

BRIDGE: 77 ST. OVER SOUTH MIDTOWN ROADWAY

STATE ROAD FROM 75th St to 85th STREET

IN KANSAS CITY

PROJECT NO.

STA. 462+10.60

JOB NO. 4-U-71-2B JACKSON

RTE. 71

COUNTY

STD. 706.35 A-4738

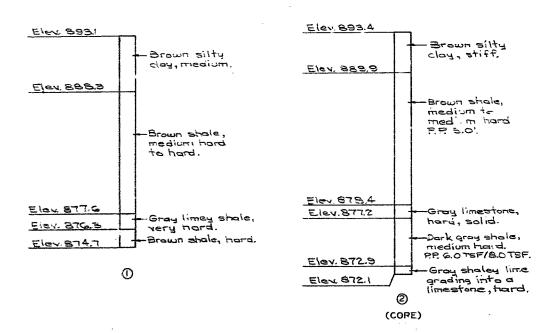
STD. 611.60

Note: This drawing of not to scale. Follow dimensions

DATE 7/25/58

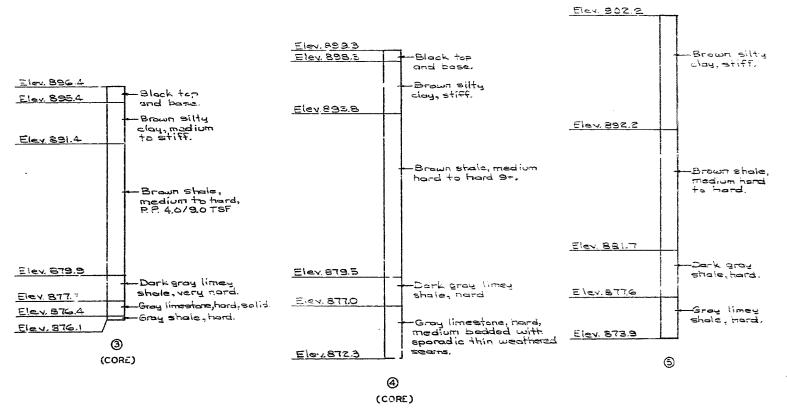
STATE	PROJ. NO:	SHEET
MO	4-4-71-28	උඉ

FINAL PLANS



FINAL	QUANTI	TIES		
ITEM		SUBSTR.	SUPERSTR	TOTAL
Class I Excavation /	Cu.4H	207		207
(72 In) Fedestrian Fence /	Lin Ft.		265	~255
Structural Stool Pi'e (10')	Lin.Pt.	850		\85B
Pre-Bore For Piling	Lin. Ft.	380		>380
Clase B Concrete /	Co.Yd.	156.5		4565
() Slab On Strel(See Spec Provis	br. pekener		792	SEL
Sefety Barrier Curb /	Lin.Ft.		500	\50 0
Slab on Semi-Deep Abutment /	Sq. Yt.		117	717
Sidewalk /	Sq.Ft.		1099	CEOM
Laminated Neoprene Sty, Pads (Steel St	octores)Ea.		12	12
Preformed Compression Exp. Jt. Seak	3.5 In Xin. Ft.		€3	~ €3
Reinforcing Steel (Srede 60)	L b.	15,780		15,780
Reinforcing Steel (Epoxy Coated)	Lb.	2690		1269C
Fabricated Structural Carbon Steel Plate	Gdr.)/Lb.		156,960	156.96
Fabricated Structural Low Alloy Steel (Pla	te Gdr.)/Lb.		35,/1Q	35,//0
Slab Drains /	Each		12	12
Painting (System C) Green	Ton		95.6	95.6
				l

Note: All concrete and reinforcing steel below top of slob and above Const. Joins under slab in Some Deep Abutments are included in superstructure quantities for Slab on Semi-Deep Abutments.
All concrete and reinforcing steel in the sidewalk are included in the superstructure quantities for sidewalks.



BORING DATA

Note: For location of borings see sheet No.1.

ESTIMATED QUANTITIES FOR	ALTER	NATE S	LAB					
		SLAB ON STEEL						
TYPE OF SLAB	ROI	REINF.(LBS.)						
	PLAIN	EPOX"	(CU,YD.)					
Cast-In-Place Conventional Forms	0	G1,410	195.6					
Precast Panel Forms	0	45,010						
Stay-In-Place Forms	0	61,410**	185.6 *					

Note: The Table of Estimated Quantities for Alternate Note: The Table of Estimated Quantities for Alternate Slabs represents the quantities used by the State in preparing the cost estimate for concrete Slabs. Variations may be encountered in these astimated quantities but these variations cannot be used for an adjustment in the Contract Unit Price per square yard of Alternate Slab used.

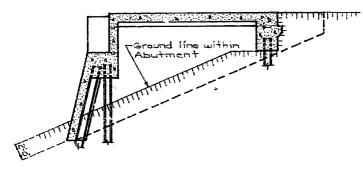
See Special Provisions for alternate methods of

forming slabs.

** Poss not include concrete required to fill corrugations of S.I.P. forms.

*** Does not include reinforcing bars used as

Precast panel quantities are based on skewed and panels.



GROUND LINE AND PILING IN ABUTMENTS

Note: In no case shall me earth within Abutments No. I and 3 be above the Ground Line shown. Forms supporting the Abutment slab may be wiftin Date. The maximum variation of the need of the pile it on the pile and the battered tope of the pile it on the position, shown on the plans shall be not more than 21 iones for pile under Abutments No I and 3.

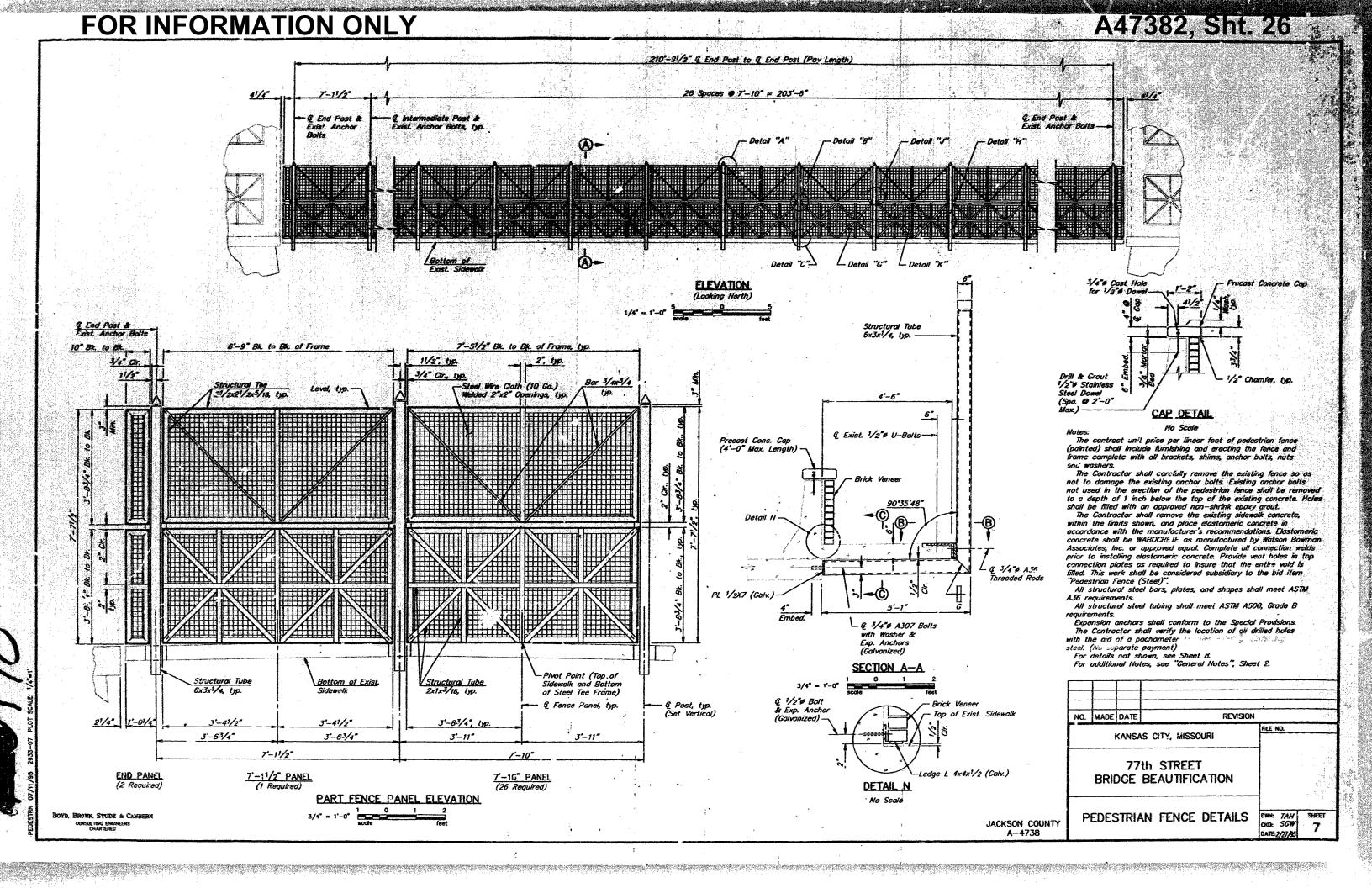
Exposed steel piles within the Abutment shall be coated with a heavy coating of an approved buttominous point.

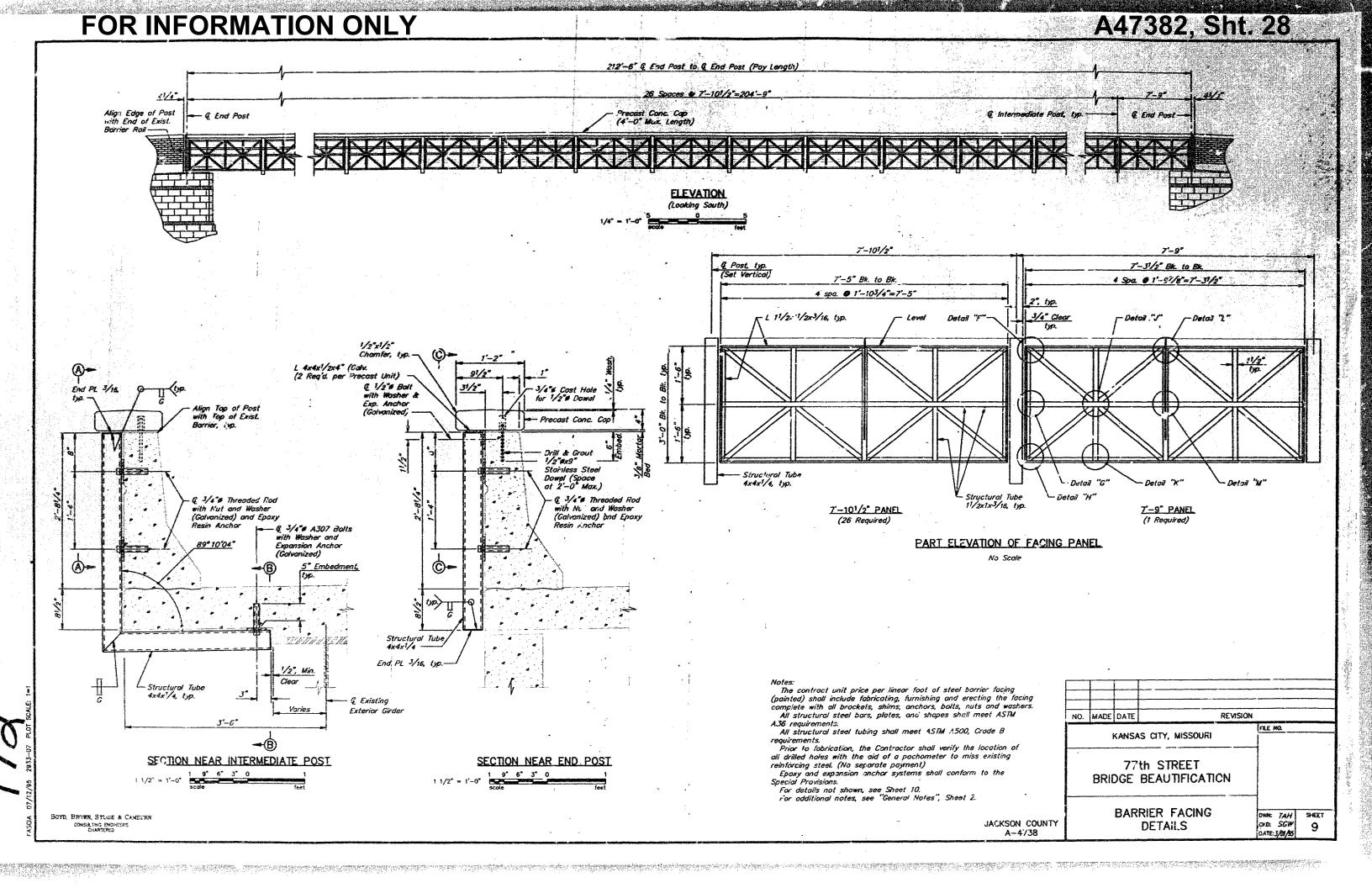
bituminous paint.

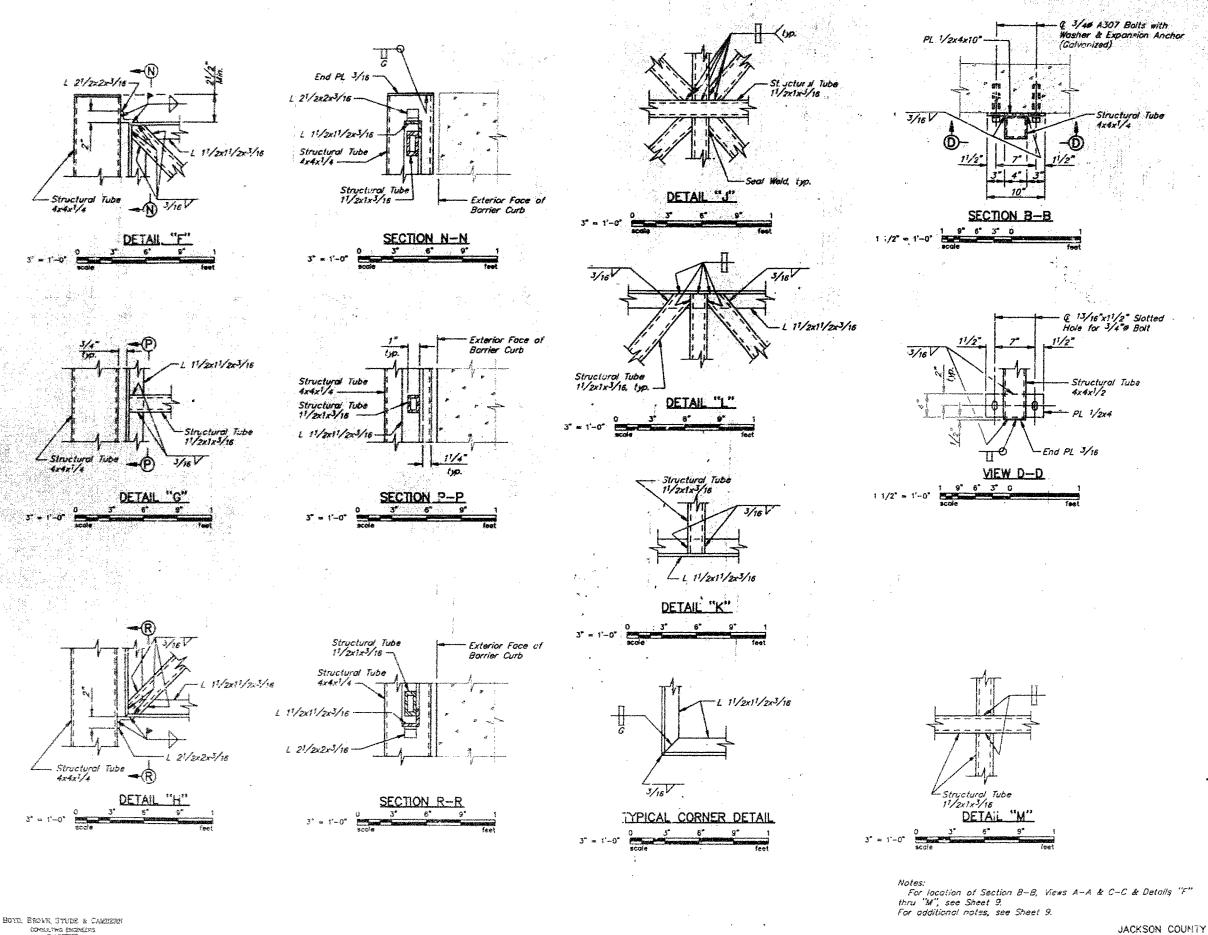
DETAILED DECEMBER 1987

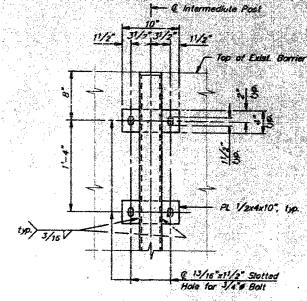
CHECKED JULY 1988

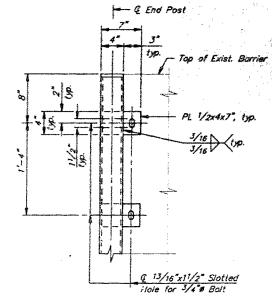
JACKSON











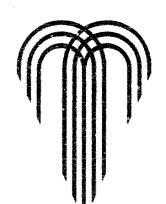
A-4738

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NO.	MADE	DATE	REVISIO	N
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	MISC		ANEOUS BARRIER ING DETAILS	DWG. TAH SHEET CKO: SGW 10

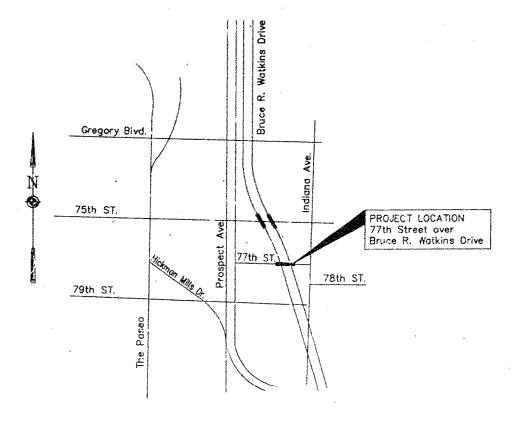
CITY OF KANSAS CITY, MISSOURI DEPARTMENT OF PUBLIC WORKS

ENGINEERING DIVISION

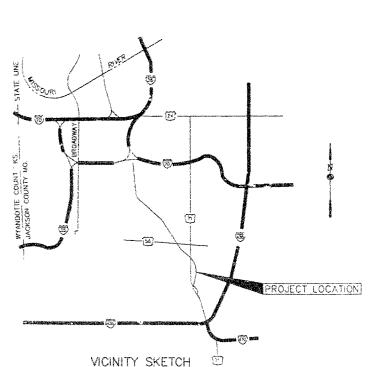
77th STREET BRIDGE BEAUTIFICATION



BRUCE R. WATKINS DRIVE



LOCATION MAP



Ochaner, Hare & Hare Kansas City, Missouri

Hellmuth, Obata & Kassabaum, Inc.

Boyd, Brown, Stude & Cambern, Chtd. Kansas City, Missouri

DuBois Consultants, Inc. Kansas City, Missouri

Group One Architects, Inc. Kancas City, Missouri



LENGTH OF PROJECT

END OF PROJECT BEGINNING OF FROJECT PROJECT LENGTH BRIDGE LENGTH

STA. 17+00 STA. 13+00 400.00" 244.71

NOTE: TRAFFIC TO BE CARRIED THRU CONSTINUCTION

EMERGENCY UTILITY NUMBERS

AME: CAN CABLEVISION
AT&T
K.C.P. & L. COMPANY - NORTH & SOUTH OF 39TH ST
KOWO - STREET & TRAFFIC DIVISION - DISPATCHER
KCHO - WATER SERVICES DEPPARTIENT 274-1256 - AFTER 5 PM 274-2463
HCI/NESTERN UNION
MISSOURI GAS ENERGY
HISSOURI PUBLIC SERVICE COMPANY
SOUTHWESTERN BELL TELEPHONE COMPANY



Charles E. Owsley, P.E. City Engineer Adopted this____ George E. Wolf Jr., P.E. Director of Public Works ENTRY NO. Project No. 16 File No

FOR FINAL REVIEW

JACKSON COUNTY

BOYD BROWN STUDE & CAMBERN

GENERAL NOTES

SPECIFICATIONS.

Design Specifications:

AASHTO Standard Specifications for Highway Bridges, 1992 Edition and subsequent interims Construction Specifications:

Standard Specifications and Design Criteria, City of Kansos City, Missouri and Special Provisions.

Missouri Standard Specifications for Highway Construction, 1993, and amendments.

All "MHTC" references herein will pertoin to the latter specification.

DESIGN UNIT STRESSES

 $f_{ij} = 4,000 \text{ psi}$ Class I Concrete fy = 60,000 psify = 36,000 psiReinforcino Steel (Grade 60) Structural Steel /b = 9,000 psi

QUANTITIES

Items not listed separately in the Summary of Quantities are subsidiary to the other items of the proposal.

DIMENSIONS

All dimensions shown on the plans are harizontal unless otherwise noted. The Contractor shall make necessary

Movember for roodway grade and cross slape.

Dimensions and Devotions pertaining to the existing structure are based on existing plans. The Contractor shall verify, by field measurement, the as-built dimensions of the existing structure prior to construction.

by qualified bidders through the City Engineer, 19th Floor, City Hall. Plans for the existing structure are available for inspection

The Contractor shall be responsible for locating all existing utilities within the work orea and providing protection for the various utilities affected before proceeding with the work.

PROTECTION OF TRAFFIC

The Contractor shall execute his work in such a manner and take such precoutions as necessary to prohibit the folling of broken concrete and other debris anto traffic possing below the structure.

The methods of protection proposed by the Contractor shall be approved by the Engineer.

MAINTENANCE OF TRAFFIC

The Control in their s in install and majorate all traffecontrol devices as shows on the pivos in accordance with section 616 of the MHTC Standard Specifications or as directed by the

The Contractor may, at his option, develop on alternate traffic control plan to be submitted to the Engineer for approval prior to its use. The traffic control plan shall be prepared in enough detail as to clearly identify proposed methods and signage necessary to provide safe travel clong 77th Street and Bruce R. Watkins Drive during all phases of construction.

STRUCTURAL EXCAVATION

All excavation required for the construction of the end monument towers, pedestriar, walls, and end posts shall be Class I Excavation. Pay limits for Class I Excavation shall be as defined in Section 206 of the MHTC Standard Specifications.

Excavation required for the construction of concrete sills for mosonry facade support at abutments and columns shall not be paid for separately but shall be considered subsidiary to the unit price bid per cubic yard for "Class I Excavation", unless

CAST-IN-PLACE CONCRETE

All cost-in-place concrete shall be Class I Concrete conforming to the requirements of the Midwest Concrete Industry Board (MCIB) Mix No. WA634-3/4-4 unless otherwise shown or noted.

All concrete masonry construction shall be performed in occordance with Section 703 of the MHTC Standard Specifications.

Surfaces which are to be exposed shall be kept free of any foreign materials which might cause staining of the concrete.

All exposed corners shall be beveled 3/4" unless otherwise.

Construction joints as shown are optional but if used shall be made only at locations shown or at locations opproved

REINFORCING STEEL

All reinforcing steel shall be deformed new billet steel conforming to the requirements of ASTM A615, Grade 60. The clear distance from the face of concrete to near edge or end of reinforcing bor shall be 2" unless otherwise shown or noted.

All bor dimensions are out to out unless otherwise shown or pioteo. All bar spacings are center to center of bar unless otherwise

shown or noted. Shop drawings and placement drawings shall be submitted

to the Engineer for review prior to construction.

All reinforcing bends shall be detailed in occordance with the Concrete Reinforcing Steel Institute (CRSI) Monual of Standard Practice, latest edition.

STRUCTURAL STEEL

All structural steel bars, plates, and shapes shall conform to the requirements of ASTM A36 unless otherwise shown or noted. All structural steel tubing shall conform to the requirements of ASTM A500, Grade B unless otherwise shown or noted. All steel fasteriers shall meet the requirements of ASTM A325 (Type 1) unless otherwise shown or noted.

Structural steel construction shall conform to the requirements of Section 712 of the MHTC Standard Specifications. Welding of structural steel shall be performed in uccordance with

STEEL BEARING PILES

the MHTC Standard Specifications.

All piles shall be driven to practical refusal. Driving shall stop when, in the opinion of the Engineer, additional driving may damage the pile. All piles shall be driven to a minimum computed bearing

Hammer Energy Required = 11,000 Ft. Lbs. (Minimum energy requirements of hammer based on plan length and design bearing value of piles)

inspection by qualified bidders through the City Engineer, 19th Floor,

DECORATIVE MASONRY VENEER

The Contractor shall instuly decorative masonry veneer on exposed concrete surfaces within the limits shown in the plans. Mosonry veneer shall consist of standard rockface (split faced) concrete masonry units, standard modular brick, and precest concrete units conforming to the requirements of the Special Provisions.

The Contractor shall secure the masonry veneer to the concrete backing with flexible anchors spaced not more than 15 inches on center vertically and 24 inches on center horizontally. The Contractor shall provide two piece anchors which permit horizontal and vertical movement but provide lateral restraint of the mason;

The masonry veneer shall be supported on cast-in-place reinforced concrete sills as shown in the plans or unless otherwise approved by the Engineer. The Contractor shall provide weepholes at concrete sills not to exceed 16 inches on center.

PROTECTIVE COATINGS

The Contractor shall apply a protective seal coat and anti-graffiti coating to all completed mosonry construction. Protective coatings shall conform to the requirements of the Special Provisions.

PAINTING OF STRUCTURAL STEEL

Structural steel surfaces of the pedestrian fencing, pedestrian handralls, barrier facing, and end monument towers shall be painted in accordance with the Special Provisions. Final topcoat color shall be dark green and shall match TNEMEC 73-G0045-ESSEX GREEN by Themec Company, Inc., CARBOLINE #2380 by Carboline, or approved equal.

Structural steel surfaces to be embedded in concrete need not be painted with the finish coating.

Unless otherwise noted, painting of the structural steel shall not be paid for separately but small be considered subsidiary to the lump sum or unit cost prices listed in the itemized proposal that include structural

PAINTING OF EXISTING BRIDGE

The Contractor shall repaint all existing structural steel (including galvanized slob drains) in accordance with Section 712.13 of the MHTC Standard Specifications (System G) unless otherwise directed by the Engineer. Final topcoat shall match color as specified for painting of structural steel.

Repainting of existing structural steel surfaces shall be paid for at the lump sum-unit price bid for "Bridge Painting".

INDEX OF SHEETS

SHEET

COVER SHEET

INDEX OF SHEETS, GENERAL NOTES & QUANTITIES

GENERAL PLAN AND ELEVATION

ABUTMENT FACADE DETAILS

COLUMN FACADE DETAILS

PEDESTRIAN FENCE DETAILS

BARRIER FACING DETAILS

END MONUMENT DETAILS END MONUMENT TOWER DETAILS

END POST DETAILS

18-20 TRAFFIC CONTROL

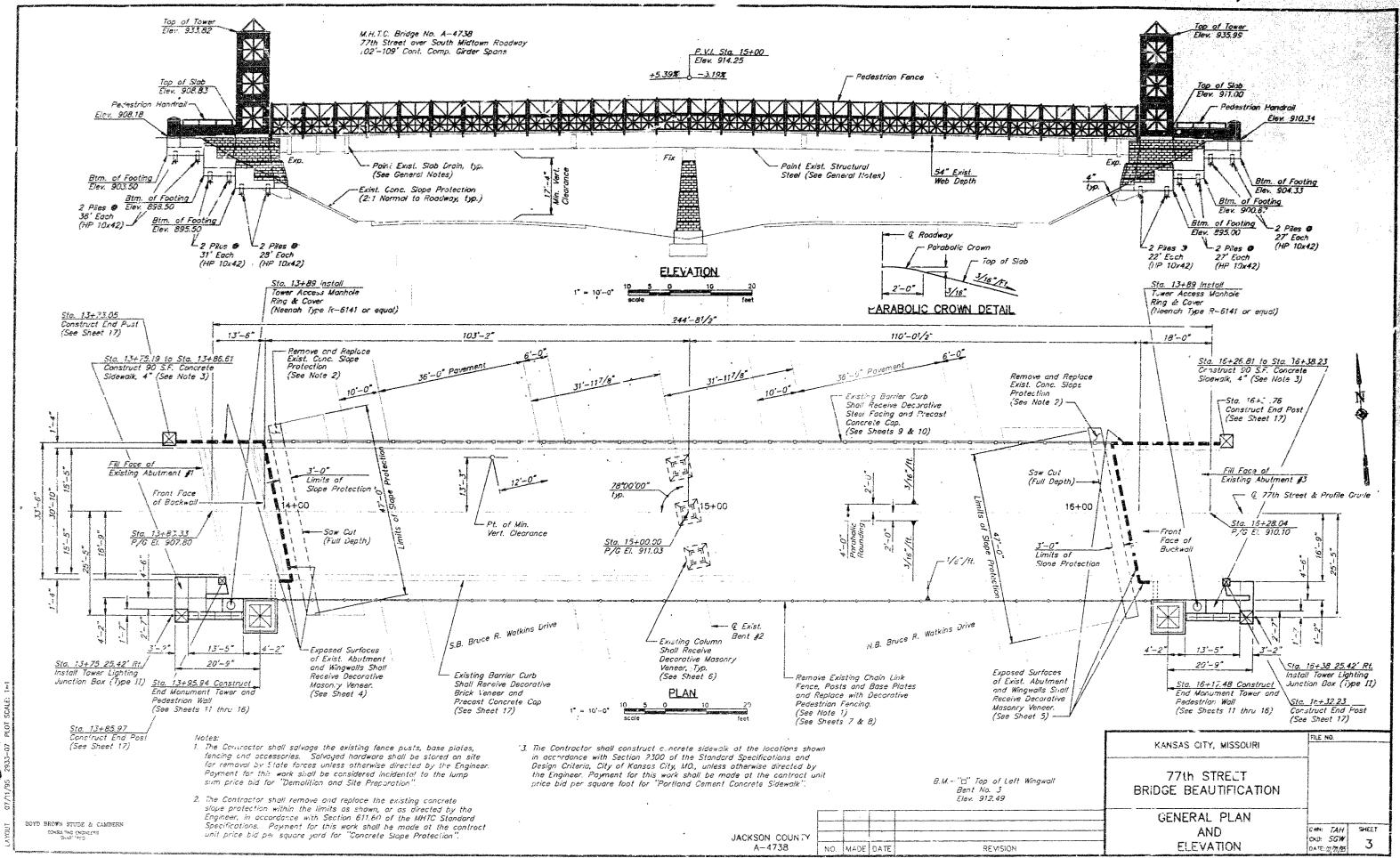
SUMMARY OF QUANTITIES								
ITEM .	UNIT	TOTAL						
MOBILIZATION	L.S.	,						
DEMOLITION AND SITE PREPARATION	LS.	1						
MAINTENANCE OF TRAFFIC	LS.	1						
CLASS I EXCAVATION	CU. YDS.	110						
CLASS I CONCRETE	CU. YDS.	92.7						
REINFORCING STEEL (GRADE 60)	LBS.	8,090						
MASONRY FACADE ON ABUTMENTS	SQ. FT.	1,026						
MASONRY FACADE ON COLUMNS	SQ. FT.	941						
MASONRY FACADE ON END MONUMENTS	SQ. FT.	* 868						
PRECAST CONCRETE CAP	LIN. FT.	533						
PEDESTRIAN FENCE (STEEL)	LIN. FT.	211						
BARRIER FACING (STEEL)	LIN. FT.	213						
PEDESTRIAN HANDRAIL (STEEL)	LIN. FT.	24						
END MONUMENT TOWER	EACH	2						
STEEL PILES (HP 10x42)	LIN. FT.	700						
CONCRETE SLOPE PROTECTION	SQ. YDS.	30						
PORTLAND CEMENT CONCRETE SIDEWALK	SQ. FT.	180						
JONEY DEGLES SYSTEM	L.S. 1	1						
GRAFFITI PROTECTION SYSTEM	LS.	1						
BRIDGE PAINTING	L.S.	7						
END POST	EACH	4						
BRICK VENEER ON BARRIER CURB	SQ. FT.	<i>33</i> 5						

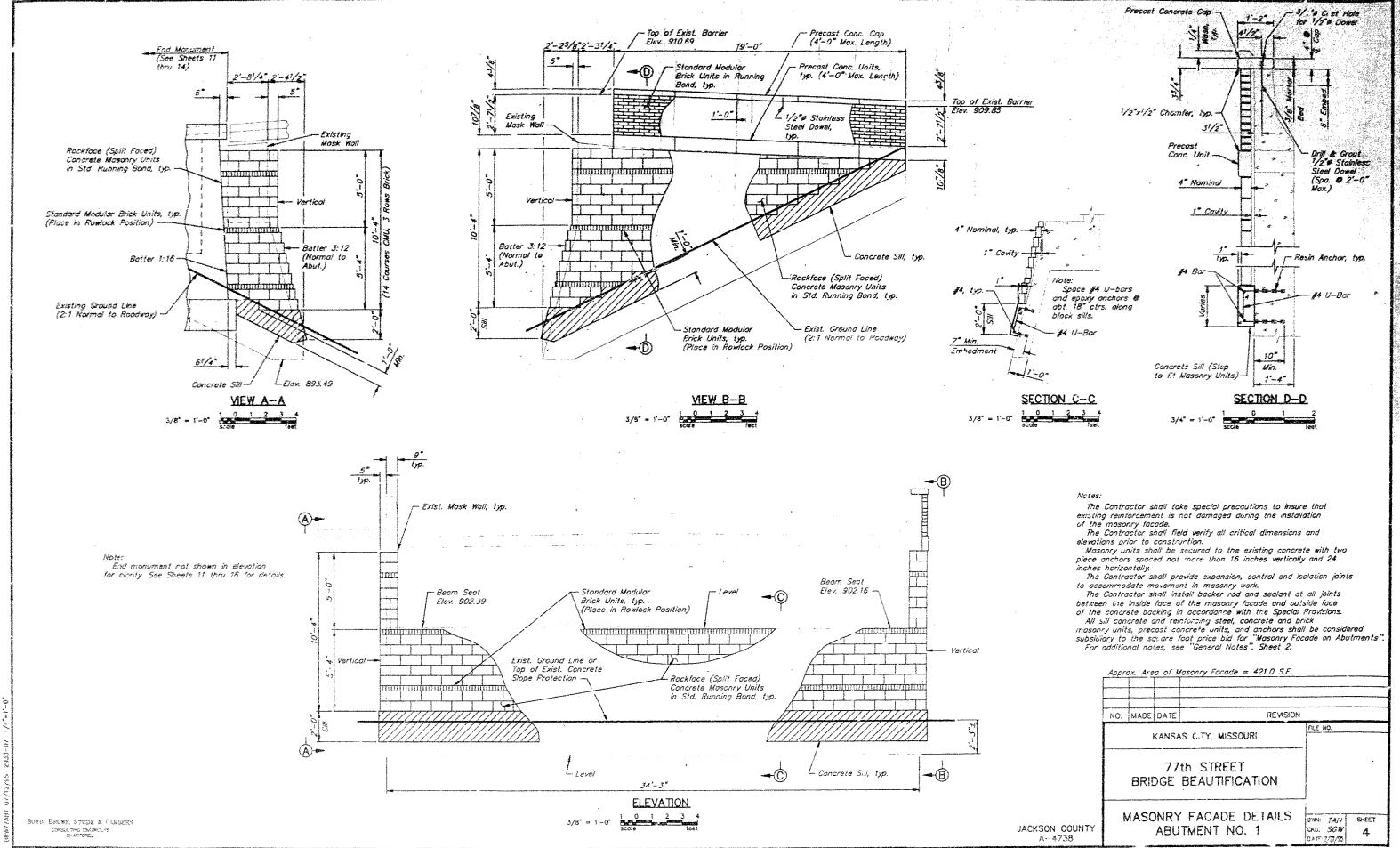
* Brick Veneer on End Monument Towers Not Included.

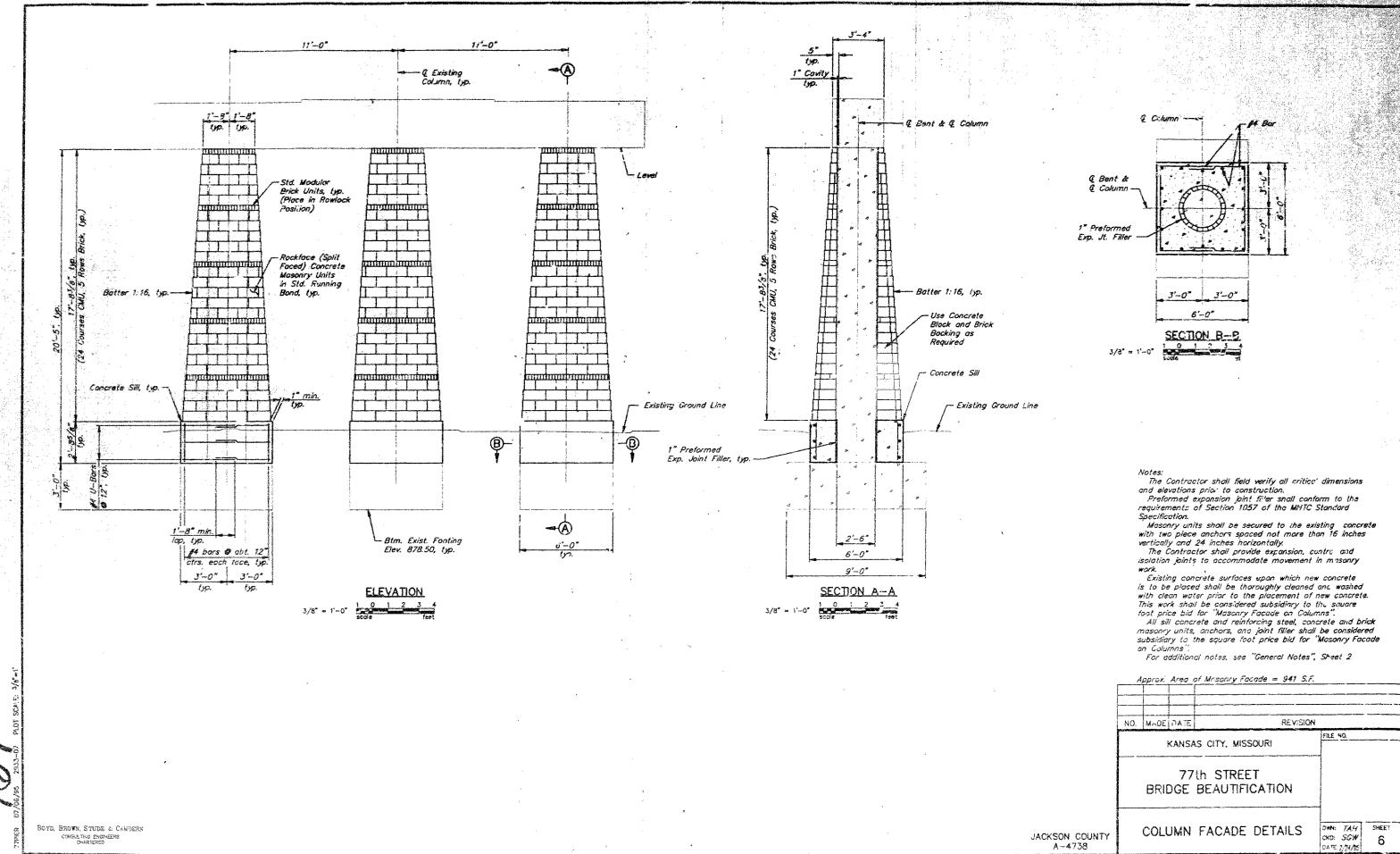
JACKSON COUNTY A-4/38

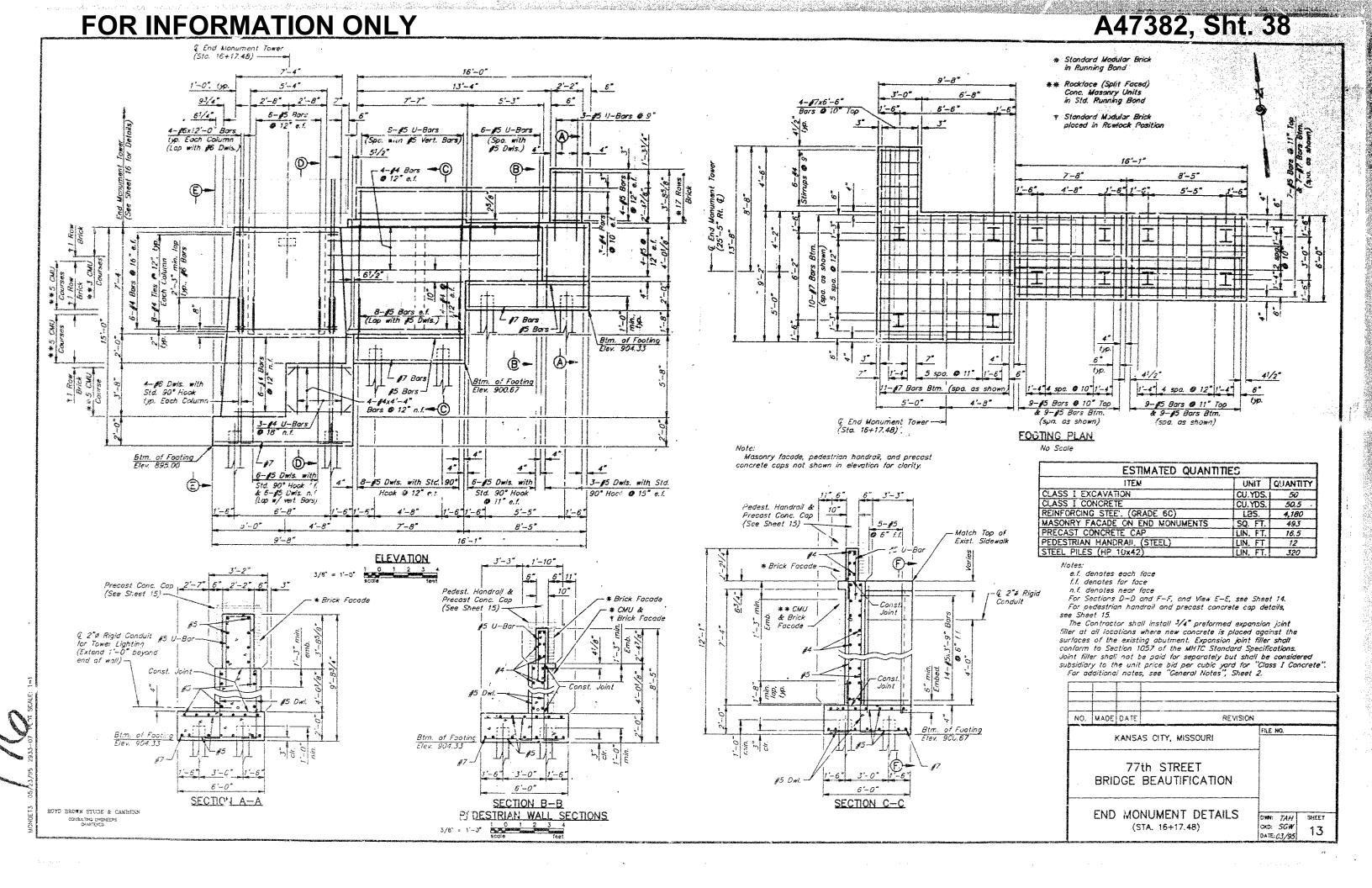
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	77th STREET BRIDGE BEAUTIFICATION					
IN			SHEETS, GENERA & QUANTITIES	L	DWN: TAH CKD: SGW	SHEET 2

BOYD EROWN STUDE & CAMPERH CHARTERD CHOMELES

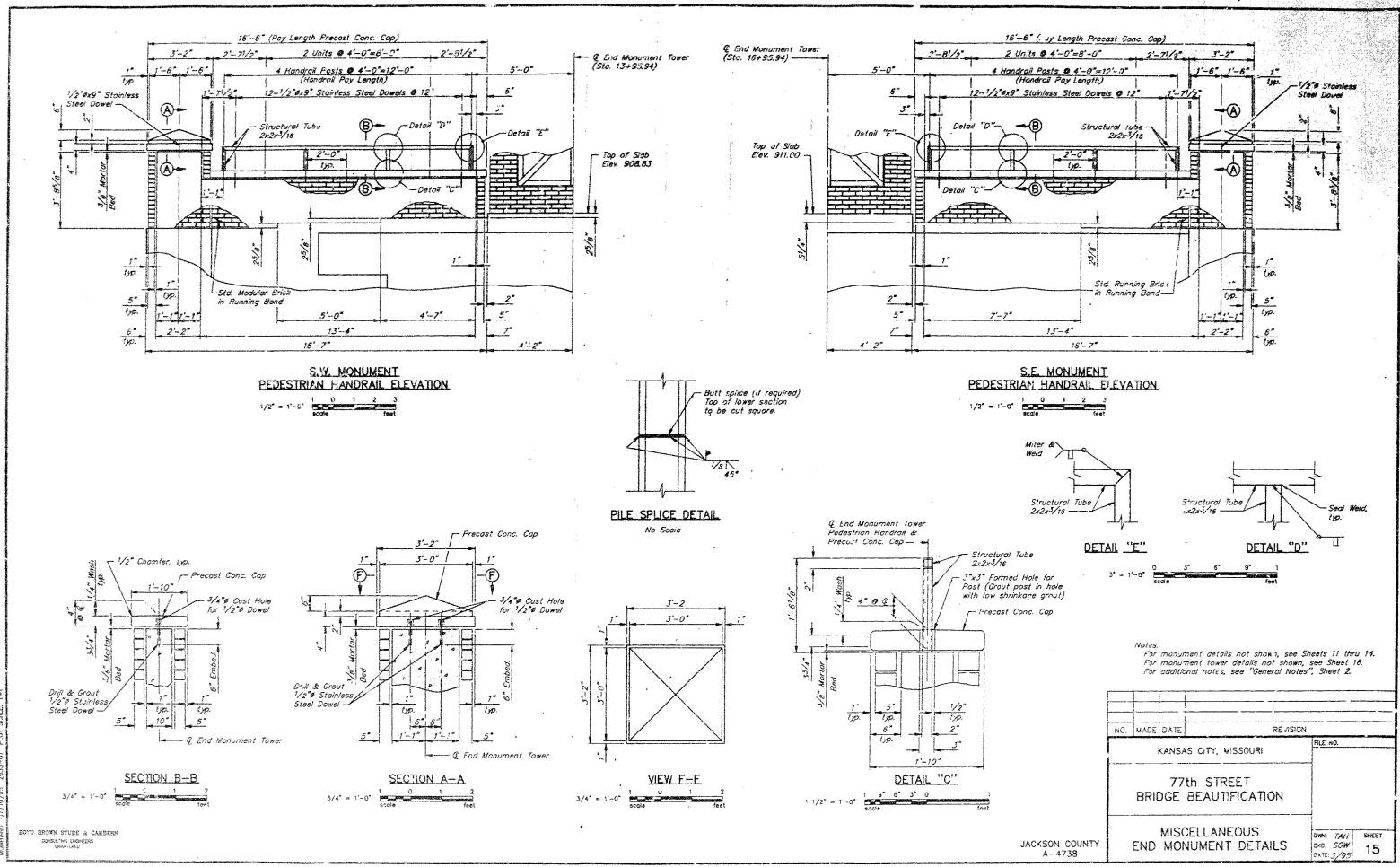


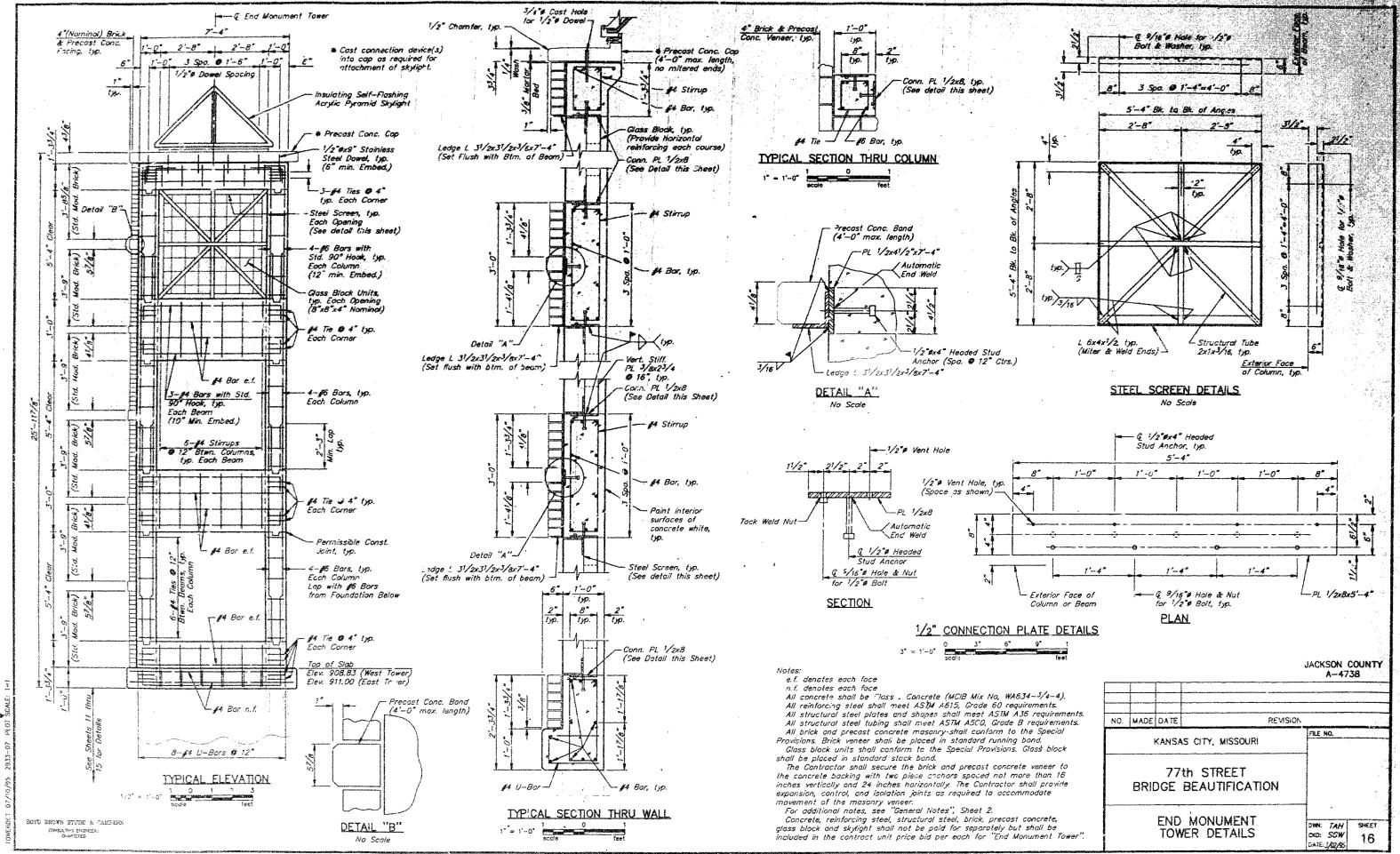


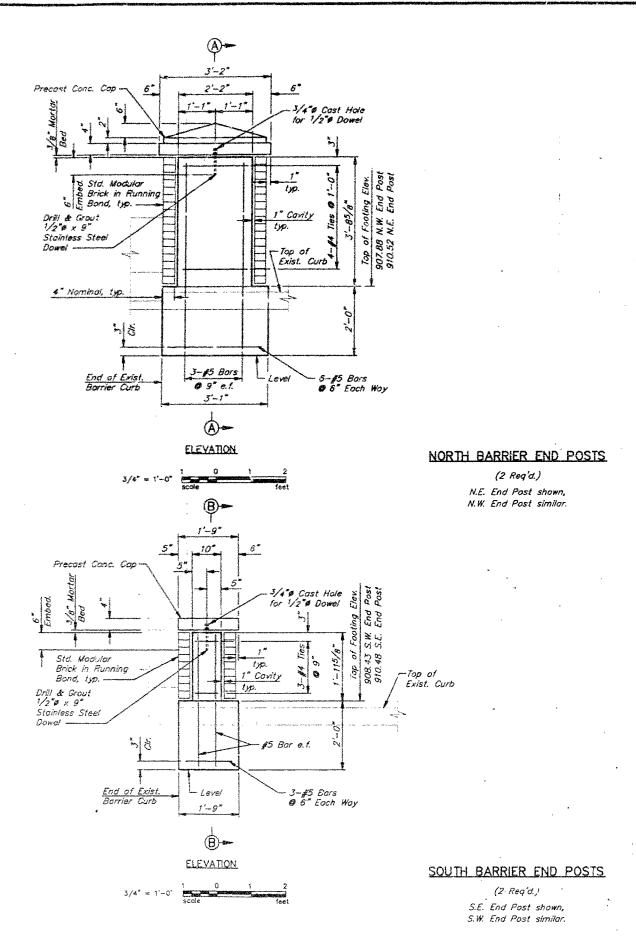


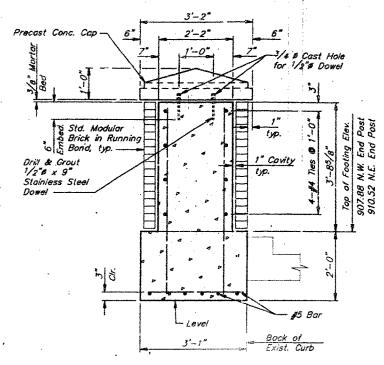


FOR INFORMATION ONLY A47382, Sht. 39 © End Monument (Sta. 16+17,48) -@ End Monument Tower (25'-5" Rt. @) Q End Monument Tower (25'-5" Rt. Q) _____ * Standard Modular Brick in Running Bond ** Rockface (Split Faced) Conc. Mesonry Units in Std. Running Bond 5'-4" * Install 1/4" Galvanized Mesh Screen 1'-0" over vent note to prevent entrance 2-8" 2-8" 4'-8" 2'-8" 51/2" of rodents. (No direct payment) * Standard Modular Brick 3-#5 Bors f.f. 6-15 Bars e.t. 61/4" 4-15 Bars •(F) placed in Rowlock Position 1'-10" 51/4" (Lop w/ Dwls.) typ. (Lop w/ Dwis.) e.f. (Lop **★3***x8* Formed Vent 41/4 Light Fixture (See Notes) 61/4 9-/4 8-45 Bars 41/4" w/ Dwis.) Light Fixture (See Notes) -Hole in Woll & # 3"x8" Forme Light Fixture (See Notes) Masonry Facade Vent Hole in Woil & Masonry Junction Box Rockface (Split Faced) CMU & Std. Mocular Rockfoce (Split Foced) (See Notes) CMU & Std. Modular Brick Focade Exist. Sidewol (Batter 1:16) (Botter 1:16) 6-#5x7-6" Bars **0** 12" 6-45 e.f. Bars (Lap with Dwis) Btwn. Wols -#5 Bors e.f (Lop with Dwls.) 6-#4 Cors Const. Join t 15 6. Const. Join 6 n & \$5 Dr. 5-45 Dwis. w/ Std. Hook @ 12" Btm. of Footing / Elev. 895.00 #7 Bors Bars @ 12" f.f. Btm. of Footing Elev. 895.00 I. in E- #3 DWS. 5-#4 U-Bors 4-\$5 Dwis. with Std. 90" Hook @ 12" n.f. & @ 12" n.f. 6 18° 1.1. 6-#5 Dwis. with Std. 90° Hook © 9° f.f. & 6-#5 Dwis. © 9° 2.f. 6'-2" 4-85 Dats. @ 12" 1.1. 6-45 Dwis. with Std. 90" Hook 6 12" n.f. & 6-45 4'-6" 6-#5x8'-6" Bars 9'-2" 3-#5 Dwls. with Std. 90" Hook 9 8" 1.1. @ 12" Btwn. Walls-13'-8" SECTION D-D Owls. 9 12" f.f. 3/8" = 1'-0" 1 2 3 VIEW E-E 9'-8" **Q** End Monument Tower SECTION F-F (sta. 16+17.48) -51/2 Precast Conc. Cop. - Q End Monument Tower & Pedest. Hondroil -Std. Modular Brick 61/2" Light Fixture (See Sheet 15)in Running Bond, typ. (See Notes) -Watch Top of Aunction Box Exist. Sidewalk (See Notes) Notes: e.f. denotes each face #5x3'-0" Bor Btm. f.f. denotes far face n.f. denotes near faca 4(55658 The Contractor shall furnish and install rigid steel conduit, junction box, and adjustable light fixtures (one fixture per wall) as shown on the plans. The Contractor shall i rnish and install rigid steel conduit Rockface (Split Faced) CMU & Std. Modular Brick Facade raceways for all conductors unless otherwise directed by the Engineer -2"# Rigid 20 Steel Conduit All materials shall meet the requirements of the Special Provisions. Electrical furnishings shall not be paid for separately but shall be 2'-0" max., 1'-10' Rockface (Split Faced) considered subsidiary to the other items of the proposal for which min. Opening: CMU & Std. Modular payment is made. Brick Facade #5x4"-0" Bor Btm. -For additional notes, see "General Notes", Sheet 2. (Batter 1:16) Const. Joint, typ. NO. MADE DATE REVISION FILE NO. 8tm. of Footing Elev. 900.67 * 7--**#**5 KANSAS CITY, MISSOURI #5x2'-3" Bars 9 Btm. of Footing Elev. 895.00 77th STREET * Field cut bors to clear openina. BRIDGE BEAUTIFICATION 3'-3" 6'-0" SECTION G-G SECTION H-H SECTION J-J END MONUMENT DETAILS BOYD BROWN STUDE & CAMBERN 1 0 1 2 3 Scole 3/8" = 1'-0" 1 2 DWN: TAH SHEET 3/8" = 1'-0" CONSULTING LINGUISERS
DHAFTERSD JACKSON COUNTY CKD: SGW (STA. 16+17.48) 14 A-4738 DATE: 03/95

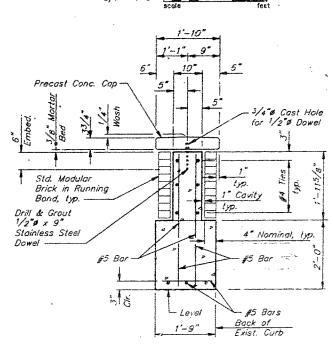


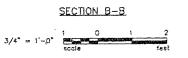






SECTION A-A





Notes:

The Contractor shall secure the brick veneer to the concrete backing with two piece anchors spaced not more than 16 inches vertically and 24 inches horizontally. The Contractor shall provide expansion, control, and isolation joints as required to accommodate movement of the brick energy.

isolation joints as required to accommodate movement of the brick veneer.

The Contractor shall install preformed expansion joint filler at all locations where new concrete is placed against the surfaces of existing concrete. Expansion joint filler shall conform to Section 1057 of the MHTC Standard Specifications. (No separate payment)

Excavation concrete, reinforcing steel, brick veneer, and precost concrete can shall not be paid for separately but shall be included in the contract, price bid per each for "End Post".

